# Stormwater Management Report

# 203 Ayer Road Harvard, MA

March 2022

<u>Submitted to:</u> Harvard Planning Board & Conservation Commission 13 Ayer Road Harvard, MA 01451

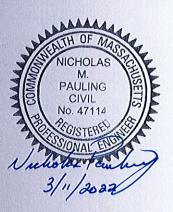
> <u>Submitted by:</u> Wheeler Realty Trust 198 Ayer Road Harvard, MA 01451

Vyonne Chern 7 Green Way Wayland, MA 01778

<u>Prepared by:</u> Goldsmith, Prest & Ringwall, Inc. 39 Main Street, Suite 301 Ayer, MA 01432

> <u>Project No:</u> 211009





# Section Title 1 Introduction and Methodology 2 Hydrology Summary for 24-hour Storm 3 Mass DEP Stormwater Management Report Checklist 4 Appendix Mapped Soil Survey Soil Mapping Overlay and Testing Locus (24" x 36") Soil Suitability Assessment for Stormwater Magement (1/13/2022) Flood Insurance Rate Map No. 25027C0314E (Eff. 7/4/2011) Pre-Development Watershed Map - Existing Conditions (11" x 17") Watershed Computations Post-Development Watershed Map - Developed Conditions (11" x 17") Watershed Computations Stormwater Quality Computations **Groundwater Recharge** Infiltrating BMP Drawdown Time Infiltrating BMP Mounding Analysis Water Quality Retention Volume TSS Removal

# Table of Contents

#### Attachments

"Commercial Development - Ayer Road Village Special Permit Application - 203 Ayer Road, Harvard, MA" Dated March 2022.

"Commercial Development - Notice of Intent - 203 Ayer Road, Harvard, MA" Dated March 2022.

Long-Term Pollution Prevention Plan & Stormwater System Operation and Maintenance Plan, Dated March 2022.

# Section 1

# Introduction and Methodology

#### Introduction and Methodology

This Stormwater Management Report is intended to accompany plans for the proposed Ayer Road Village Special Permit Application, Commercial Development at 203 Ayer Road in Harvard. Included in this report are calculations that support a final engineering design as required by the state's Wetlands Protection Act Regulations and the Town of Harvard's ordinances and regulations. Site specific information is presented under two scenarios, "pre-development" and "post-development" conditions, so that potential impacts due to the project can be identified, quantified and, as necessary, mitigated.

The final design intent seeks to meet the following interrelated goals:

- 1. Limit stormwater runoff rates and volume for the 2-, 10-, 50- and 100-year storm events to existing (pre-development) levels;
- Reduce the runoff rates and volume for the 2-, 10-year storm events by 5% of the existing (pre-development) levels per Harvard's Stormwater Regulations;
- 3. Maintain the volume of stormwater recharged per storm event to those of existing (pre-development) levels;
- 4. Prevent appreciable sediment and other suspended solids and contaminants transport by trapping them on site via Best Management Practices;
- 5. Provide adequate drainage for new surfaces;
- 6. Maintain existing drainage patterns while providing a cost-effective engineering solution that addresses regulatory as well as real-world constraints.

#### Site Description

The proposed project is commercial development to be filed under the Town of Harvard Ayer Road Village Special Permit application. The 11.3± acre project site is located on 203 Ayer Road and designated as Parcel 8-62-002 by the Town of Harvard Assessor. The project area is a 7.95± portion located on the northeastern side of the project site.

The project area is primarily low brush vegetations with some wooded areas. There is an existing hydrologically connected Bordering Vegetated Wetlands (BVW) located at the northeastern corner of the project site. There is an existing 24" culvert located at the northeastern corner of the wetland area going under Ayer Road and allowing stormwater to discharge into a bigger wetland system east of the project site. The project area generally slopes towards the hydrologically connected Bordering Vegetated Wetlands (BVW) located at the northeastern corner of the project site.

Available NRCS soils mapping for the project shows diverse soils, ranging from Hydrologic Soil Group A-C. Hydraulic Soil Group (HSG) A consisting approximately 1/3 of the project area, designated as Merrimac fine sandy loam, is located on the southern portion of the project area. Hydraulic Soil Group B consisting of approximately 1/3 of the project area, designated as Sudbury fine sandy loam, is located on the middle portion of the project area. Hydraulic Soil Group C consisting of the remaining 1/3 of the project area, designated as Walpole sandy loam, is located on the northern portion of the project area. Onsite soil evaluations were performed, and the logs reveal that the mapping is typically consistent with the field evaluation. Although the Hydraulic Soil Group for the project is generally split into three areas as mentioned above, the area containing HSG B is the largest portion within the project area. Proposed stormwater management systems will generally be located within this area of HSG B, therefore HSG B will be used for analysis purposes.

Under the pre-development scenario, the project area will be viewed as a single subcatchment area. As shown on the plan entitled "PRE-DEVELOPMENT – WATERSHED MAP", included within the attached Appendix, subcatchment SC1.0 outlines the project area flowing towards the proposed wetland replication, as shown as Analysis Point AP-1.

#### Project Description

The proposed development will construct three commercial use buildings with parking, pedestrian access, and a driveway connection on Ayer Road. The three proposed commercial-use buildings, their associated parking areas and driveway access onto Ayer Road are shown in the attached Site Plan. Building A will be constructed as a recreational center specifically for the purpose of providing court space for badminton. The badminton center will have a gross floor area of 29,998 sq. ft. The proposed development will provide 120 parking spaces, five of which shall be handicap accessible parking spaces, for the recreational center. Building B and C are shown as general commercial/office-use buildings that will be subjected to final layout design once an end-user has been identified.

In order to offset the increase in stormwater runoff on site, various stormwater Best Management Practices (BMP) are proposed. On-site stormwater BMPs are designed to capture and treat stormwater, provide groundwater recharge, and reduce stormwater runoff than existing conditions.

Under the post-development scenario, the project has been divided into a total of 15 subcatchment areas, shown on the plan entitled "POST-DEVELOPMENT – WATERSHED MAP", and included in the attached Appendix, outlining runoff to the AP-1.

Subcatchment SC1.1, SC1.2, SC1.3, and SC3.3 outline stormwater runoff that will get collected by deep sump hooded catch basins and discharge into sediment forebay No. 1 prior to continuing into Infiltration Basin (IB-1). IB-1 will have an 8" HDPE discharge pipe and an emergency overflow weir to allow stormwater to continue into the resource area.

Subcatchment SC3.1 outlines stormwater runoff from the western portion of the proposed roof area that will directly discharge into Infiltration Chambers (IC-1). SC3.2 outlines runoff from the proposed pavement area that will get collected by a deep sump hooded catch basin and discharge

Subcatchment SC3.1 outlines stormwater runoff from the western portion of the proposed roof area that will directly discharge into Infiltration Chambers (IC-1). SC3.2 outlines runoff from the proposed pavement area that will get collected by a deep sump hooded catch basin and discharge into an Isolator Row for IC-1. IC-1 will have a discharge pipe to allow stormwater to continue into the resource area.

Subcatchment SC2.1, SC2.2, SC4.2 and SC4.4 outline stormwater runoff that will get collected by deep sump hooded catch basins and discharge into an Isolator Row for Infiltration Chambers (IC-2). SC4.1 outlines stormwater runoff from the eastern portion of the proposed roof area that will directly discharge into Infiltration Chambers (IC-2). IC-2 will have a discharge pipe to allow stormwater to continue into the resource area.

Subcatchment SC2.3, SC4.2 and SC4.5 outline stormwater runoff that will get collected by deep sump hooded catch basins and discharge into Infiltration Basin (IB-2). IB-2 will have a couple of discharge pipes to allow stormwater to continue into the resource area.

Subcatchment SC5.1 outlines untreated stormwater runoff from pervious surface. As mentioned above, SC5.1 will exclude the proposed wetland replication area that was previously approved by the Town of Harvard Conservation Commission.

#### Hydrologic and Hydraulic Computation Methodology

Runoff rates were computed using the Soil Conservation Service TR-20 Method entitled "Urban Hydrology for Small Watersheds" within the HydroCAD Stormwater Modeling software platform. The following 24-hour rainfall events from the Northeast Regional Climate Center (NRCC) Extreme Precipitation Tables database were analyzed:

Frequency (years): 2, 10, 50 and 100

As outlined above, runoff from the site has been analyzed at one point under the pre-development and post-development conditions. As a standard for comparison AP-1 is represented in both the pre and the post development cases.

#### Summary of Results

Peak discharge rates and volumes of the calculated runoff for both conditions analyzed are displayed in the HYDROLOGY SUMMARY that follows. As shown within the summary, the peak discharge rates and volumes at Analysis Point AP-1 for all analyzed storm events are less than or equal to those under pre-development conditions. Peak discharge rates and volumes for the 2- and 10-yr storm events under the proposed conditions are also at least five percent lower than the existing conditions, in accordance with the Town of Harvard's Stormwater Management Regulations.

The deep sump hooded catch basins, Isolator Row, infiltration chambers, sediment forebay and infiltration basins work together to provide an expected Total Suspended Solids (TSS) removal of 85% for each of the treatment train, prior to discharging into the proposed wetland replication area.

The two sets of infiltration chambers will retain and infiltrate 12,898 cubic feet of runoff prior to discharging, well in excess of the minimum required 3,540 cubic feet occurring under existing conditions and displaced by the proposed development. The proposed infiltration volume will also be in excess of the required Water Quality Volume generated by the proposed impervious area on site.

The proposed development meets the MADEP Stormwater Management Standards through the use of Best Management Practices that address groundwater recharge, water quality (first flush) retention, and suspended solids removal within sustainable BMP's. See Appendix for computed solids quantities / removal process trains, and water quality runoff volumes.

# Section 2

# Hydrology Summary for 24-hour Storm

# **HYDROLOGY SUMMARY FOR 24-HOUR STORM**

203 Ayer Road, Harvard, MA Project No. 211009

## PEAK DISCHARGE RATE

#### **Pre-Development (cfs)**

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	2.8	7.9	17.2	22.9
5% Reduction [1]	2.7	7.5		

#### **Development (cfs)**

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	2.2	6.9	15.5	21.7

#### Pre-Development vs. Developed (cfs)

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	-0.5	-0.6	-1.7	-1.2

## PEAK DISCHARGE VOLUME

#### **Pre-Development (Cubic feet)**

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	14,640	35,632	74,402	99,106
5% Reduction [1]	13,908	33,850		

#### **Development (Cubic feet)**

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	12,098	32,113	72,662	98,772

#### Pre-Development vs. Developed (Cubic feet)

Analysis Point	2-YR	10-YR	25-YR	100-YR
AP-1	-1,810	-1,737	-1,740	-334

[1] Harvard's Wetlands Protection Bylaw Regulations: Chapter 147-14.C.(1) Stormwater Management: Peak runoff rates and volume under proposed conditions must be reduced by at least five percent compared to existing conditions for the 2- and 10-yr frequency storm event, and peak runoff rates and volumes under proposed conditions compared to existing conditions shall not exceed existing peak runoff rates and volume for the 50- and 100-year frequency storm events.

Section 3

Mass DEP Stormwater Management Report Checklist



# Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

# A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.<sup>1</sup> This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8<sup>2</sup>
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

<sup>&</sup>lt;sup>1</sup> The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

<sup>&</sup>lt;sup>2</sup> For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



# Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

# **B. Stormwater Checklist and Certification**

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

*Note:* Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

# **Registered Professional Engineer's Certification**

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Longterm Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



2022 Signature and Date

Checklist

**Project Type:** Is the application for new development, redevelopment, or a mix of new and redevelopment?

New development

- Redevelopment
- Mix of New Development and Redevelopment



**LID Measures:** Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

$\boxtimes$	No disturbance to any Wetland Resource Areas
$\boxtimes$	Site Design Practices (e.g. clustered development, reduced frontage setbacks)
	Reduced Impervious Area (Redevelopment Only)
$\square$	Minimizing disturbance to existing trees and shrubs
	LID Site Design Credit Requested:
	Credit 1
	Credit 2
	Credit 3
	Use of "country drainage" versus curb and gutter conveyance and pipe
	Bioretention Cells (includes Rain Gardens)
	Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
	Treebox Filter
	Water Quality Swale
	Grass Channel
	Green Roof
$\boxtimes$	Other (describe): Direct roof recharge

#### **Standard 1: No New Untreated Discharges**

- $\boxtimes$  No new untreated discharges
- $\boxtimes$  Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



#### Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

Calculations provided to show that post-development peak discharge rates do not exceed predevelopment rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24hour storm.

#### Standard 3: Recharge

Soil Analysis provided.

- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.

🖂 Static
----------

Dynamic Field<sup>1</sup>

 $\boxtimes$  Runoff from all impervious areas at the site discharging to the infiltration BMP.

Simple Dynamic

Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

Recharge BMPs have been sized to infiltrate	e the Required Recharge Volume.
---	---------------------------------

Recharge BMPs have been sized to infiltrate the Required Recharge Volume only to the maximum
extent practicable for the following reason:

- Site is comprised solely of C and D soils and/or bedrock at the land surface
- M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- Solid Waste Landfill pursuant to 310 CMR 19.000
- Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- $\boxtimes$  Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

<sup>&</sup>lt;sup>1</sup> 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



#### Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

#### **Standard 4: Water Quality**

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
  - is within the Zone II or Interim Wellhead Protection Area
  - is near or to other critical areas
  - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
  - involves runoff from land uses with higher potential pollutant loads.
- The Required Water Quality Volume is reduced through use of the LID site Design Credits.
- Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist	(continued)	)
-----------	-------------	---

#### Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
  - The ½" or 1" Water Quality Volume or
  - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

#### Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does *not* cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

#### **Standard 6: Critical Areas**

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



# Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

Limited	Proj	ect
---------	------	-----

- Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- Bike Path and/or Foot Path
- Redevelopment Project
- Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

#### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is *not* covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

#### **Standard 9: Operation and Maintenance Plan**

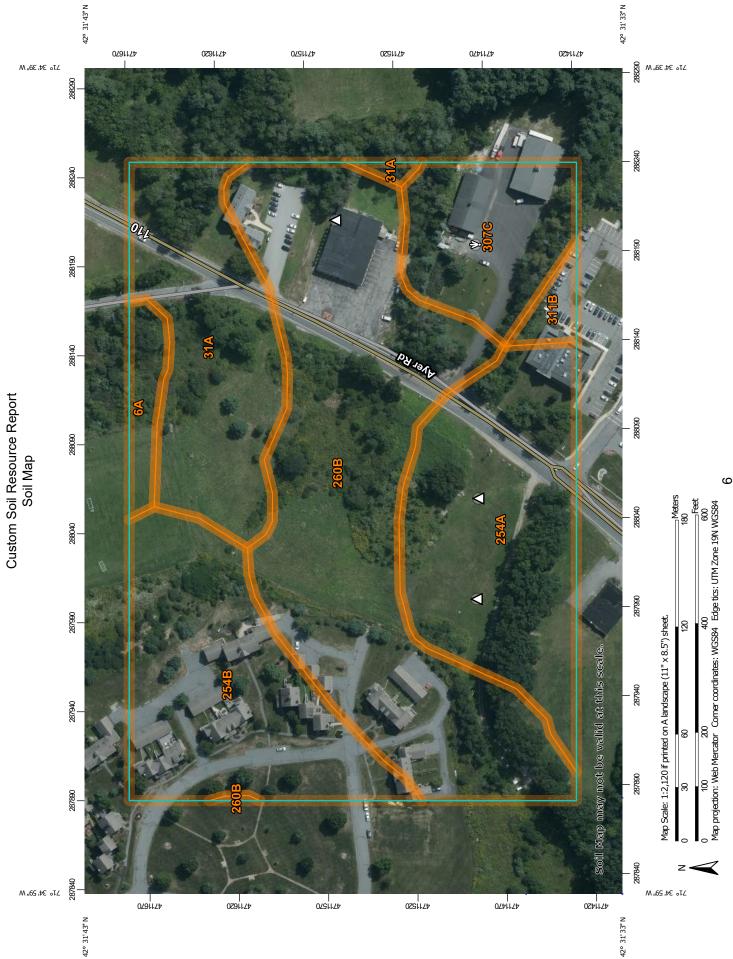
- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
  - Name of the stormwater management system owners;
  - Party responsible for operation and maintenance;
  - Schedule for implementation of routine and non-routine maintenance tasks;
  - Plan showing the location of all stormwater BMPs maintenance access areas;
  - Description and delineation of public safety features;
  - Estimated operation and maintenance budget; and
  - Operation and Maintenance Log Form.
- The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
  - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
  - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

#### Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted *prior to* the discharge of any stormwater to post-construction BMPs.

Section 4

<u>Appendix</u>



Area of Interest (AOI)       Soil Map Unit Polygons       Soil Map Unit Lines       Net Spot         Soil Map Unit Lines       Soil Map Unit Lines       Very Story Spot         Soil Map Unit Lines       Soil Map Unit Lines       Very Story Spot         Soil Map Unit Lines       Soil Map Unit Lines       Very Story Spot         Soil Map Unit Lines       Soil Map Unit Lines       Very Story Spot         Soil Map Unit Lines       Very Story Spot       Very Story Spot         Soil Map Unit Lines       Very Story Spot       Very Story Spot         Soil Map Unit Lines       Very Story Spot       Very Story Spot         Soil Map Unit Lines       Very Story Spot       Very Story Spot         Spoil Features       Soil Map Unit Points       Very Story Spot         Spot Spot       Borow Pit       Very Story       Very Story         Sol Closed Depression       Very Story       User Rails       Very Story         Sol Closed Depression       Very Story       User Rails       Very Story         Sol Closed Depression       Very Story       User Rails       User Rails         Soil Map of Spot       User Rails       User Rails       User Rails         Soil Map of Spot       User Rails       User Rails       User Rails         Soil Map of Spot <th></th>	
<ul> <li>Soil Map Unit Lines</li> <li>Canals</li> <li>Soil Map Unit Lines</li> <li>Soil Map Unit Lines</li> <li>Soil Map Unit Lines</li> <li>Canals</li> <li>Soil Map Unit Lines</li> <li>Carvel Pit</li> <li>Carvel Pit</li></ul>	
Lines Points ssion mp Backgrounc v Mater ier	
Points ssion mp Backgrounc v Mater	
water Featu Ssion mp Background v Mater	
Pit ot Depression Depression Spot Spot Spot Spot Spot Spot Spot Spot	
ot Transportation Depression Electron Spot Revealed I al Water Mater Jerop	
Depression Dit Spot spot r Spot r Swamp r swamp Quarry Duarry Ineous Water Ineous Water Itrop	
Pit Spot Spot ow Background r swamp Quarry Quarry Ineous Water Introp	
<ul> <li>Spot</li> <li>W</li> <li>Background</li> <li>r swamp</li> <li>Quarry</li> <li>Quarry</li> <li>Mater</li> <li>Mater</li> <li>Itcrop</li> </ul>	
ow Background r swamp Eackground Quarry Cuarry Ineous Water al Water Acrop	Coordinate System: Web Mercator (EPSG:3857)
Background wamp ackground Jarry ous Water Mater rop	Maps from the Web Soil Survey are based on the Web Mercator
2	projection, which preserves direction and shape but distorts
or Quarry Ilaneous Water mial Water Outcrop	Albers equal-area conic projection, should be used if more
silaneous Water nial Water Outcrop	accurate calculations of distance or area are required.
mial Water Outcrop	This product is generated from the USDA-NRCS certified data as
Outcrop	of the version date(s) listed below.
	Soil Survey Area: Worcester County, Massachusetts,
Saline Spot	
Sandy Spot	
Severely Eroded Spot	Soil map units are labeled (as space allows) for map scales
Sinkhole	
Slide or Slip	Date(s) aerial images were photographed: Aug 12, 2019—Sep
Sodic Spot	29, 2019

# Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
6A	Scarboro mucky fine sandy loam, 0 to 3 percent slopes	0.5	2.3%
31A	Walpole sandy loam, 0 to 3 percent slopes	3.4	15.2%
254A	Merrimac fine sandy loam, 0 to 3 percent slopes	4.2	18.9%
254B	Merrimac fine sandy loam, 3 to 8 percent slopes	3.9	17.5%
260B	Sudbury fine sandy loam, 3 to 8 percent slopes	8.1	36.2%
307C	Paxton fine sandy loam, 8 to 15 percent slopes, extremely stony	1.9	8.6%
311B	Woodbridge fine sandy loam, 0 to 8 percent slopes, very stony	0.3	1.3%
Totals for Area of Interest		22.3	100.0%

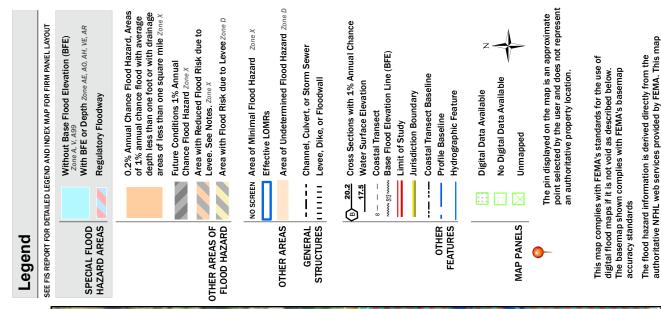
# National Flood Hazard Layer FIRMette

°35'6"W 42°31'50"N

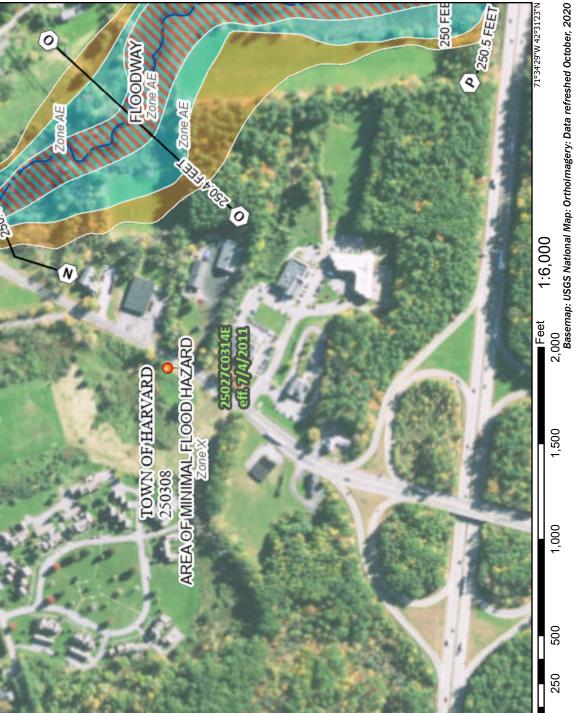


2

Σ



reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or was exported on 2/8/2022 at 2:34 PM and does not become superseded by new data over time. This map image is void if the one or more of the following map legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for elements do not appear: basemap imagery, flood zone labels, regulatory purposes.



No. 211009

Date: 1/17/22

Commonwealth of Massachusetts Harvard, Massachusetts

# Soil Suitability Assessment for Stormwater Management

Performed by: Limhuot Tiv, GPR Inc	Date: 1/13/22
Witnessed by: N/A	
Location Address:	Owner's Name: Wheeler Realty Trust
or Lot No. 203 Ayer Road	Address: 198 Ayer Road
Harvard, MA	Harvard, MA 01451
	Telephone No.
New Construction 🗹 Upgrade 🔲 Repa	r 🗖
Office Review	
—	
	n/a Soil Map Unit 254A, 260B & 31A
Soil Name Merrimac fine sandy loam Soil Limitations	well drained
Soil Name Sudbury fine sandy loam Soil Limitations	well drained, shallow to groundwater poorly drained, shallow to groundwater
Soil Name Wapole sandy loam Soil Limitations	
Surficial Geologic Report Available: No 🔲 Yes	
Year Published Mass Mapper Publication Scale	n/a
Geologic Material(Map Unit) Sand and gravel, Till or l	bedrock
Landform Ground Morraine	
Flood Insurance Rate Map: 25027C0314E	
	s <b>V</b>
Within 500 Year Flood Boundary No 🗹 Yea	
Within 100 Year Flood Boundary No 🗹 Yea	
Within Velocity Zone No 🗹 Yes	
Wetland Area:	
National Wetlands Inventory Map (map unit) N/A	
Wetlands Conservancy Program Map (map unit) N/A	
······································	
Current Water Resource Conditions (USGS): Month	February
Range: Above Normal 🗹 Normal 🔲 Below Nor	
Other Reference Reviewed USGS	

Location Address or Lot #: 203 Ayer Road Harvard, MA

## **On-Site Review**

Deep Hole #: 122-1	Date:	01/13/22 Time:		8:00AM	Weather:	Cloudy 27°
Location (identify on site pla	in)	See Attached Ske	etch			
Land Use vacant lot		Slope (%)	0-3%		Surfaces Stones	few
(eg woodland, agricultural fi	eld, va	cant lot etc)				
Vegatation mixed hardwood	s and	pines				
Landform Ground Morrain	e					
Position on landscape	See a	ttached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	age Way	>100	feet	
Possible Wet Area	>100	feet Proper	rty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

Deep Observation Hole Log							
Hole # 122	-1	NB 14/E-31			Suface El. 301.9		
Depth from	Soil	Soil Texture	Soil Color	Soil	Other		
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,		
(inches)					Consistency, % Gravel)		
0-12	А	FSL	10YR 3/3				
12-42	C1	S	10YR 5/4				
42-85	C2	SL	2.5Y 5/3	@42"			
				10YR 6/4			
				2.5Y 6/2			
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	ROPOSED	DISPOSAL AREA		
	( 1 · )				· D 1 1 05"		

	Depth to Bedrock: 85"	
None	Weeping from Pit Face:	None
42"		
	None	42"

Location Address or Lot #: 203 Ayer Road Harvard, MA

## **On-Site Review**

Deep Hole #: 122-2	Date:	01/13/22 Time:		8:20AM	Weather:	Cloudy 27°
Location (identify on site pla	n)	See Attached Ske	etch			
Land Use vacant lot		Slope (%)	0-3%		Surfaces Stones	few
(eg woodland, agricultural fi	eld, va	cant lot etc)				
Vegatation mixed hardwood	ls and j	pines				
Landform Ground Morrain	e					
Position on landscape	See at	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	ige Way	>100	feet	
Possible Wet Area	>100	feet Proper	ty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

Deep Observation Hole Log							
Hole # 122	-2	NB 14/E-31			Suface El. 304.0		
Depth from Surface (inches)	Soil Horizon	Soil Texture (USDA)	Soil Color (MUNSELL)	Soil Mottling	Other (Stucture, Stones, Boulders, Consistency, % Gravel)		
0-12 12-50 50-91	A C1 C2	FSL S SL	10YR 3/3 10YR 5/4 2.5Y 5/4	@50" 10YR 6/4 2.5Y 6/2			
	*MINIMUM OF 2 HOLES REQUIRED AT EVERY PROPOSED DISPOSAL AREA Parent Material (geologic) Glacial Till Depth to Bedrock: >91"						

 Parent Material (geologic)
 Glacial Till
 Depth to Bedrock: >91"

 Depth to Groundwater: Standing Water in the Hole
 88"
 Weeping from Pit Face: 78"

 Estimated Seasonal High Groundwater in the Hole
 50"
 Aditional Notes

122-2

Location Address or Lot #: 203 Ayer Road Harvard, MA

# **On-Site Review**

Deep Hole #: 122-3	Date:	01/13/22 Time:		8:40AM	Weather:	Cloudy 27°
Location (identify on site pl	an)	See Attached Ske	etch			
Land Use vacant lot		Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural f	ield, va	cant lot etc)				
Vegatation mixed hardwood	ls and j	pines				
Landform Ground Morrain	e					
Position on landscape	See at	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	ige Way	>100	feet	
Possible Wet Area	>100	feet Proper	ty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

Deep Observation Hole Log							
Hole # 122-	-3	NB 14/E-31			Suface El. 295.4		
Depth from Surface (inches)	Soil Horizon	Soil Texture (USDA)	Soil Color (MUNSELL)	Soil Mottling	Other (Stucture, Stones, Boulders, Consistency, % Gravel)		
0-12 12-27 27-102	A C1 C2	FSL S FSL	10YR 3/3 10YR 6/4 2.5Y 5/4	@24" 7.5YR 5/6 2.5Y 6/2			
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY P	PROPOSED	DISPOSAL AREA		

Parent Material (geologic) Glacial Till	Depth to Bedrock: >102"	
Depth to Groundwater: Standing Water in the Hole	60" Weeping from Pit Face: 30"	
Estimated Seasonal High Groundwater in the Hole	24"	
Aditional Notes		

Location Address or Lot #: 203 Ayer Road Harvard, MA

# **On-Site Review**

Deep Hole #: 122-4	Date:	01/13/22 Time:		9:00AM	Weather:	Cloudy 27°
Location (identify on site pla	n)	See Attached Ske	etch			
Land Use vacant lot		Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural fie	eld, va	cant lot etc)				
Vegatation mixed hardwoods	s and p	oines				
Landform Ground Morraine	;					
Position on landscape	See at	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	ige Way	>100	feet	
Possible Wet Area	>100	feet Proper	ty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

	Deep Observation Hole Log							
Hole # 122	-4	NB 14/E-31		Suface El. 289.4				
Depth from Surface (inches)	Soil Horizon	Soil Texture (USDA)	Soil Color (MUNSELL)	Soil Mottling	Other (Stucture, Stones, Boulders, Consistency, % Gravel)			
0-12 12-26 26-40 40-50 50-102	A Fill Ab C1 C2	FSL LS FSL FS FSL	10YR 3/3 10YR 6/4 10YR 4/3 2.5Y 6/1 2.5Y 5/4	@45" 7.5YR 5/6 2.5Y 6/2				

Location Address or Lot #: 203 Ayer Road Harvard, MA

# **On-Site Review**

Deep Hole #: 122-5 Date:	01/13/22 Time:	9:30AM	Weather:	Cloudy 27°
Location (identify on site plan)	See Attached Ske	etch		
Land Use vacant lot	Slope (%)	3-8%	Surfaces Stones	few
(eg woodland, agricultural field, v	acant lot etc)			
Vegatation mixed hardwoods and	pines			
Landform Ground Morraine				
Position on landscape See a	ttached Sketch			
Distances from:				
Open Water Body >100	feet Draina	age Way >10	0 feet	
Possible Wet Area >100	feet Proper	rty Line 92	feet	
Drinking Water Well >100	feet Other:			
			feet	

	Deep Observation Hole Log							
Hole # 122-	-5	NB 14/E-31			Suface El. 288.3			
Depth from	Soil	Soil Texture	Soil Color	Soil	Other			
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,			
(inches)					Consistency, % Gravel)			
0-40	Fill	FSL	10YR 3/3					
40-48	C1	S	10YR 5/4					
48-108	C2	FSL	2.5Y 5/4	@50"				
				7.5YR 5/6				
				2.5Y 6/2				
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	ROPOSED	DISPOSAL AREA			
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >108"			

122-5

Location Address or Lot #: 203 Ayer Road Harvard, MA

# **On-Site Review**

Deep Hole #: 122-6 I	Date: 01/13/22 Ti	ime:	10:00AM	Weather:	Cloudy 27°
Location (identify on site plan	) See Attached	l Sketch			
Land Use vacant lot	Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural fiel	ld, vacant lot etc	.)			
Vegatation mixed hardwoods	and pines				
Landform Ground Morraine					
Position on landscape S	See attached Sketc	h			
Distances from:					
Open Water Body >	>100 feet D	rainage Way	>100	feet	
Possible Wet Area >	>100 feet Pr	operty Line	92±	feet	
Drinking Water Well >	>100 feet O	ther:			
				feet	

	Deep Observation Hole Log						
Hole # 122-	-6	NB 14/E-31		Suface El. 281.2			
Depth from	Soil	Soil Texture	Soil Color	Soil	Other		
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,		
(inches)					Consistency, % Gravel)		
0-15	А	FSL	10YR 3/3	@15"			
15-80	C1	FSL	GLEY 3/N	7.5YR 5/8			
80-108	C2	FLS	2.5Y 4/3				
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	PROPOSED	DISPOSAL AREA		
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >108"		
Depth to Ground	dwater: Stand	ling Water in the			eping from Pit Face: 35"		
Estimated Seaso	Estimated Seasonal High Groundwater in the Hole 15"						
Aditional Notes							

Location Address or Lot #: 203 Ayer Road Harvard, MA

# **On-Site Review**

Deep Hole #: 122-7 I	Date: 01/13/22 T	ime:	10:30AM	Weather:	Cloudy 27°
Location (identify on site plan	) See Attache	d Sketch			
Land Use vacant lot	Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural fiel	ld, vacant lot etc.	)			
Vegatation mixed hardwoods	and pines				
Landform Ground Morraine					
Position on landscape S	See attached Sket	ch			
Distances from:					
Open Water Body >	>100 feet E	Drainage Way	>100	feet	
Possible Wet Area >	>100 feet P	roperty Line	92±	feet	
Drinking Water Well >	>100 feet C	Other:			
				feet	

	Deep Observation Hole Log						
Hole # 122-	-7	NB 14/E-31		Suface El. 283.8			
Depth from	Soil	Soil Texture	Soil Color	Soil	Other		
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,		
(inches)					Consistency, % Gravel)		
0-15	А	FSL	7.5YR 2.5/2	@15"			
15-28	C1	S	10YR 4/2	10YR 5/6			
28-99	C2	FSL	GLEY 3/N				
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY H	PROPOSED	DISPOSAL AREA		
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >99"		
Depth to Ground	dwater: Stand	ding Water in the		Wee	eping from Pit Face: 20"		
Estimated Seaso	Estimated Seasonal High Groundwater in the Hole 15"						
Aditional Notes							

Location Address or Lot #: 203 Ayer Road Harvard, MA

# **On-Site Review**

Deep Hole #: 122-8	Date:	01/13/22 Time:		11:00AM	Weather:	Cloudy 27°
Location (identify on site pla	ın)	See Attached Ske	etch		""	
Land Use vacant lot		Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural fi	eld, va	cant lot etc)				
Vegatation mixed hardwood	ls and p	oines				
Landform Ground Morrain	e					
Position on landscape	See at	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	ige Way	>100	feet	
Possible Wet Area	>100	feet Proper	ty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

	Deep Observation Hole Log							
Hole # 122-	-8	NB 14/E-31			Suface El. 292.5			
Depth from	Soil	Soil Texture	Soil Color	Soil	Other			
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,			
(inches)					Consistency, % Gravel)			
0-12	А	FSL	10YR 3/3					
12-28	C1	LS	10YR 5/4					
28-76	C2	SL	2.5Y 5/3	@60"				
				10YR 6/4				
				2.5Y 6/2				
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY I	PROPOSED	DISPOSAL AREA			
Parent Material	(geologic)	Glacial Till		Depth	n to Bedrock: >76"			
Depth to Ground	dwater: Stand	ling Water in the	Hole 72"	Wee	eping from Pit Face: 70"			
Estimated Seaso	onal High Gr	oundwater in the	Hole 60"					

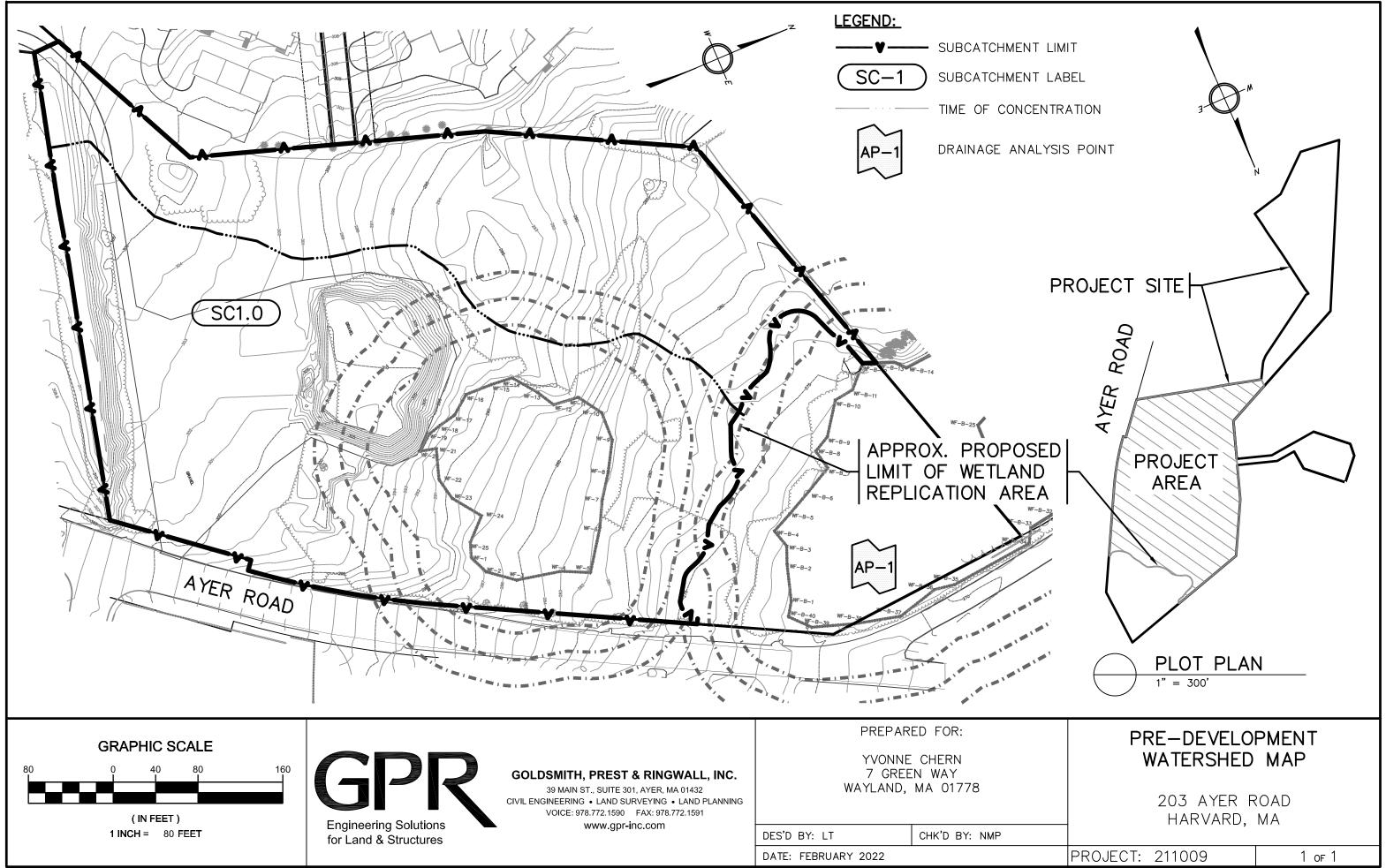
Aditional Notes

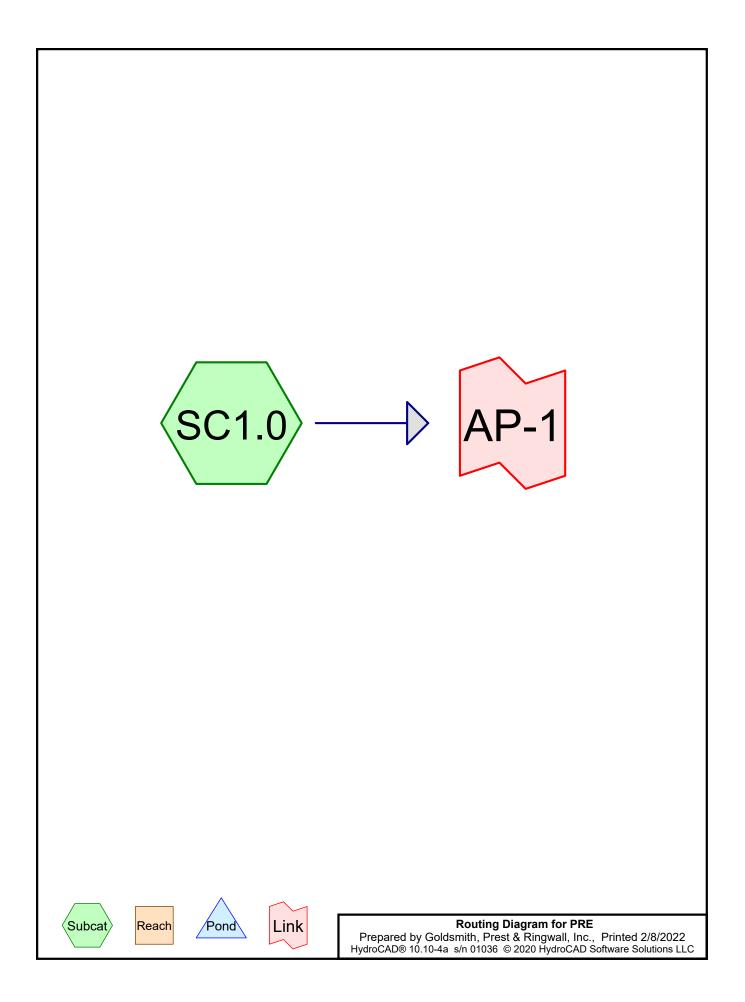
Location Address or Lot#: 203 Ayer Road Harvard, MA

# **Determination for Seasonal High Water Table**

## Method Used:

	Depth observed standing Depth weeping from side		10010101010101010101010	ches	
	Depth to soil mottles Ground water adjustmen	t feet			
Index Wel	Number	Reading Date		Index Well Level	
Adjustmen	t Factor	Adjusted Ground	d Water Level		nan Insender 2017 - Der Angelander
Depth of N	aturally Occuring Pervio	us Material			
	Does at least four feet of	naturally occurin	g nervious mate	rial exist in all areas	andreas and an and a second
	observed throughout the				Yes
	If not, what is the depth of	of naturally occur	ing pervious mat	terial?	Feet
Certificatio	<u>n</u>				
	I certify that I am current pursuant to 310 CMR 15 has been performed by m	.017 to conduct so	oil evaluations a	nd that the above an	alysis
	in 310 CMR 15.017. I fu		e		
	on the attached soil evalu	ation form, are ac	ccurate and in ac	cordance with 310 (	CMR
	15.100 through 15.107.				
	Signature	lan Zi		Date 2/	19/22
Notes:					





#### PRE Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Printed 2/8/2022 Page 2

E	vent#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
	1	2-Year	NRCC 24-hr	D	Default	24.00	1	3.13	2
	2	10-Year	NRCC 24-hr	D	Default	24.00	1	4.68	2
	3	50-Year	NRCC 24-hr	D	Default	24.00	1	7.00	2
	4	100-Year	NRCC 24-hr	D	Default	24.00	1	8.34	2

## **Rainfall Events Listing**

# Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Printed 2/8/2022 Page 3

# Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
183,615	61	>75% Grass cover, Good, HSG B (SC1.0)
52,672	96	Gravel surface, HSG B (SC1.0)
33,694	55	Woods, Good, HSG B (SC1.0)
269,982	67	TOTAL AREA

# PRE

203	B Ayer Road, Harvard, MA
PRE NRCC 24-hr I	D 2-Year Rainfall=3.13"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 2/8/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC	Page 4

Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Link AP-1:

Inflow=2.8 cfs 14,640 cf Primary=2.8 cfs 14,640 cf

Subcatchment SC1.0:

Runoff Area=269,982 sf 0.00% Impervious Runoff Depth=0.65" Flow Length=757' Tc=13.4 min CN=67 Runoff=2.8 cfs 14,640 cf

Total Runoff Area = 269,982 sf Runoff Volume = 14,640 cf Average Runoff Depth = 0.65" 100.00% Pervious = 269,982 sf 0.00% Impervious = 0 sf

# Summary for Link AP-1:

Inflow Area	=	269,982 sf,	0.00% Impervious,	Inflow Depth =	0.65"	for 2-Year event
Inflow	=	2.8 cfs @	12.24 hrs, Volume=	14,640 0	of	
Primary	=	2.8 cfs @	12.24 hrs, Volume=	14,640 d	of, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs

#### Summary for Subcatchment SC1.0:

Runoff = 2.8 cfs @ 12.24 hrs, Volume= 14,640 cf, Depth= 0.65"

A	rea (sf)	CN D	escription					
1	83,615	61 >	61 >75% Grass cover, Good, HSG B					
	33,694		,	od, HSG B				
	52,672	96 G	Gravel surfa	ace, HSG E	3			
2	69,982	67 V	Veighted A	verage				
2	69,982	1	00.00% Pe	ervious Are	a			
_				<b>•</b> •	<b>—</b> • • •			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.3	35	0.1429	0.14		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.13"			
1.0	15	0.1333	0.25		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.13"			
6.5	556	0.0420	1.43		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.2	27	0.1481	1.92		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.4	124	0.0450	1.48		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
13.4	757	Total						

203	Ayer Road, Harvard, MA
PRE NRCC 24-hr D	10-Year Rainfall=4.68"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 2/8/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC	Page 6

Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Link AP-1:

Inflow=7.9 cfs 35,632 cf Primary=7.9 cfs 35,632 cf

Subcatchment SC1.0:

Runoff Area=269,982 sf 0.00% Impervious Runoff Depth=1.58" Flow Length=757' Tc=13.4 min CN=67 Runoff=7.9 cfs 35,632 cf

Total Runoff Area = 269,982 sf Runoff Volume = 35,632 cf Average Runoff Depth = 1.58" 100.00% Pervious = 269,982 sf 0.00% Impervious = 0 sf

# Summary for Link AP-1:

Inflow Area =	269,982 sf,	0.00% Impervious,	Inflow Depth = 1.58	" for 10-Year event
Inflow =	7.9 cfs @	12.22 hrs, Volume=	35,632 cf	
Primary =	7.9 cfs @	12.22 hrs, Volume=	35,632 cf, A	tten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs

#### Summary for Subcatchment SC1.0:

Runoff = 7.9 cfs @ 12.22 hrs, Volume= 35,632 cf, Depth= 1.58"

A	rea (sf)	CN D	escription					
1	83,615	61 >	61 >75% Grass cover, Good, HSG B					
	33,694	55 V	Voods, Go	od, HSG B				
	52,672	<u>96</u>	Gravel surfa	ace, HSG E	3			
2	69,982	67 V	Veighted A	verage				
2	69,982	1	00.00% Pe	ervious Are	а			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.3	35	0.1429	0.14		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.13"			
1.0	15	0.1333	0.25		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.13"			
6.5	556	0.0420	1.43		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.2	27	0.1481	1.92		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.4	124	0.0450	1.48		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
13.4	757	Total						

	203 Ayer Road, Harvard, MA
PRE NRCC 24-hi	D 50-Year Rainfall=7.00"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 2/8/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC	Page 8

Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Link AP-1:

Inflow=17.2 cfs 74,402 cf Primary=17.2 cfs 74,402 cf

Subcatchment SC1.0:

Runoff Area=269,982 sf 0.00% Impervious Runoff Depth=3.31" Flow Length=757' Tc=13.4 min CN=67 Runoff=17.2 cfs 74,402 cf

Total Runoff Area = 269,982 sf Runoff Volume = 74,402 cf Average Runoff Depth = 3.31" 100.00% Pervious = 269,982 sf 0.00% Impervious = 0 sf

# Summary for Link AP-1:

Inflow Area =	269,982 sf,	0.00% Impervious,	Inflow Depth =	3.31"	for 50-Year event
Inflow =	17.2 cfs @	12.22 hrs, Volume=	74,402 c	f	
Primary =	17.2 cfs @	12.22 hrs, Volume=	74,402 ct	f, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs

#### Summary for Subcatchment SC1.0:

Runoff = 17.2 cfs @ 12.22 hrs, Volume= 74,402 cf, Depth= 3.31"

A	rea (sf)	CN D	escription					
1	83,615	61 >	61 >75% Grass cover, Good, HSG B					
	33,694	55 V	Voods, Go	od, HSG B				
	52,672	<u>96</u>	Gravel surfa	ace, HSG E	3			
2	69,982	67 V	Veighted A	verage				
2	69,982	1	00.00% Pe	ervious Are	а			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.3	35	0.1429	0.14		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.13"			
1.0	15	0.1333	0.25		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.13"			
6.5	556	0.0420	1.43		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.2	27	0.1481	1.92		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.4	124	0.0450	1.48		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
13.4	757	Total						

	203 Ayer Road, Harvard, MA
PRE	NRCC 24-hr D 100-Year Rainfall=8.34"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 2/8/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solu	tions LLC Page 10

Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Link AP-1:

Inflow=22.9 cfs 99,106 cf Primary=22.9 cfs 99,106 cf

Subcatchment SC1.0:

Runoff Area=269,982 sf 0.00% Impervious Runoff Depth=4.41" Flow Length=757' Tc=13.4 min CN=67 Runoff=22.9 cfs 99,106 cf

Total Runoff Area = 269,982 sf Runoff Volume = 99,106 cf Average Runoff Depth = 4.41" 100.00% Pervious = 269,982 sf 0.00% Impervious = 0 sf

## Summary for Link AP-1:

 Inflow Area =
 269,982 sf, 0.00% Impervious, Inflow Depth = 4.41" for 100-Year event

 Inflow =
 22.9 cfs @ 12.21 hrs, Volume=
 99,106 cf

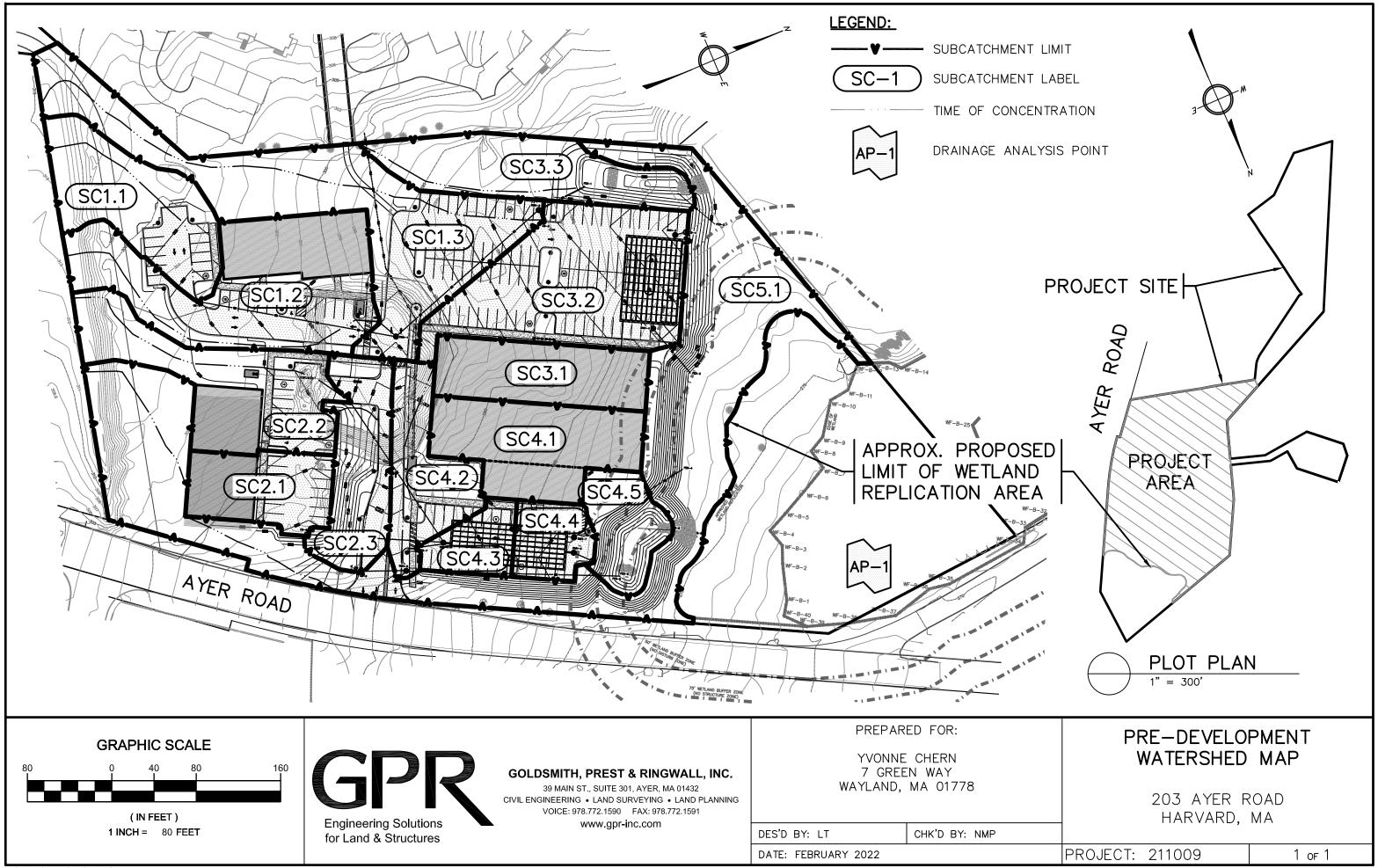
 Primary =
 22.9 cfs @ 12.21 hrs, Volume=
 99,106 cf, Atten= 0%, Lag= 0.0 min

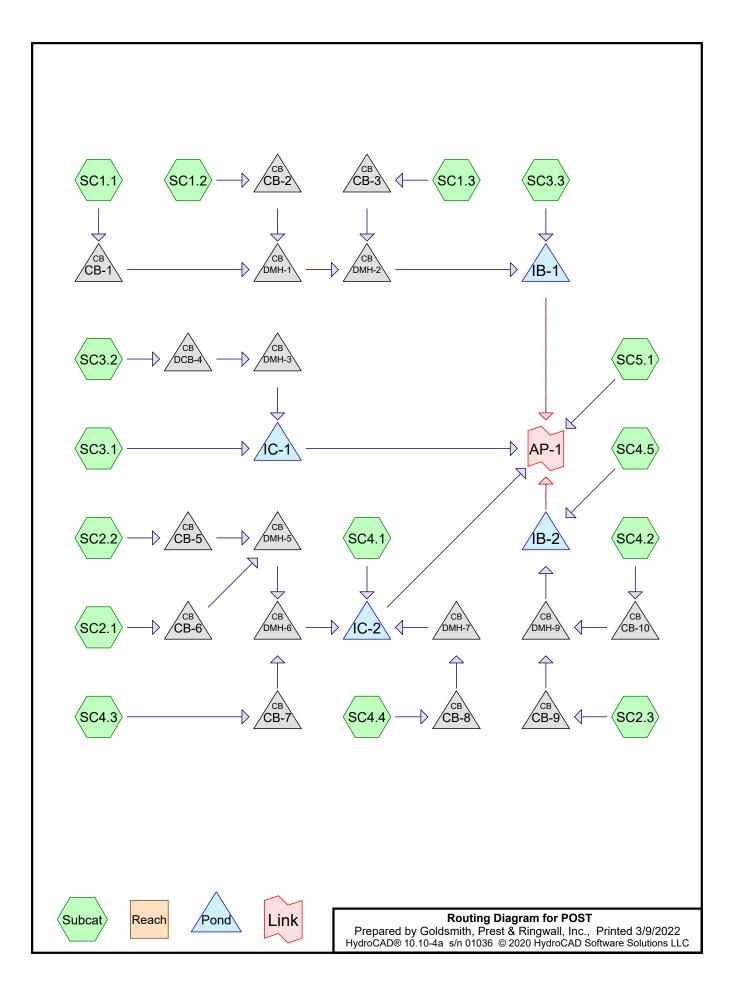
Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs

#### Summary for Subcatchment SC1.0:

Runoff = 22.9 cfs @ 12.21 hrs, Volume= 99,106 cf, Depth= 4.41"

Α	rea (sf)	CN D	escription					
1	83,615	61 >	61 >75% Grass cover, Good, HSG B					
	33,694	55 V	Voods, Go	od, HSG B				
	52,672	96 G	Gravel surfa	ace, HSG E	3			
2	69,982	67 V	Veighted A	verage				
2	69,982	1	00.00% Pe	ervious Are	а			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.3	35	0.1429	0.14		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.13"			
1.0	15	0.1333	0.25		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.13"			
6.5	556	0.0420	1.43		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.2	27	0.1481	1.92		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.4	124	0.0450	1.48		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
13.4	757	Total						





#### POST Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Printed 3/9/2022 Page 2

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	NRCC 24-hr	D	Default	24.00	1	3.13	2
2	10-Year	NRCC 24-hr	D	Default	24.00	1	4.68	2
3	50-Year	NRCC 24-hr	D	Default	24.00	1	7.00	2
4	100-Year	NRCC 24-hr	D	Default	24.00	1	8.34	2

# **Rainfall Events Listing**

POST
Prepared by Goldsmith, Prest & Ringwall, Inc.
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Printed 3/9/2022 Page 3

# Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
119,677	61	>75% Grass cover, Good, HSG B (SC1.1, SC1.2, SC1.3, SC2.1, SC2.2, SC2.3,
		SC3.1, SC3.2, SC3.3, SC4.2, SC4.3, SC4.4, SC4.5, SC5.1)
9,151	96	Gravel surface, HSG B (SC1.1, SC1.2, SC1.3, SC2.2, SC3.3, SC5.1)
71,721	98	Paved parking, HSG B (SC1.1, SC1.2, SC1.3, SC2.1, SC2.2, SC2.3, SC3.2,
		SC4.2, SC4.3, SC4.4, SC5.1)
42,613	98	Roofs, HSG B (SC1.2, SC2.1, SC2.2, SC3.1, SC4.1)
7,036	98	Unconnected pavement, HSG B (SC1.1, SC1.2, SC1.3, SC2.1, SC2.2, SC2.3,
		SC3.2, SC4.2, SC4.3, SC4.4, SC5.1)
19,783	55	Woods, Good, HSG B (SC1.1, SC1.2, SC1.3, SC2.2, SC5.1)
269,982	78	TOTAL AREA

2	03 Ayer Road, Harvard, MA
POST NRCC 24-h	r D <sup>2</sup> -Year Rainfall=3.13"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 3/9/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC	Page 4

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>0.83" Flow Length=233' Tc=5.8 min CN=71 Runoff=0.3 cfs 1,058 cf
SubcatchmentSC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>1.69" Flow Length=335' Tc=9.0 min CN=85 Runoff=1.0 cfs 3,732 cf
SubcatchmentSC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>1.10" Flow Length=520' Tc=13.0 min CN=76 Runoff=0.7 cfs 3,064 cf
Subcatchment SC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>2.68" Tc=5.0 min CN=96 Runoff=0.6 cfs 2,071 cf
Subcatchment SC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>1.41" Flow Length=295' Tc=7.8 min CN=81 Runoff=0.6 cfs 2,182 cf
SubcatchmentSC2.3:	Runoff Area=11,540 sf   37.45% Impervious   Runoff Depth>1.05" Flow Length=185'   Tc=5.0 min   CN=75   Runoff=0.3 cfs  1,005 cf
SubcatchmentSC3.1:	Runoff Area=11,615 sf 100.00% Impervious Runoff Depth>2.90" Tc=5.0 min CN=98 Runoff=0.7 cfs 2,803 cf
SubcatchmentSC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>2.57" Flow Length=118' Slope=0.0380 '/' Tc=5.2 min CN=95 Runoff=1.5 cfs 5,508 cf
SubcatchmentSC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>0.53" Flow Length=216' Tc=5.6 min CN=64 Runoff=0.2 cfs 642 cf
SubcatchmentSC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>2.90" Tc=5.0 min CN=98 Runoff=1.0 cfs 3,619 cf
SubcatchmentSC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>1.35" Flow Length=231' Tc=5.0 min CN=80 Runoff=0.4 cfs 1,176 cf
SubcatchmentSC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>2.57" Tc=5.0 min CN=95 Runoff=0.3 cfs 1,004 cf
SubcatchmentSC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>2.47" Tc=5.0 min CN=94 Runoff=0.3 cfs 1,059 cf
SubcatchmentSC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>0.41" Tc=5.0 min CN=61 Runoff=0.0 cfs 221 cf
SubcatchmentSC5.1:	Runoff Area=61,802 sf 2.47% Impervious Runoff Depth>0.49" Flow Length=362' Tc=10.0 min CN=63 Runoff=0.5 cfs 2,502 cf
Pond CB-1:	Peak Elev=297.80' Inflow=0.3 cfs 1,058 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=0.3 cfs 1,058 cf

<b>POST</b> Prepared by Goldsmith, Pres HydroCAD® 10.10-4a_s/n 01036	203 Ayer Road, Harvard, MA NRCC 24-hr D 2-Year Rainfall=3.13" at & Ringwall, Inc. Printed 3/9/2022 © 2020 HydroCAD Software Solutions LLC Page 5
Pond CB-10:	Peak Elev=285.35' Inflow=0.4 cfs 1,176 cf 12.0" Round Culvert n=0.013 L=21.0' S=0.0100 '/' Outflow=0.4 cfs 1,176 cf
Pond CB-2:	Peak Elev=294.37' Inflow=1.0 cfs 3,732 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0200 '/' Outflow=1.0 cfs 3,732 cf
Pond CB-3:	Peak Elev=288.01' Inflow=0.7 cfs 3,064 cf 12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/' Outflow=0.7 cfs 3,064 cf
Pond CB-5:	Peak Elev=294.45' Inflow=0.6 cfs 2,182 cf 12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/' Outflow=0.6 cfs 2,182 cf
Pond CB-6:	Peak Elev=295.93' Inflow=0.6 cfs 2,071 cf 12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/' Outflow=0.6 cfs 2,071 cf
Pond CB-7:	Peak Elev=288.55' Inflow=0.3 cfs 1,004 cf 12.0" Round Culvert n=0.013 L=22.0' S=0.0136 '/' Outflow=0.3 cfs 1,004 cf
Pond CB-8:	Peak Elev=288.81' Inflow=0.3 cfs 1,059 cf 12.0" Round Culvert n=0.013 L=15.0' S=0.0200 '/' Outflow=0.3 cfs 1,059 cf
Pond CB-9:	Peak Elev=287.30' Inflow=0.3 cfs 1,005 cf 12.0" Round Culvert n=0.013 L=69.0' S=0.0319 '/' Outflow=0.3 cfs 1,005 cf
Pond DCB-4:	Peak Elev=285.28' Inflow=1.5 cfs 5,508 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/' Outflow=1.5 cfs 5,508 cf
Pond DMH-1:	Peak Elev=294.15' Inflow=1.3 cfs 4,790 cf 12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/' Outflow=1.3 cfs 4,790 cf
Pond DMH-2:	Peak Elev=287.62' Inflow=1.9 cfs 7,855 cf 18.0" Round Culvert n=0.013 L=48.0' S=0.0100 '/' Outflow=1.9 cfs 7,855 cf
Pond DMH-3:	Peak Elev=284.74' Inflow=1.5 cfs 5,508 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=1.5 cfs 5,508 cf
Pond DMH-5:	Peak Elev=289.65' Inflow=1.2 cfs  4,253 cf 12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/' Outflow=1.2 cfs  4,253 cf
Pond DMH-6:	Peak Elev=288.55' Inflow=1.4 cfs 5,258 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=1.4 cfs 5,258 cf
Pond DMH-7:	Peak Elev=288.54' Inflow=0.3 cfs 1,059 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=0.3 cfs 1,059 cf
Pond DMH-9:	Peak Elev=285.13' Inflow=0.7 cfs 2,182 cf 15.0" Round Culvert n=0.013 L=185.0' S=0.0146 '/' Outflow=0.7 cfs 2,182 cf
Pond IB-1: Discarded=0.0 cfs 661 cf	Peak Elev=285.59' Storage=916 cf Inflow=2.1 cfs 8,497 cf Primary=1.2 cfs 7,421 cf Secondary=0.5 cfs 204 cf Outflow=1.7 cfs 8,287 cf

<b>POST</b> Prepared by Goldsmith, Prest HydroCAD® 10.10-4a s/n 01036 (	NRCC 24-hr D 2-	r Road, Harvard, MA Y <i>ear Rainfall=3.13"</i> Printed 3/9/2022 <u>Page 6</u>
Pond IB-2:	Peak Elev=282.76' Storage=764 cf II	nflow=0.7 cfs 2,402 cf
	Primary=0.2 cfs 1,840 cf Secondary=0.0 cfs 0 cf Ou	utflow=0.2 cfs 1,840 cf
Pond IC-1:	Peak Elev=284.74' Storage=3,540 cf II	nflow=2.3 cfs 8,310 cf
	Discarded=0.1 cfs 6,387 cf Primary=0.0 cfs 0 cf Ou	utflow=0.1 cfs 6,387 cf
Pond IC-2:	Peak Elev=288.55' Storage=4,577 cf II	nflow=2.7 cfs 9,936 cf
	Discarded=0.1 cfs 6,355 cf Primary=0.0 cfs 130 cf Ou	utflow=0.1 cfs 6,485 cf
Link AP-1:	Int	flow=2.2 cfs 12,098 cf
	Prim	nary=2.2 cfs 12,098 cf
Tatal David (Charac	- 000 000 - ( . D	

### Total Runoff Area = 269,982 sf Runoff Volume = 31,647 cf Average Runoff Depth = 1.41" 55.04% Pervious = 148,611 sf 44.96% Impervious = 121,371 sf

## Summary for Subcatchment SC1.1:

Runoff = 0.3 cfs @ 12.14 hrs, Volume= 1,058 cf,	, Depth>	0.83"
---	----------	-------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN [	Description				
	4,381	61 >	61 >75% Grass cover, Good, HSG B				
	5,583	55 \	Voods, Go	od, HSG B			
	328	98 l	Jnconnecte	ed pavemer	nt, HSG B		
	3,654	98 F	Paved park	ing, HSG B	5		
	1,269	96 (	Gravel surfa	ace, HSG E	3		
	15,215	71 \	Veighted A	verage			
	11,233	7	'3.83% Pei	vious Area			
	3,982	2	26.17% Imp	pervious Are	ea		
	328	8	8.23% Unco	onnected			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
3.4	34	0.2400	0.17		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.12"		
0.9	16	0.1875	0.28		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.12"		
1.0	83	0.0361	1.33		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
0.5	100	0.0300	3.52		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
5.8	233	Total					

#### **Summary for Subcatchment SC1.2:**

Runoff = 1.0 cfs @ 12.16 hrs, Volume= 3,732 cf, Depth> 1.69"

Area (sf)	CN	Description
6,620	61	>75% Grass cover, Good, HSG B
2,398	55	Woods, Good, HSG B
1,172	98	Unconnected pavement, HSG B
8,000	98	Roofs, HSG B
7,203	98	Paved parking, HSG B
1,052	96	Gravel surface, HSG B
26,444	85	Weighted Average
10,069		38.08% Pervious Area
16,375		61.92% Impervious Area
1,172		7.16% Unconnected

## POST

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Page 8

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.9	41	0.2400	0.17		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
1.4	9	0.0200	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
3.4	222	0.0248	1.10		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	63	0.0300	3.52		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
9.0	335	Total			

# Summary for Subcatchment SC1.3:

Runoff	=	0.7 cfs @	12.22 hrs,	Volume=	3,064 cf,	Depth>	1.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN E	Description					
	15,029	61 >	>75% Grass cover, Good, HSG B					
	3,894	55 V	Voods, Go	od, HSG B				
	1,741	96 C	Gravel surfa	ace, HSG E	3			
	275	98 l	Jnconnecte	ed pavemer	nt, HSG B			
	12,548	98 F	Paved park	ing, HSG B	3			
	33,487	76 V	Veighted A	verage				
	20,664	6	61.71% Per	vious Area				
	12,823	3	8.29% Imp	ervious Ar	ea			
	275	2	2.14% Unco	onnected				
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.2	50	0.0800	0.12		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.12"			
3.2	188	0.0372	0.96		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
2.0	160	0.0372	1.35		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.6	122	0.0300	3.52		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
13.0	520	Total						

## **Summary for Subcatchment SC2.1:**

Runoff	=	0.6 cfs @	12.11 hrs, Volume=	2,071 cf, Depth> 2.68"
--------	---	-----------	--------------------	------------------------

POST

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description						
	595	61	>75% Gras	s cover, Go	bood, HSG B				
	423	98	Unconnecte	ed pavemer	ent, HSG B				
	4,000	98	Roofs, HSG	B					
	4,263	98	Paved park	ing, HSG B	В				
	9,281	96	Weighted A	verage					
	595		6.41% Perv	ious Area					
	8,686		93.59% Imp	pervious Ar	rea				
	423		4.87% Unco	onnected					
-				0					
Tc	Length	Slop		Capacity	I				
<u>(min)</u>	(feet)	(ft/f	t) (ft/sec)	(cfs)					
5.0					Direct Entry,				

#### **Summary for Subcatchment SC2.2:**

Runoff	=	0.6 cfs @	12.15 hrs,	Volume=	2,182 cf, Depth> 1.41"
--------	---	-----------	------------	---------	------------------------

A	rea (sf)	CN D	escription					
	5,529		61 >75% Grass cover, Good, HSG B					
	2,637		,	od, HSG B				
	889			ed pavemer	nt, HSG B			
	4,000		loofs, HSG					
	4,580			ing, HSG B				
	929	96 G	Gravel surfa	ace, HSG E	}			
	18,563	81 V	Veighted A	verage				
	9,094	4	8.99% Per	vious Area				
	9,469	5	1.01% Imp	pervious Ar	ea			
	889	9	.39% Unco	onnected				
_								
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.6	50	0.2400	0.18		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.12"			
0.2	8	0.0200	0.71		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
2.5	151	0.0200	0.99		Shallow Concentrated Flow,			
	_				Short Grass Pasture Kv= 7.0 fps			
0.5	86	0.0200	2.87		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
7.8	295	Total						

#### Summary for Subcatchment SC2.3:

Runoff = 0.3 cfs @ 12.12 hrs, Volume= 1,005 cf, Depth> 1.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Area (sf)	CN [	Description		
7,218	61 >	>75% Gras	s cover, Go	bod, HSG B
293	98 l	Jnconnecte	ed pavemei	nt, HSG B
4,029	98 F	Paved park	ing, HSG E	}
11,540	75 \	Veighted A	verage	
7,218	6	32.55% Per	vious Area	
4,321			pervious Ar	ea
293	6	6.77% Unco	onnected	
	<b>.</b> .			
Tc Length	Slope	Velocity	Capacity	Description
(min) (feet)	(ft/ft)	(ft/sec)	(cfs)	
2.3 42	0.1430	0.31		Sheet Flow,
				Grass: Short n= 0.150 P2= 3.12"
0.1 8	0.0500	1.18		Sheet Flow,
				Smooth surfaces n= 0.011 P2= 3.12"
0.5 135	0.0500	4.54		Shallow Concentrated Flow,
				Paved Kv= 20.3 fps
2.9 185	Total,	Increased t	o minimum	1 Tc = 5.0 min

#### Summary for Subcatchment SC3.1:

Runoff = 0.7 cfs @ 12.11 hrs, Volume= 2,803 cf, Depth> 2.90"

Ar	ea (sf)	CN	Description						
	11,615	98	Roofs, HSG	в					
	0	61	>75% Grass	s cover, Go	bod, HSG B				
	11,615	98	Weighted A	Weighted Average					
	0		0.00% Perv	ious Area					
	11,615		100.00% Im	pervious A	Area				
Тс	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
5.0					Direct Entry,				

#### Summary for Subcatchment SC3.2:

Runoff = 1.5 cfs @ 12.11 hrs, Volume= 5,508 cf, Depth> 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN E	Description		
	2,372	61 >	75% Gras	s cover, Go	bod, HSG B
	21,883	98 F	Paved park	ing, HSG B	
	1,422	98 L	Inconnecte	ed pavemer	nt, HSG B
	25,677	95 V	Veighted A	verage	
	2,372	ç	.24% Perv	ious Area	
	23,305	ç	0.76% Imp	pervious Ar	ea
	1,422	6	6.10% Unco	onnected	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.4	50	0.0380	0.19		Sheet Flow,
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
5.2	118	Total			

## Summary for Subcatchment SC3.3:

Runoff = 0.2 cfs @ 12.14 hrs, Volume=

642 cf, Depth> 0.53"

_	A	rea (sf)	CN [	Description					
		13,540			,	ood, HSG B			
_		1,127	96 (	Gravel surfa	ace, HSG E	}			
		14,666	64 \	Neighted A	verage				
		14,666		100.00% Pe	ervious Are	а			
	_		-		<b>-</b>				
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.7	50	0.0600	0.23		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.12"			
	1.9	166	0.0422	1.44		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	5.6	216	Total						

#### Summary for Subcatchment SC4.1:

Runoff = 1.0 cfs @ 12.11 hrs, Volume= 3,619 cf, Depth> 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Area (sf)	CN	Description					
14,998	98	Roofs, HSG	В				
14,998		100.00% Impervious Area					
Tc Length (min) (feet) 5.0	Slope (ft/ft)	,	Capacity (cfs)	Description Direct Entry,			

#### Summary for Subcatchment SC4.2:

Runoff = 0.4 cfs @ 12.12 hrs, Volume= 1,176 cf, Depth> 1.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Area (s	sf) (	CN D	escription		
5,06	50	61 >7	75% Grass	s cover, Go	bod, HSG B
1,45	50	98 U	nconnecte	ed pavemer	nt, HSG B
3,97	75	98 Pa	aved parki	ing, HSG B	
10,48	34	80 W	/eighted A	verage	
5,06	50	48	3.26% Per	vious Area	
5,42	24	5	1.74% Imp	ervious Are	ea
1,45	50	26	6.72% Uno	connected	
Tc Len	gth	Slope	Velocity	Capacity	Description
<u>(min)</u> (fe	et)	(ft/ft)	(ft/sec)	(cfs)	
4.0	50 0	0.0500	0.21		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.4	33 0	0.0500	1.57		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.6 1	148 0	).0440	4.26		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
5.0 2	231 T	otal			

#### Summary for Subcatchment SC4.3:

Runoff = 0.3 cfs @ 12.11 hrs, Volume= 1,004 cf, Depth> 2.57"

POST

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description						
	406	61	>75% Gras	s cover, Go	ood, HSG B				
	175	98	Unconnecte	ed pavemer	ent, HSG B				
	4,101	98	Paved park	ing, HSG B	B				
	4,682	95	Weighted A	verage					
	406		8.68% Perv	ious Area					
	4,276		91.32% Imp	pervious Ar	rea				
	175		4.09% Unc	onnected					
Тс	Length	Slop	,	Capacity	1				
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
5.0					Direct Entry,				

## Summary for Subcatchment SC4.4:

Runoff = 0.3 cfs @ 12.11 hrs, Volume= 1,059 cf, Depth> 2.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Α	rea (sf)	CN I	Description						
	564	61 :	>75% Gras	s cover, Go	ood, HSG B				
	541	98	Unconnected pavement, HSG B						
	4,030	98	Paved parking, HSG B						
	5,136	94	94 Weighted Average						
	564	564 10.99% Pervious Area							
	4,571	89.01% Impervious Area							
	541	·	11.83% Un	connected					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)					
5.0					Direct Entry,				

#### Summary for Subcatchment SC4.5:

Runoff = 0.0 cfs @ 12.14 hrs, Volume= 221 cf, Depth> 0.41"

A	rea (sf)	CN D	Description							
	6,391	61 >	>75% Grass cover, Good, HSG B							
	6,391	100.00% Pervious Area								
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

# Summary for Subcatchment SC5.1:

Runoff = 0.5 cfs @ 12.20 hrs, Volume= 2,502 cf,	Depth> 0.49"
---	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN D	escription		
	51,973	61 >75% Grass cover, Good, HSG B			
	5,271			od, HSG B	
	70			ed pavemer	
	1,456			ing, HSG B	
	3,033			ace, HSG B	
	61,802 60,277		/eighted A	verage vious Area	
	1,525			ervious Area	
	70		.56% Unco		
	10		.0070 01100	Sinteolog	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
3.8	36	0.2000	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
2.1	14	0.0200	0.11		Sheet Flow,
	0.40		4.00		Grass: Short n= 0.150 P2= 3.12"
4.1	312	0.0334	1.28		Shallow Concentrated Flow,
40.0	000	<b>T</b> . 4 . 1			Short Grass Pasture Kv= 7.0 fps
10.0	362	Total			
				Summar	y for Pond CB-1:
				Gammar	
Inflow Ar	ea =	15 2 <sup>-</sup>	15 sf 26 <sup>-</sup>	17% Imperv	/ious, Inflow Depth > 0.83" for 2-Year event
Inflow	=			4 hrs, Volu	
Outflow	=			4 hrs, Volu	
Primary	=	0.3 cf	ˈs @ 12.1	4 hrs, Volu	ume= 1,058 cf
				_	
				ie Span= 0.	00-24.00 hrs, dt= 0.05 hrs / 2
		0'@12.1	4 nrs		
FIOOD EI	ev= 301.5	50'			
Device	Routing		Invert O	utlet Device	es
#1	Primary	2	97.50' <b>1</b> 2	2.0" Roun	d Culvert
	-				PP, projecting, no headwall, Ke= 0.900
					Invert= 297.50' / 293.56' S= 0.0232 '/' Cc= 0.900
			n	= 0.013 Cc	prrugated PE, smooth interior, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.3 cfs @ 12.14 hrs HW=297.80' TW=294.13' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.3 cfs @ 1.46 fps)

# Summary for Pond CB-10:

Inflow Area =	10,484 sf, 51.7	74% Impervious,	Inflow Depth > 1.35" for 2-Year event
Inflow =	0.4 cfs @ 12.1	12 hrs, Volume=	1,176 cf
Outflow =	0.4 cfs @ 12.1	12 hrs, Volume=	1,176 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.4 cfs @ 12.1	12 hrs, Volume=	1,176 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.35' @ 12.12 hrs Flood Elev= 289.00'

Device	Routing	Invert	Outlet Devices
#1	Primary		12.0" Round Culvert
			L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 285.00' / 284.79' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.3 cfs @ 12.12 hrs HW=285.35' TW=285.12' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.3 cfs @ 2.13 fps)

# Summary for Pond CB-2:

Inflow Area =	26,444 sf, 61.92% Impervious	s, Inflow Depth > 1.69" for 2-Year event
Inflow =	1.0 cfs @ 12.16 hrs, Volume	= 3,732 cf
Outflow =	1.0 cfs @ 12.16 hrs, Volume	= 3,732 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.0 cfs @ 12.16 hrs, Volume	= 3,732 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.37' @ 12.16 hrs Flood Elev= 297.70'

<u>Device</u> R	Routing	Invert	Outlet Devices
#1 P	Primary		<b>12.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.0 cfs @ 12.16 hrs HW=294.36' TW=294.14' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.0 cfs @ 1.79 fps)

# Summary for Pond CB-3:

Inflow Area	a =	33,487 sf,	38.29% Impervious,	Inflow Depth > 1.10" for 2-Year event
Inflow	=	0.7 cfs @	12.22 hrs, Volume=	3,064 cf
Outflow	=	0.7 cfs @	12.22 hrs, Volume=	3,064 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.7 cfs @	12.22 hrs, Volume=	3,064 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

# POST NRCC 24-hr D Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Peak Elev= 288.01' @ 12.22 hrs Flood Elev= 291.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.50'	<b>12.0" Round Culvert</b> L= 8.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.7 cfs @ 12.22 hrs HW=288.00' TW=287.58' (Dynamic Tailwater) ☐ 1=Culvert (Barrel Controls 0.7 cfs @ 2.55 fps)

# Summary for Pond CB-5:

Inflow Area =	18,563 sf, 51.01% Impervious,	Inflow Depth > 1.41" for 2-Year event
Inflow =	0.6 cfs @ 12.15 hrs, Volume=	2,182 cf
Outflow =	0.6 cfs @ 12.15 hrs, Volume=	2,182 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.6 cfs $\overline{@}$ 12.15 hrs, Volume=	2,182 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.45' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert
			L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.6 cfs @ 12.15 hrs HW=294.45' TW=289.63' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.6 cfs @ 1.80 fps)

# Summary for Pond CB-6:

Inflow Area =	= 9,281 sf,	93.59% Impervious,	Inflow Depth > 2.68"	for 2-Year event
Inflow =	0.6 cfs @	12.11 hrs, Volume=	2,071 cf	
Outflow =	0.6 cfs @	12.11 hrs, Volume=	2,071 cf, Atte	en= 0%, Lag= 0.0 min
Primary =	0.6 cfs @	12.11 hrs, Volume=	2,071 cf	2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 295.93' @ 12.11 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	<b>12.0" Round Culvert</b> L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.6 cfs @ 12.11 hrs HW=295.93' TW=289.62' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 0.6 cfs @ 1.75 fps)

# Summary for Pond CB-7:

Inflow Area	a =	4,682 sf,	91.32% Impervious,	Inflow Depth > 2.57" for 2-Year event	
Inflow	=	0.3 cfs @	12.11 hrs, Volume=	1,004 cf	
Outflow	=	0.3 cfs @	12.11 hrs, Volume=	1,004 cf, Atten= 0%, Lag= 0.0 mi	n
Primary	=	0.3 cfs @	12.11 hrs, Volume=	1,004 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.55' @ 14.99 hrs Flood Elev= 292.00'

Device	Routing	Invert	Outlet Devices
<u></u> #1	Primary		<b>12.0" Round Culvert</b> L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
			II- 0.013 Conducted FL, shooth interior, Thow Area- 0.79 st

**Primary OutFlow** Max=0.3 cfs @ 12.11 hrs HW=288.34' TW=288.17' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.3 cfs @ 1.78 fps)

# Summary for Pond CB-8:

Inflow Area :	= 5,13	6 sf, 89.01% Imperviou	s, Inflow Depth > 2.47	for 2-Year event
Inflow =	= 0.3 cfs	@ 12.11 hrs, Volume	= 1,059 cf	
Outflow =	= 0.3 cfs	🧔 12.11 hrs, Volume	= 1,059 cf, At	tten= 0%, Lag= 0.0 min
Primary =	= 0.3 cfs	@ 12.11 hrs, Volume	= 1,059 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.81' @ 12.11 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices	
<u></u> #1	Primary		<b>12.0" Round Culvert</b> L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=0.3 cfs @ 12.11 hrs HW=288.80' TW=287.92' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.3 cfs @ 1.48 fps)

# Summary for Pond CB-9:

Inflow Area =	11,540 sf,	37.45% Impervious,	Inflow Depth > 1.05" for 2-Year event
Inflow =	0.3 cfs @	12.12 hrs, Volume=	1,005 cf
Outflow =	0.3 cfs @	12.12 hrs, Volume=	1,005 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.3 cfs @	12.12 hrs, Volume=	1,005 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.30' @ 12.12 hrs Flood Elev= 291.00'

#1 Primary 287.00' <b>12.0'' Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.00' / 284.80' S= 0.0319 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=0.3 cfs @ 12.12 hrs HW=287.30' TW=285.12' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.3 cfs @ 1.46 fps)

# Summary for Pond DCB-4:

Inflow Area	=	25,677 sf,	90.76% Impervious,	Inflow Depth > 2.57	for 2-Year event
Inflow	=	1.5 cfs @	12.11 hrs, Volume=	5,508 cf	
Outflow	=	1.5 cfs @	12.11 hrs, Volume=	5,508 cf, At	tten= 0%, Lag= 0.0 min
Primary	=	1.5 cfs @	12.11 hrs, Volume=	5,508 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.28' @ 12.11 hrs Flood Elev= 288.50'

Device Routing Invert Outlet Devices	
#1 Primary 284.50' <b>12.0'' Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' C n= 0.013 Corrugated PE, smooth interior, Flow Area=	

Primary OutFlow Max=1.5 cfs @ 12.11 hrs HW=285.26' TW=284.70' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 1.5 cfs @ 2.34 fps)

# Summary for Pond DMH-1:

Inflow Area	a =	41,659 sf,	48.86% Impervious,	Inflow Depth > 1.38" for 2-Year event
Inflow	=	1.3 cfs @	12.15 hrs, Volume=	4,790 cf
Outflow	=	1.3 cfs @	12.15 hrs, Volume=	4,790 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.3 cfs @	12.15 hrs, Volume=	4,790 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

# 203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D2-Year Rainfall=3.13"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 19

Peak Elev= 294.15' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	<b>12.0" Round Culvert</b> L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.3 cfs @ 12.15 hrs HW=294.15' TW=287.61' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 1.3 cfs @ 2.23 fps)

#### Summary for Pond DMH-2:

Inflow Area	a =	75,146 sf,	44.15% Impervious,	Inflow Depth > 1.25"	for 2-Year event
Inflow	=	1.9 cfs @	12.17 hrs, Volume=	7,855 cf	
Outflow	=	1.9 cfs @	12.17 hrs, Volume=	7,855 cf, Atte	en= 0%, Lag= 0.0 min
Primary	=	1.9 cfs @	12.17 hrs, Volume=	7,855 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.62' @ 12.17 hrs Flood Elev= 291.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.90'	18.0" Round Culvert
			L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

**Primary OutFlow** Max=1.9 cfs @ 12.17 hrs HW=287.61' TW=285.46' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.9 cfs @ 2.26 fps)

# **Summary for Pond DMH-3:**

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth > 2.	57" for 2-Year event
Inflow	=	1.5 cfs @	12.11 hrs, Volume=	5,508 cf	
Outflow	=	1.5 cfs @	12.11 hrs, Volume=	5,508 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.5 cfs @	12.11 hrs, Volume=	5,508 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 284.74' @ 14.66 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.10'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.5 cfs @ 12.11 hrs HW=284.70' TW=284.26' (Dynamic Tailwater) ↓ 1=Culvert (Barrel Controls 1.5 cfs @ 2.81 fps)

# Summary for Pond DMH-5:

Inflow Area	a =	27,844 sf,	65.20% Impervious,	Inflow Depth > 1.83" for 2-Year event
Inflow	=	1.2 cfs @	12.13 hrs, Volume=	4,253 cf
Outflow	=	1.2 cfs @	12.13 hrs, Volume=	4,253 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.2 cfs @	12.13 hrs, Volume=	4,253 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.65' @ 12.13 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	289.00'	<b>12.0" Round Culvert</b> L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.1 cfs @ 12.13 hrs HW=289.63' TW=288.18' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.1 cfs @ 2.13 fps)

# Summary for Pond DMH-6:

Inflow Area	ı =	32,526 sf,	68.96% Impervious,	Inflow Depth > 1.94" f	or 2-Year event
Inflow	=	1.4 cfs @	12.13 hrs, Volume=	5,258 cf	
Outflow	=	1.4 cfs @	12.13 hrs, Volume=	5,258 cf, Atten=	= 0%, Lag= 0.0 min
Primary	=	1.4 cfs @	12.13 hrs, Volume=	5,258 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.55' @ 14.94 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	<b>24.0" Round Culvert</b> L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf
Primary	/ OutFlow	Max=1.4 cfs @	12.13 hrs HW=288.18' TW=287.88' (Dynamic Tailwater)

**1=Culvert** (Barrel Controls 1.4 cfs @ 2.77 fps)

203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D2-Year Rainfall=3.13"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 21

# Summary for Pond DMH-7:

Inflow Area =	5,136 sf, 89.01% Impervious,	Inflow Depth > 2.47" for 2-Year event
Inflow =	0.3 cfs @ 12.11 hrs, Volume=	1,059 cf
Outflow =	0.3 cfs @ 12.11 hrs, Volume=	1,059 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.3 cfs @ 12.11 hrs, Volume=	1,059 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.54' @ 14.76 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
<u></u> #1	Primary		<b>24.0" Round Culvert</b> L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.3 cfs @ 12.11 hrs HW=287.92' TW=287.84' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.3 cfs @ 1.46 fps)

#### Summary for Pond DMH-9:

Inflow Area =	22,024 sf,	44.25% Impervious,	Inflow Depth > 1.19" for 2-Year event
Inflow =	0.7 cfs @	12.12 hrs, Volume=	2,182 cf
Outflow =	0.7 cfs @	12.12 hrs, Volume=	2,182 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.7 cfs @	12.12 hrs, Volume=	2,182 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.13' @ 12.12 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.70'	<b>15.0" Round Culvert</b> L= 185.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.70' / 282.00' S= 0.0146 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.6 cfs @ 12.12 hrs HW=285.12' TW=282.55' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.6 cfs @ 1.74 fps)

# Summary for Pond IB-1:

Inflow Area =	89,812 sf, 3	36.94% Impervious,	Inflow Depth > 1.14" for 2-Year event
Inflow =	2.1 cfs @ 12	2.16 hrs, Volume=	8,497 cf
Outflow =	1.7 cfs @ 12	2.25 hrs, Volume=	8,287 cf, Atten= 17%, Lag= 5.3 min
Discarded =	0.0 cfs @ 12	2.25 hrs, Volume=	661 cf
Primary =	1.2 cfs @ 12	2.25 hrs, Volume=	7,421 cf
Secondary =	0.5 cfs @ 12	2.25 hrs, Volume=	204 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 285.59' @ 12.25 hrs Surf.Area= 1,073 sf Storage= 916 cf

Plug-Flow detention time= 25.5 min calculated for 8,287 cf (98% of inflow) Center-of-Mass det. time= 11.9 min (892.1 - 880.2)

Volume	Invert	Avail.S	Storage	Storage Descriptio	n	
#1	284.00'	3	3,366 cf	Custom Stage Data (Irregular)Listed below (Recalc)		below (Recalc)
	-		<b>.</b> .			
Elevatio		urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
284.0	00	208	72.5	0	0	208
285.0	00	661	129.3	413	413	1,126
286.0	00	1,425	189.1	1,019	1,432	2,649
287.0	00	2,492	246.3	1,934	3,366	4,643
Device	Routing	Inve	rt Outle	et Devices		
#1	Discarded	284.0	0' <b>1.02</b>	0 in/hr Exfiltration	over Surface area	l
#2	Secondary	285.5	0' <b>8.0'</b>	long x 15.0' bread	th Broad-Crested	Rectangular Weir
				d (feet) 0.20 0.40		
				. (English) 2.68 2.		
#3	Primary	284.5		Round Culvert		
	,			0.0' CPP, projectin	ng, no headwall, Ke	e= 0.900
						0.0667 '/' Cc= 0.900
				.013, Flow Area= 0		

**Discarded OutFlow** Max=0.0 cfs @ 12.25 hrs HW=285.58' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

**Primary OutFlow** Max=1.1 cfs @ 12.25 hrs HW=285.58' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 1.1 cfs @ 3.29 fps)

Secondary OutFlow Max=0.5 cfs @ 12.25 hrs HW=285.58' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 0.5 cfs @ 0.77 fps)

# Summary for Pond IB-2:

Inflow Area =	28,415 sf, 34.30% Impervious,	Inflow Depth > 1.01" for 2-Year event
Inflow =	0.7 cfs @ 12.12 hrs, Volume=	2,402 cf
Outflow =	0.2 cfs @ 12.41 hrs, Volume=	1,840 cf, Atten= 75%, Lag= 17.4 min
Primary =	0.2 cfs @ 12.41 hrs, Volume=	1,840 cf
Secondary =	0.0 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 282.76' @ 12.41 hrs Surf.Area= 1,189 sf Storage= 764 cf

Plug-Flow detention time= 173.0 min calculated for 1,836 cf (76% of inflow) Center-of-Mass det. time= 71.2 min ( 958.3 - 887.1 )

Page 23

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Volume	Invert	Avail	.Storage	Storage Description	า	
#1	282.00'		4,789 cf	Custom Stage Dat	t <b>a (Irregular)</b> Listed	below (Recalc)
Elevatio (fee 282.0 283.0 284.0 285.0	et) 20 20 20	urf.Area (sq-ft) 834 1,315 1,855 2,452	Perim. (feet) 150.9 170.2 189.4 208.4	Inc.Store (cubic-feet) 0 1,065 1,577 2,147	Cum.Store (cubic-feet) 0 1,065 2,643 4,789	Wet.Area (sq-ft) 834 1,353 1,931 2,564
Device	Routing	Inv	vert Outle	et Devices		
#1	Secondary	284.		long x 22.0' bread		
#2	Primary	282.	Coef 50' <b>8.0''</b> L= 3 Inlet	d (feet) 0.20 0.40 0 f. (English) 2.68 2.7 <b>Round Culvert</b> 7.4' CPP, projecting / Outlet Invert= 282. .013 Corrugated PE	0 2.70 2.64 2.63 g, no headwall, Ke 50' / 278.50' S= 0	2.64 2.64 2.63 = 0.900 .1070 '/' Cc= 0.900

Primary OutFlow Max=0.2 cfs @ 12.41 hrs HW=282.76' TW=0.00' (Dynamic Tailwater) ←2=Culvert (Inlet Controls 0.2 cfs @ 1.37 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=282.00' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

# Summary for Pond IC-1:

Inflow Area =	37,292 sf,	93.64% Impervious,	Inflow Depth > 2.67" for 2-Year event
Inflow =	2.3 cfs @	12.11 hrs, Volume=	8,310 cf
Outflow =	0.1 cfs @	10.85 hrs, Volume=	6,387 cf, Atten= 95%, Lag= 0.0 min
Discarded =	0.1 cfs @	10.85 hrs, Volume=	6,387 cf
Primary =	0.0 cfs @	0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 284.74' @ 14.67 hrs Surf.Area= 4,404 sf Storage= 3,540 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 137.8 min ( 917.8 - 780.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
		9,501 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
			L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.1 cfs @ 10.85 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

**Primary OutFlow** Max=0.0 cfs @ 0.00 hrs HW=283.50' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Controls 0.0 cfs) **3=Orifice/Grate** (Controls 0.0 cfs)

# **Summary for Pond IC-2:**

Inflow Area =	52,660 sf, 79.76% Impervious,	Inflow Depth > 2.26" for 2-Year event
Inflow =	2.7 cfs @ 12.12 hrs, Volume=	9,936 cf
Outflow =	0.1 cfs @ 14.94 hrs, Volume=	6,485 cf, Atten= 95%, Lag= 169.3 min
Discarded =	0.1 cfs @ 10.75 hrs, Volume=	6,355 cf
Primary =	0.0 cfs @ 14.94 hrs, Volume=	130 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.55' @ 14.94 hrs Surf.Area= 4,361 sf Storage= 4,577 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 119.7 min (914.9 - 795.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows
		0.414 of	Total Available Storage

9,414 ct I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
			L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.1 cfs @ 10.75 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=0.0 cfs @ 14.94 hrs HW=288.55' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 0.0 cfs of 0.2 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.0 cfs @ 0.76 fps)

# Summary for Link AP-1:

Inflow Area	a =	269,982 sf,	44.96% Impervious,	Inflow Depth > (	0.54" for 2-Year event
Inflow	=	2.2 cfs @	12.25 hrs, Volume=	12,098 cf	
Primary	=	2.2 cfs @	12.25 hrs, Volume=	12,098 cf	, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

20	3 Ayer Road, Harvard, MA
POST NRCC 24-hr L	0 10-Year Rainfall=4.68"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 3/9/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC	Page 26

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>1.87" Flow Length=233' Tc=5.8 min CN=71 Runoff=0.7 cfs 2,377 cf
SubcatchmentSC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>3.07" Flow Length=335' Tc=9.0 min CN=85 Runoff=1.8 cfs 6,758 cf
SubcatchmentSC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>2.26" Flow Length=520' Tc=13.0 min CN=76 Runoff=1.5 cfs 6,320 cf
SubcatchmentSC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>4.21" Tc=5.0 min CN=96 Runoff=0.9 cfs 3,257 cf
Subcatchment SC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>2.70" Flow Length=295' Tc=7.8 min CN=81 Runoff=1.2 cfs 4,175 cf
SubcatchmentSC2.3:	Runoff Area=11,540 sf 37.45% Impervious Runoff Depth>2.19" Flow Length=185' Tc=5.0 min CN=75 Runoff=0.6 cfs 2,106 cf
SubcatchmentSC3.1:	Runoff Area=11,615 sf 100.00% Impervious Runoff Depth>4.44" Tc=5.0 min CN=98 Runoff=1.1 cfs 4,298 cf
SubcatchmentSC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>4.10" Flow Length=118' Slope=0.0380 '/' Tc=5.2 min CN=95 Runoff=2.4 cfs 8,771 cf
SubcatchmentSC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>1.37" Flow Length=216' Tc=5.6 min CN=64 Runoff=0.5 cfs 1,679 cf
SubcatchmentSC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>4.44" Tc=5.0 min CN=98 Runoff=1.5 cfs 5,550 cf
SubcatchmentSC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>2.61" Flow Length=231' Tc=5.0 min CN=80 Runoff=0.7 cfs 2,283 cf
SubcatchmentSC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>4.10" Tc=5.0 min CN=95 Runoff=0.4 cfs 1,599 cf
Subcatchment SC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>3.99" Tc=5.0 min CN=94 Runoff=0.5 cfs 1,707 cf
SubcatchmentSC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>1.18" Tc=5.0 min CN=61 Runoff=0.2 cfs 628 cf
SubcatchmentSC5.1:	Runoff Area=61,802 sf 2.47% Impervious Runoff Depth>1.30" Flow Length=362' Tc=10.0 min CN=63 Runoff=1.6 cfs 6,720 cf
Pond CB-1:	Peak Elev=297.99' Inflow=0.7 cfs 2,377 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=0.7 cfs 2,377 cf

<b>POST</b> Prepared by Goldsmith, Pres <u>HydroCAD® 10.10-4a_s/n 01036</u>	203 Ayer Road, Harvard, MA NRCC 24-hr D 10-Year Rainfall=4.68" t & Ringwall, Inc. Printed 3/9/2022 © 2020 HydroCAD Software Solutions LLC Page 27
Pond CB-10:	Peak Elev=285.55' Inflow=0.7 cfs 2,283 cf 12.0" Round Culvert n=0.013 L=21.0' S=0.0100 '/' Outflow=0.7 cfs 2,283 cf
Pond CB-2:	Peak Elev=295.00' Inflow=1.8 cfs 6,758 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0200 '/' Outflow=1.8 cfs 6,758 cf
Pond CB-3:	Peak Elev=288.30' Inflow=1.5 cfs 6,320 cf 12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/' Outflow=1.5 cfs 6,320 cf
Pond CB-5:	Peak Elev=294.65' Inflow=1.2 cfs 4,175 cf 12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/' Outflow=1.2 cfs 4,175 cf
Pond CB-6:	Peak Elev=296.05' Inflow=0.9 cfs 3,257 cf 12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/' Outflow=0.9 cfs 3,257 cf
Pond CB-7:	Peak Elev=288.92' Inflow=0.4 cfs 1,599 cf 12.0" Round Culvert n=0.013 L=22.0' S=0.0136 '/' Outflow=0.4 cfs 1,599 cf
Pond CB-8:	Peak Elev=288.93' Inflow=0.5 cfs 1,707 cf 12.0" Round Culvert n=0.013 L=15.0' S=0.0200 '/' Outflow=0.5 cfs 1,707 cf
Pond CB-9:	Peak Elev=287.46' Inflow=0.6 cfs 2,106 cf 12.0" Round Culvert n=0.013 L=69.0' S=0.0319 '/' Outflow=0.6 cfs 2,106 cf
Pond DCB-4:	Peak Elev=285.71' Inflow=2.4 cfs 8,771 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/' Outflow=2.4 cfs 8,771 cf
Pond DMH-1:	Peak Elev=294.65' Inflow=2.5 cfs 9,134 cf 12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/' Outflow=2.5 cfs 9,134 cf
Pond DMH-2:	Peak Elev=287.98' Inflow=3.8 cfs 15,454 cf 18.0" Round Culvert n=0.013 L=48.0' S=0.0100 '/' Outflow=3.8 cfs 15,454 cf
Pond DMH-3:	Peak Elev=285.71' Inflow=2.4 cfs 8,771 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=2.4 cfs 8,764 cf
Pond DMH-5:	Peak Elev=289.94' Inflow=2.0 cfs 7,432 cf 12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/' Outflow=2.0 cfs 7,432 cf
Pond DMH-6:	Peak Elev=288.93' Inflow=2.4 cfs 9,031 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=2.4 cfs 9,031 cf
Pond DMH-7:	Peak Elev=288.92' Inflow=0.5 cfs 1,707 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=0.5 cfs 1,700 cf
Pond DMH-9:	Peak Elev=285.34' Inflow=1.3 cfs 4,388 cf 15.0" Round Culvert n=0.013 L=185.0' S=0.0146 '/' Outflow=1.3 cfs 4,388 cf
Pond IB-1:	Peak Elev=285.76' Storage=1,113 cf Inflow=4.2 cfs 17,134 cf

Pond IB-1: Peak Elev=285.76' Storage=1,113 cf Inflow=4.2 cfs 17,134 cf Discarded=0.0 cfs 810 cf Primary=1.3 cfs 13,654 cf Secondary=2.8 cfs 2,433 cf Outflow=4.1 cfs 16,897 cf

<b>POST</b> Prepared by Goldsmith, Pr <u>HydroCAD® 10.10-4a_s/n 0103</u>		203 Ayer Road, Harvard, MA for D 10-Year Rainfall=4.68" Printed 3/9/2022 Page 28
Pond IB-2:	Peak Elev=283.18' Storage= Primary=0.8 cfs  4,422 cf   Secondary=0.0 cfs	1,307 cf Inflow=1.5 cfs 5,016 cf s 0 cf Outflow=0.8 cfs 4,422 cf
Pond IC-1:	Peak Elev=285.71' Storage=6, Discarded=0.1 cfs 7,162 cf Primary=0.0 cfs	710 cf Inflow=3.5 cfs 13,061 cf s 0 cf Outflow=0.1 cfs 7,162 cf
Pond IC-2:	Peak Elev=288.92' Storage=5, Discarded=0.1 cfs 7,131 cf Primary=1.0 cfs 4,88	776 cf Inflow=4.3 cfs 16,280 cf 84 cf Outflow=1.1 cfs 12,015 cf
Link AP-1:		Inflow=6.9 cfs 32,113 cf Primary=6.9 cfs 32,113 cf

# Total Runoff Area = 269,982 sf Runoff Volume = 58,227 cf Average Runoff Depth = 2.59" 55.04% Pervious = 148,611 sf 44.96% Impervious = 121,371 sf

# Summary for Subcatchment SC1.1:

$-$ 0.7 03 ( $w_{1}$ 12.10 113, Volume 2,077 0, Depute 1.07	Runoff =	0.7 cfs @	12.13 hrs, Volume=	2,377 cf, Depth> 1.87"
---	----------	-----------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN [	Description						
	4,381	61 >	>75% Grass cover, Good, HSG B						
	5,583	55 \	Voods, Go	od, HSG B					
	328	98 l	Jnconnecte	ed pavemer	nt, HSG B				
	3,654	98 F	Paved park	ing, HSG B	5				
	1,269	96 (	Gravel surface, HSG B						
	15,215	71 \	Weighted Average						
	11,233	7	73.83% Pervious Area						
	3,982		26.17% Impervious Area						
	328	3	8.23% Unconnected						
_				_					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.4	34	0.2400	0.17		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
0.9	16	0.1875	0.28		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.12"				
1.0	83	0.0361	1.33		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.5	100	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
5.8	233	Total							

# **Summary for Subcatchment SC1.2:**

Runoff = 1.8 cfs @ 12.16 hrs, Volume= 6,758 cf, Depth> 3.07"

Area (sf)	CN	Description			
6,620	61	>75% Grass cover, Good, HSG B			
2,398	55	Woods, Good, HSG B			
1,172	98	Unconnected pavement, HSG B			
8,000	98	Roofs, HSG B			
7,203	98	Paved parking, HSG B			
1,052	96	Gravel surface, HSG B			
26,444	85	Weighted Average			
10,069		38.08% Pervious Area			
16,375		61.92% Impervious Area			
1,172		7.16% Unconnected			

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Page 30

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
3.9	41	0.2400	0.17		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
1.4	9	0.0200	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
3.4	222	0.0248	1.10		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	63	0.0300	3.52		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
9.0	335	Total			

# Summary for Subcatchment SC1.3:

Runoff = 1.5 cfs @ 12.21 hrs, Volume= 6,320 cf, Depth> 2.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN E	Description						
	15,029	61 >	75% Gras	s cover, Go	bod, HSG B				
	3,894	55 V	Voods, Go	od, HSG B					
	1,741	96 0	Gravel surfa	ace, HSG E	3				
	275	98 L	Jnconnected pavement, HSG B						
	12,548	98 F	aved park	ing, HSG B					
	33,487	76 V	Veighted A	verage					
	20,664	6	1.71% Per	vious Area					
	12,823	3	8.29% Imp	pervious Are	ea				
	275	2	.14% Unco	onnected					
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.2	50	0.0800	0.12		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
3.2	188	0.0372	0.96		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
2.0	160	0.0372	1.35		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.6	122	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
13.0	520	Total							

# Summary for Subcatchment SC2.1:

Runoff = 0.9 cfs @ 12.11 hrs, Volume= 3,257 cf, Depth> 4.21"

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Α	rea (sf)	CN	Description					
	595	61	>75% Gras	s cover, Go	ood, HSG B			
	423	98	Unconnecte	ed pavemer	nt, HSG B			
	4,000	98	Roofs, HSG	βB				
	4,263	98	Paved park	ing, HSG B	5			
	9,281	96	Weighted A	verage				
	595		6.41% Pervious Area					
	8,686		93.59% Impervious Area					
	423		4.87% Unconnected					
Тс	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

#### **Summary for Subcatchment SC2.2:**

Runoff	=	1.2 cfs @	12.15 hrs,	Volume=	4,175 cf,	Depth> 2.70"
--------	---	-----------	------------	---------	-----------	--------------

A	rea (sf)	CN D	escription						
	5,529		61 >75% Grass cover, Good, HSG B						
	2,637			od, HSG B					
	889			ed pavemei	nt, HSG B				
	4,000	98 R	loofs, HSG	ЪВ					
	4,580			ing, HSG E					
	929	96 G	Fravel surfa	ace, HSG E	}				
	18,563	81 V	Veighted A	verage					
	9,094	4	8.99% Per	vious Area					
	9,469	5	1.01% Imp	pervious Ar	ea				
	889	9	9.39% Unconnected						
_				_					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
4.6	50	0.2400	0.18		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
0.2	8	0.0200	0.71		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
2.5	151	0.0200	0.99		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.5	86	0.0200	2.87		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
7.8	295	Total							

#### Summary for Subcatchment SC2.3:

Runoff = 0.6 cfs @ 12.12 hrs, Volume= 2,106 cf, Depth> 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

 Α	rea (sf)	CN E	escription						
	7,218	61 >	75% Gras	s cover, Go	bod, HSG B				
	293			ed pavemer					
	4,029	98 F	aved park	ing, HSG B	}				
	11,540	75 V	75 Weighted Average						
	7,218	6	2.55% Per	vious Area					
	4,321		37.45% Impervious Area						
	293	6	6.77% Unconnected						
_				_					
Τc	Length	Slope	Velocity	Capacity	Description				
 nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
2.3	42	0.1430	0.31		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.12"				
0.1	8	0.0500	1.18		Sheet Flow,				
					Smooth surfaces n= 0.011 P2= 3.12"				
0.5	135	0.0500	4.54		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
2.9	185	Total, I	ncreased t	o minimum	Tc = 5.0 min				

# Summary for Subcatchment SC3.1:

Runoff = 1.1 cfs @ 12.11 hrs, Volume= 4,298 cf, Depth> 4.44"

Ar	ea (sf)	CN	Description							
	11,615	98	Roofs, HSG B							
	0	61	>75% Gras	>75% Grass cover, Good, HSG B						
	11,615	98	Weighted Average							
	0		0.00% Pervious Area							
	11,615		100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description					
5.0					Direct Entry,					

#### Summary for Subcatchment SC3.2:

Runoff = 2.4 cfs @ 12.11 hrs, Volume= 8,771 cf, Depth> 4.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN E	Description						
	2,372	61 >	75% Gras	s cover, Go	bod, HSG B				
	21,883	98 F	Paved park	ing, HSG B					
	1,422	98 L	Inconnecte	ed pavemer	nt, HSG B				
	25,677	95 V	Veighted A	verage					
	2,372	ç	9.24% Pervious Area						
	23,305	ç	90.76% Impervious Area						
	1,422	6	6.10% Unconnected						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
4.4	50	0.0380	0.19		Sheet Flow,				
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
5.2	118	Total							

# Summary for Subcatchment SC3.3:

Runoff = 0.5 cfs @ 12.13 hrs, Volume= 1,679 cf, Depth> 1.37"

	A	rea (sf)	CN [	Description					
		13,540				ood, HSG B			
_		1,127	96 (	Gravel surfa	ace, HSG E	}			
		14,666	64 Weighted Average						
	14,666 100.00% Perviou			00.00% Pe	ervious Are	a			
	_				<b>-</b>				
	Тс	Length	Slope		Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.7	50	0.0600	0.23		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.12"			
	1.9	166	0.0422	1.44		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	5.6	216	Total						

#### Summary for Subcatchment SC4.1:

Runoff = 1.5 cfs @ 12.11 hrs, Volume= 5,550 cf, Depth> 4.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

Area (sf)	CN	Description					
14,998	98	98 Roofs, HSG B					
14,998	14,998 100.00% Impervious Area						
Tc Length (min) (feet)	Slop (ft/f	,	Capacity (cfs)	·			
5.0				Direct Entry,			

#### Summary for Subcatchment SC4.2:

Runoff = 0.7 cfs @ 12.12 hrs, Volume= 2,283 cf, Depth> 2.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

Area (s	sf) (	CN D	escription		
5,06	50	61 >7	75% Grass	s cover, Go	bod, HSG B
1,45	50	98 U	nconnecte	ed pavemer	nt, HSG B
3,97	75	98 Pa	aved parki	ing, HSG B	
10,48	34	80 W	/eighted A	verage	
5,06	5,060 48.26% Pervious Area			vious Area	
5,42	5,424 51.74% Impervious Ar			ervious Are	ea
1,45	1,450 26.72% Unconnected			connected	
Tc Len	gth	Slope	Velocity	Capacity	Description
<u>(min)</u> (fe	et)	(ft/ft)	(ft/sec)	(cfs)	
4.0	50 0	0.0500	0.21		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.4	33 0	0.0500	1.57		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.6 1	148 0	).0440	4.26		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
5.0 2	231 T	otal			

# **Summary for Subcatchment SC4.3:**

Runoff = 0.4 cfs @ 12.11 hrs, Volume= 1,599 cf, Depth> 4.10"

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

A	rea (sf)	CN	I Description							
	406	61	>75% Gras	s cover, Go	ood, HSG B					
	175	98	Unconnecte	ed pavemer	ent, HSG B					
	4,101	98	Paved parking, HSG B							
	4,682	95	Weighted A	verage						
	406		8.68% Pervious Area							
	4,276		91.32% Impervious Area							
	175		4.09% Unconnected							
Тс	Length	Slope	,	Capacity	Description					
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)						
5.0					Direct Entry,					

# Summary for Subcatchment SC4.4:

Runoff = 0.5 cfs @ 12.11 hrs, Volume= 1,707 cf, Depth> 3.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN I	N Description						
	564	61 :	>75% Gras	s cover, Go	ood, HSG B				
	541	98 I	Jnconnecte	ed pavemer	ent, HSG B				
	4,030	98 I	Paved parking, HSG B						
	5,136	94 \	Neighted A	verage					
	564		10.99% Pei	vious Area	a				
	4,571	8	39.01% Imp	pervious Are	rea				
	541		11.83% Un	connected					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)					
5.0					Direct Entry,				

# Summary for Subcatchment SC4.5:

Runoff = 0.2 cfs @ 12.13 hrs, Volume= 628 cf, Depth> 1.18"

Α	rea (sf)	CN E	<b>Description</b>						
	6,391	61 >	61 >75% Grass cover, Good, HSG B						
	6,391	100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

# Summary for Subcatchment SC5.1:

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN D	escription					
	51,973				bod, HSG B			
	5,271		55 Woods, Good, HSG B					
	70			ed pavemer				
	1,456 3,033			ing, HSG B ace, HSG E				
	<u> </u>		Veighted A	· · · · ·	5			
	60,277			rvious Area				
	1,525	-		ervious Area				
	70		.56% Unc		-			
Tc	Length	Slope	Velocity		Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.8	36	0.2000	0.16		Sheet Flow,			
2.1	14	0.0200	0.11		Woods: Light underbrush n= 0.400 P2= 3.12" Sheet Flow,			
۷.۱	14	0.0200	0.11		Grass: Short $n = 0.150$ P2= 3.12"			
4.1	312	0.0334	1.28		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
10.0	362	Total						
	Summary for Pond CB-1:							
				Summar				
Inflow A	rea =	15.2	15 sf 26	17% Imperv	vious, Inflow Depth > 1.87" for 10-Year event			
Inflow	=			13 hrs, Volu				
Outflow	=		fs @ 12.′	13 hrs, Volu	ume= 2,377 cf, Atten= 0%, Lag= 0.0 min			
Primary	=	0.7 c	fs @ 12.′	13 hrs, Volu	ume= 2,377 cf			
Deuting	Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2							
		9' @ 12.		ie Span= 0.	.00-24.00 hrs, at= 0.05 hrs / 2			
	ev= 297.9		101115					
	00 001.0							
Device	Routing		Invert C	Outlet Device	es			
#1	Primary	2		2.0" Roun				
					CPP, projecting, no headwall, Ke= 0.900			
					Invert= 297.50' / 293.56' S= 0.0232 '/' Cc= 0.900			
			n	- 0.013 60	orrugated PE, smooth interior, Flow Area= 0.79 sf			

Primary OutFlow Max=0.7 cfs @ 12.13 hrs HW=297.97' TW=294.57' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.7 cfs @ 1.85 fps)

# Summary for Pond CB-10:

Inflow Area =	10,484 sf, 51.74% Impervious, Inflow Depth	> 2.61" for 10-Year event
Inflow =	0.7 cfs @ 12.12 hrs, Volume= 2,28	3 cf
Outflow =	0.7 cfs @ 12.12 hrs, Volume= 2,28	3 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.7 cfs @ 12.12 hrs, Volume= 2,28	3 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.55' @ 12.12 hrs Flood Elev= 289.00'

900 sf

Primary OutFlow Max=0.7 cfs @ 12.12 hrs HW=285.54' TW=285.32' (Dynamic Tailwater) ↓ 1=Culvert (Outlet Controls 0.7 cfs @ 2.27 fps)

# Summary for Pond CB-2:

Inflow Area =	26,444 sf, 61.92% Imperviou	s, Inflow Depth > 3.07" for 10-Year event
Inflow =	1.8 cfs @ 12.16 hrs, Volume	e= 6,758 cf
Outflow =	1.8 cfs @ 12.16 hrs, Volume	e= 6,758 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.8 cfs @ 12.16 hrs, Volume	e= 6,758 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 295.00' @ 12.15 hrs Flood Elev= 297.70'

Device	Routing	Invert	Outlet Devices
#1	<u> </u>		<b>12.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.8 cfs @ 12.16 hrs HW=294.95' TW=294.61' (Dynamic Tailwater)

# Summary for Pond CB-3:

Inflow Area	a =	33,487 sf,	38.29% Impervious,	Inflow Depth > 2.26" for 10-Year event
Inflow	=	1.5 cfs @	12.21 hrs, Volume=	6,320 cf
Outflow	=	1.5 cfs @	12.21 hrs, Volume=	6,320 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.5 cfs @	12.21 hrs, Volume=	6,320 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 288.30' @ 12.21 hrs Flood Elev= 291.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.50'	<b>12.0" Round Culvert</b> L= 8.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.5 cfs @ 12.21 hrs HW=288.29' TW=287.91' (Dynamic Tailwater) ☐ 1=Culvert (Barrel Controls 1.5 cfs @ 3.01 fps)

#### Summary for Pond CB-5:

Inflow Area	a =	18,563 sf,	51.01% Impervious,	Inflow Depth > 2.70" for 10-Year event
Inflow	=	1.2 cfs @	12.15 hrs, Volume=	4,175 cf
Outflow	=	1.2 cfs @	12.15 hrs, Volume=	4,175 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.2 cfs @	12.15 hrs, Volume=	4,175 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.65' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert
			L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.2 cfs @ 12.15 hrs HW=294.65' TW=289.93' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.2 cfs @ 2.17 fps)

# Summary for Pond CB-6:

Inflow Area	a =	9,281 sf,	93.59% Impervious,	Inflow Depth > 4.21" for 10-Year event
Inflow	=	0.9 cfs @	12.11 hrs, Volume=	3,257 cf
Outflow	=	0.9 cfs @	12.11 hrs, Volume=	3,257 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.9 cfs @	12.11 hrs, Volume=	3,257 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 296.05' @ 12.11 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	<b>12.0" Round Culvert</b> L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.9 cfs @ 12.11 hrs HW=296.04' TW=289.90' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 0.9 cfs @ 1.98 fps)

# Summary for Pond CB-7:

Inflow Are	a =	4,682 sf,	91.32% Impervious,	Inflow Depth > 4.10" for 10-Year event
Inflow	=	0.4 cfs @	12.11 hrs, Volume=	1,599 cf
Outflow	=	0.4 cfs @	12.11 hrs, Volume=	1,599 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.4 cfs @	12.11 hrs, Volume=	1,599 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.92' @ 12.41 hrs Flood Elev= 292.00'

Device Routing Invert Outlet Devices	
#1 Primary 288.00' <b>12.0'' Round Culvert</b> L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' n= 0.013 Corrugated PE, smooth interior, Flow Are	Cc= 0.900

Primary OutFlow Max=0.0 cfs @ 12.11 hrs HW=288.56' TW=288.60' (Dynamic Tailwater)

# Summary for Pond CB-8:

Inflow Area =	5,136 sf,	89.01% Impervious,	Inflow Depth > 3.99" for 10-Year event
Inflow =	0.5 cfs @	12.11 hrs, Volume=	1,707 cf
Outflow =	0.5 cfs @	12.11 hrs, Volume=	1,707 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.5 cfs @	12.11 hrs, Volume=	1,707 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.93' @ 12.41 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices
<u></u> #1	Primary	288.50'	<b>12.0" Round Culvert</b> L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
<b>_</b> .			40.44 hrs. 1044.000.001 TML 000.401 (Demonstration)

Primary OutFlow Max=0.5 cfs @ 12.11 hrs HW=288.88' TW=288.46' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.5 cfs @ 1.67 fps)

# Summary for Pond CB-9:

Inflow Area =	11,540 sf,	37.45% Impervious,	Inflow Depth > 2.19" for 10-Year event
Inflow =	0.6 cfs @	12.12 hrs, Volume=	2,106 cf
Outflow =	0.6 cfs @	12.12 hrs, Volume=	2,106 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.6 cfs @	12.12 hrs, Volume=	2,106 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.46' @ 12.12 hrs Flood Elev= 291.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.00'	<b>12.0" Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.00' / 284.80' S= 0.0319 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.6 cfs @ 12.12 hrs HW=287.45' TW=285.32' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.6 cfs @ 1.80 fps)

# Summary for Pond DCB-4:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth > 4.10" for 10-Year event
Inflow	=	2.4 cfs @	12.11 hrs, Volume=	8,771 cf
Outflow	=	2.4 cfs @	12.11 hrs, Volume=	8,771 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.4 cfs @	12.11 hrs, Volume=	8,771 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.71' @ 16.82 hrs Flood Elev= 288.50'

Device Routing Invert Outlet Devices	
#1 Primary 284.50' <b>12.0'' Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' C n= 0.013 Corrugated PE, smooth interior, Flow Area=	

Primary OutFlow Max=2.3 cfs @ 12.11 hrs HW=285.60' TW=284.94' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.3 cfs @ 2.95 fps)

# Summary for Pond DMH-1:

Inflow Area	=	41,659 sf,	48.86% Impervious,	Inflow Depth > 2.63" for 10-Year event
Inflow	=	2.5 cfs @	12.15 hrs, Volume=	9,134 cf
Outflow	=	2.5 cfs @	12.15 hrs, Volume=	9,134 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.5 cfs @	12.15 hrs, Volume=	9,134 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

# 203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D10-Year Rainfall=4.68"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 41

Peak Elev= 294.65' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	<b>12.0" Round Culvert</b> L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.5 cfs @ 12.15 hrs HW=294.65' TW=287.97' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 2.5 cfs @ 3.15 fps)

#### Summary for Pond DMH-2:

Inflow Area =	75,146 sf,	44.15% Impervious,	Inflow Depth > 2.47" for 10-Year event
Inflow =	3.8 cfs @	12.16 hrs, Volume=	15,454 cf
Outflow =	3.8 cfs @	12.16 hrs, Volume=	15,454 cf, Atten= 0%, Lag= 0.0 min
Primary =	3.8 cfs @	12.16 hrs, Volume=	15,454 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.98' @ 12.16 hrs Flood Elev= 291.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.90'	18.0" Round Culvert
			L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.7 cfs @ 12.16 hrs HW=287.96' TW=285.75' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.7 cfs @ 2.77 fps)

# **Summary for Pond DMH-3:**

Inflow Area	=	25,677 sf,	90.76% Impervious,	Inflow Depth > 4.10" for 10-Year event
Inflow :	=	2.4 cfs @	12.11 hrs, Volume=	8,771 cf
Outflow :	=	2.4 cfs @	12.11 hrs, Volume=	8,764 cf, Atten= 0%, Lag= 0.0 min
Primary :	=	2.4 cfs @	12.11 hrs, Volume=	8,764 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.71' @ 16.78 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.10'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.3 cfs @ 12.11 hrs HW=284.94' TW=284.71' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 2.3 cfs @ 1.85 fps)

# Summary for Pond DMH-5:

Inflow Area	a =	27,844 sf,	65.20% Impervious,	Inflow Depth > 3.20" for 10-Year event
Inflow	=	2.0 cfs @	12.13 hrs, Volume=	7,432 cf
Outflow	=	2.0 cfs @	12.13 hrs, Volume=	7,432 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.0 cfs @	12.13 hrs, Volume=	7,432 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.94' @ 12.13 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices	
#1	Primary	289.00'	12.0" Round Culvert	
			L= 88.0' CPP, projecting, no headwall, Ke= 0.900	
			Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900	
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=1.9 cfs @ 12.13 hrs HW=289.91' TW=288.68' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.9 cfs @ 2.57 fps)

# Summary for Pond DMH-6:

Inflow Area =	32,526 sf,	68.96% Impervious,	Inflow Depth > 3.33" for 10-Year event
Inflow =	2.4 cfs @	12.13 hrs, Volume=	9,031 cf
Outflow =	2.4 cfs @	12.13 hrs, Volume=	9,031 cf, Atten= 0%, Lag= 0.0 min
Primary =	2.4 cfs @	12.13 hrs, Volume=	9,031 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.93' @ 12.36 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	<b>24.0" Round Culvert</b> L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.4 cfs @ 12.13 hrs HW=288.66' TW=288.53' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.4 cfs @ 1.40 fps) 203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D10-Year Rainfall=4.68"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 43

# Summary for Pond DMH-7:

Inflow Area =	5,136 sf, 89.01	1% Impervious,	Inflow Depth > 3.99"	for 10-Year event
Inflow =	0.5 cfs @ 12.11	1 hrs, Volume=	1,707 cf	
Outflow =	0.5 cfs @ 12.11	1 hrs, Volume=	1,700 cf, Atte	en= 0%, Lag= 0.0 min
Primary =	0.5 cfs @ 12.11	1 hrs, Volume=	1,700 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.92' @ 12.37 hrs Flood Elev= 292.40'

#1 Drimony 287.60' 24.0" Bound Culvert	Device	Routing	Invert	Outlet Devices
L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf	#1	Primary	287.60'	Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900

Primary OutFlow Max=0.5 cfs @ 12.11 hrs HW=288.46' TW=288.45' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 0.5 cfs @ 0.37 fps)

#### Summary for Pond DMH-9:

Inflow Area =	22,024 sf,	44.25% Impervious,	Inflow Depth > 2.39" for 10-Year event
Inflow =	1.3 cfs @	12.12 hrs, Volume=	4,388 cf
Outflow =	1.3 cfs @	12.12 hrs, Volume=	4,388 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.3 cfs @	12.12 hrs, Volume=	4,388 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.34' @ 12.12 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.70'	<b>15.0" Round Culvert</b> L= 185.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.70' / 282.00' S= 0.0146 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.3 cfs @ 12.12 hrs HW=285.32' TW=283.05' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.3 cfs @ 2.12 fps)

# Summary for Pond IB-1:

Inflow Area =	89,812 sf,	36.94% Impervious,	Inflow Depth > 2.29" for 10-Year event
Inflow =	4.2 cfs @	12.16 hrs, Volume=	17,134 cf
Outflow =	4.1 cfs @	12.18 hrs, Volume=	16,897 cf, Atten= 3%, Lag= 1.5 min
Discarded =	0.0 cfs @	12.18 hrs, Volume=	810 cf
Primary =	1.3 cfs @	12.18 hrs, Volume=	13,654 cf
Secondary =	2.8 cfs @	12.18 hrs, Volume=	2,433 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 285.76' @ 12.18 hrs Surf.Area= 1,213 sf Storage= 1,113 cf

Plug-Flow detention time= 17.1 min calculated for 16,862 cf (98% of inflow) Center-of-Mass det. time= 9.2 min ( 865.6 - 856.4 )

Volume	Invert	Avail.S	Storage	Storage Description	on	
#1	284.00'	3	8,366 cf	Custom Stage Da	<b>ata (Irregular)</b> Liste	d below (Recalc)
_					<b>a a</b>	
Elevatio		urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
284.0	00	208	72.5	0	0	208
285.0	00	661	129.3	413	413	1,126
286.0	00	1,425	189.1	1,019	1,432	2,649
287.0	00	2,492	246.3	1,934	3,366	4,643
Device	Routing	Inve	ert Outle	et Devices		
#1	Discarded	284.0	0' <b>1.02</b>	0 in/hr Exfiltration	over Surface area	a
#2	Secondary	285.5	0' <b>8.0'</b> I	long x 15.0' bread	Ith Broad-Crested	Rectangular Weir
				d (feet) 0.20 0.40		
				f. (Engĺish) 2.68 2.		
#3	Primary	284.5		Round Culvert		
	,			0.0' CPP, projecti	ng, no headwall, K	(e= 0.900
						0.0667 '/' Cc= 0.900
				.013, Flow Area= (		

**Discarded OutFlow** Max=0.0 cfs @ 12.18 hrs HW=285.75' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

**Primary OutFlow** Max=1.3 cfs @ 12.18 hrs HW=285.75' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 1.3 cfs @ 3.65 fps)

Secondary OutFlow Max=2.7 cfs @ 12.18 hrs HW=285.75' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 2.7 cfs @ 1.35 fps)

# Summary for Pond IB-2:

Inflow Area =	28,415 sf,	34.30% Impervious,	Inflow Depth > 2.12" for 10-Year event
Inflow =	1.5 cfs @	12.12 hrs, Volume=	5,016 cf
Outflow =	0.8 cfs @	12.22 hrs, Volume=	4,422 cf, Atten= 49%, Lag= 6.2 min
Primary =	0.8 cfs @	12.22 hrs, Volume=	4,422 cf
Secondary =	0.0 cfs @	0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 283.18' @ 12.22 hrs Surf.Area= 1,404 sf Storage= 1,307 cf

Plug-Flow detention time= 101.2 min calculated for 4,422 cf (88% of inflow) Center-of-Mass det. time= 41.8 min ( 902.9 - 861.2 )

Page 45

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Volume	Inver	rt Avai	l.Storage	Storage Description	n	
#1	282.00	)'	4,789 cf	Custom Stage Dat	<b>ta (Irregular)</b> Listed	below (Recalc)
Elevatio (fee 282.0	et)	Surf.Area (sq-ft) 834	Perim. (feet) 150.9	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0	Wet.Area (sq-ft) 834
282.0 283.0 284.0 285.0	00	1,315 1,855 2,452	170.2 189.4 208.4	1,065 1,577 2,147	1,065 2,643 4,789	834 1,353 1,931 2,564
Device	Routing	Inv	vert Outle	et Devices		
#1	Secondar	y 284		' long x 22.0' bread		
#2	Primary	282	Coet 50' <b>8.0''</b> L= 3 Inlet	d (feet) 0.20 0.40 ( f. (English) 2.68 2.7 <b>Round Culvert</b> 7.4' CPP, projecting / Outlet Invert= 282. .013 Corrugated PE	70 2.70 2.64 2.63 g, no headwall, Ke .50' / 278.50' S= 0	2.64 2.64 2.63 = 0.900 0.1070 '/' Cc= 0.900

Primary OutFlow Max=0.8 cfs @ 12.22 hrs HW=283.17' TW=0.00' (Dynamic Tailwater) ←2=Culvert (Inlet Controls 0.8 cfs @ 2.21 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=282.00' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

# Summary for Pond IC-1:

Inflow Area =	37,292 sf,	93.64% Impervious,	Inflow Depth > 4.20" for 10-Year event
Inflow =	3.5 cfs @	12.11 hrs, Volume=	13,061 cf
Outflow =	0.1 cfs @	9.55 hrs, Volume=	7,162 cf, Atten= 97%, Lag= 0.0 min
Discarded =	0.1 cfs @	9.55 hrs, Volume=	7,162 cf
Primary =	0.0 cfs @	0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.71' @ 16.78 hrs Surf.Area= 4,404 sf Storage= 6,710 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 91.9 min ( 859.0 - 767.1 )

Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
-		9 501 cf	Total Available Storage

9,501 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
			L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 9.55 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

**Primary OutFlow** Max=0.0 cfs @ 0.00 hrs HW=283.50' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Controls 0.0 cfs) **3=Orifice/Grate** (Controls 0.0 cfs)

# **Summary for Pond IC-2:**

Inflow Area =	52,660 sf, 79.76%	6 Impervious, Infl	low Depth > 3.71"	for 10-Year event
Inflow =	4.3 cfs @ 12.12 l	nrs, Volume=	16,280 cf	
Outflow =	1.1 cfs @ 12.37 l	nrs, Volume=	12,015 cf, Atte	n= 74%, Lag= 14.9 min
Discarded =	0.1 cfs @ 9.25 l	nrs, Volume=	7,131 cf	-
Primary =	1.0 cfs @ 12.37 l	nrs, Volume=	4,884 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.92' @ 12.37 hrs Surf.Area= 4,361 sf Storage= 5,776 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 58.0 min (841.6 - 783.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows
		0.414 of	Total Available Storage

9,414 ct I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
			L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.1 cfs @ 9.25 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=1.0 cfs @ 12.37 hrs HW=288.92' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 1.0 cfs of 1.1 cfs potential flow) -3=Orifice/Grate (Orifice Controls 1.0 cfs @ 2.20 fps)

#### Summary for Link AP-1:

Inflow Area =	269,982 sf,	44.96% Impervious,	Inflow Depth > 1.43"	for 10-Year event
Inflow =	6.9 cfs @	12.20 hrs, Volume=	32,113 cf	
Primary =	6.9 cfs @	12.20 hrs, Volume=	32,113 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

	203 Ayer Road, Harvard, MA
POST	NRCC 24-hr D 50-Year Rainfall=7.00"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 3/9/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solution	ons LLC Page 48

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>3.72" Flow Length=233' Tc=5.8 min CN=71 Runoff=1.4 cfs 4,714 cf
SubcatchmentSC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>5.24" Flow Length=335' Tc=9.0 min CN=85 Runoff=3.0 cfs 11,551 cf
SubcatchmentSC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>4.24" Flow Length=520' Tc=13.0 min CN=76 Runoff=2.8 cfs 11,836 cf
SubcatchmentSC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>6.52" Tc=5.0 min CN=96 Runoff=1.3 cfs 5,041 cf
SubcatchmentSC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>4.80" Flow Length=295' Tc=7.8 min CN=81 Runoff=2.0 cfs 7,419 cf
Subcatchment SC2.3:	Runoff Area=11,540 sf 37.45% Impervious Runoff Depth>4.14" Flow Length=185' Tc=5.0 min CN=75 Runoff=1.2 cfs 3,986 cf
SubcatchmentSC3.1:	Runoff Area=11,615 sf 100.00% Impervious Runoff Depth>6.76" Tc=5.0 min CN=98 Runoff=1.7 cfs 6,539 cf
SubcatchmentSC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>6.40" Flow Length=118' Slope=0.0380 '/' Tc=5.2 min CN=95 Runoff=3.6 cfs 13,695 cf
SubcatchmentSC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>3.00" Flow Length=216' Tc=5.6 min CN=64 Runoff=1.1 cfs 3,662 cf
SubcatchmentSC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>6.76" Tc=5.0 min CN=98 Runoff=2.2 cfs 8,444 cf
SubcatchmentSC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>4.69" Flow Length=231' Tc=5.0 min CN=80 Runoff=1.2 cfs 4,097 cf
SubcatchmentSC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>6.40" Tc=5.0 min CN=95 Runoff=0.7 cfs 2,497 cf
SubcatchmentSC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>6.28" Tc=5.0 min CN=94 Runoff=0.7 cfs 2,689 cf
SubcatchmentSC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>2.70" Tc=5.0 min CN=61 Runoff=0.4 cfs 1,437 cf
SubcatchmentSC5.1:	Runoff Area=61,802 sf 2.47% Impervious Runoff Depth>2.89" Flow Length=362' Tc=10.0 min CN=63 Runoff=3.8 cfs 14,889 cf
Pond CB-1:	Peak Elev=298.23' Inflow=1.4 cfs 4,714 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=1.4 cfs 4,714 cf

<b>POST</b> Prepared by Goldsmith, Pre HydroCAD® 10.10-4a_s/n 0103	203 Ayer Road, Harvard, MA NRCC 24-hr D 50-Year Rainfall=7.00" est & Ringwall, Inc. Printed 3/9/2022 6 © 2020 HydroCAD Software Solutions LLC Page 49
Pond CB-10:	Peak Elev=285.83' Inflow=1.2 cfs 4,097 cf 12.0" Round Culvert n=0.013 L=21.0' S=0.0100 '/' Outflow=1.2 cfs 4,097 cf
Pond CB-2:	Peak Elev=297.07' Inflow=3.0 cfs 11,551 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0200 '/' Outflow=3.0 cfs 11,551 cf
Pond CB-3:	Peak Elev=289.44' Inflow=2.8 cfs 11,836 cf 12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/' Outflow=2.8 cfs 11,836 cf
Pond CB-5:	Peak Elev=294.96' Inflow=2.0 cfs 7,419 cf 12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/' Outflow=2.0 cfs 7,419 cf
Pond CB-6:	Peak Elev=296.21' Inflow=1.3 cfs 5,041 cf 12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/' Outflow=1.3 cfs 5,041 cf
Pond CB-7:	Peak Elev=289.87' Inflow=0.7 cfs 2,497 cf 12.0" Round Culvert n=0.013 L=22.0' S=0.0136 '/' Outflow=0.7 cfs 2,497 cf
Pond CB-8:	Peak Elev=289.86' Inflow=0.7 cfs 2,689 cf 12.0" Round Culvert n=0.013 L=15.0' S=0.0200 '/' Outflow=0.7 cfs 2,689 cf
Pond CB-9:	Peak Elev=287.66' Inflow=1.2 cfs 3,986 cf 12.0" Round Culvert n=0.013 L=69.0' S=0.0319 '/' Outflow=1.2 cfs 3,986 cf
Pond DCB-4:	Peak Elev=286.98' Inflow=3.6 cfs 13,695 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/' Outflow=3.6 cfs 13,695 cf
Pond DMH-1:	Peak Elev=296.08' Inflow=4.3 cfs 16,265 cf 12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/' Outflow=4.3 cfs 16,265 cf
Pond DMH-2:	Peak Elev=288.68' Inflow=6.8 cfs 28,101 cf 18.0" Round Culvert n=0.013 L=48.0' S=0.0100 '/' Outflow=6.8 cfs 28,101 cf
Pond DMH-3:	Peak Elev=286.49' Inflow=3.6 cfs 13,695 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=3.6 cfs 13,694 cf
Pond DMH-5:	Peak Elev=290.70' Inflow=3.3 cfs 12,460 cf 12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/' Outflow=3.3 cfs 12,460 cf
Pond DMH-6:	Peak Elev=289.88' Inflow=3.9 cfs 14,958 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=3.9 cfs 14,958 cf
Pond DMH-7:	Peak Elev=289.85' Inflow=0.7 cfs 2,689 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=0.7 cfs 2,675 cf
Pond DMH-9:	Peak Elev=285.61' Inflow=2.4 cfs 8,083 cf 15.0" Round Culvert n=0.013 L=185.0' S=0.0146 '/' Outflow=2.4 cfs 8,083 cf
Pond IB-1:	Peak Elev=285.94' Storage=1,344 cf Inflow=7.9 cfs 31,763 cf

Peak Elev=285.94' Storage=1,344 cf Inflow=7.9 cfs 31,763 cf Discarded=0.0 cfs 984 cf Primary=1.4 cfs 22,824 cf Secondary=6.2 cfs 7,685 cf Outflow=7.7 cfs 31,494 cf

<b>POST</b> Prepared by Goldsmith, F <u>HydroCAD® 10.10-4a_s/n 010</u>	203 Ayer Road, Harvard, I NRCC 24-hr D 50-Year Rainfall=7.0 rest & Ringwall, Inc. Printed 3/9/20 36 © 2020 HydroCAD Software Solutions LLC Page	<i>00"</i> 022
Pond IB-2:	Peak Elev=283.72' Storage=2,142 cf Inflow=2.9 cfs 9,520 Primary=1.2 cfs 8,888 cf Secondary=0.0 cfs 0 cf Outflow=1.2 cfs 8,888	
Pond IC-1:	Peak Elev=286.48' Storage=8,592 cf Inflow=5.3 cfs 20,23 Discarded=0.1 cfs 7,934 cf Primary=0.8 cfs 4,784 cf Outflow=0.9 cfs 12,718	
Pond IC-2:	Peak Elev=289.85' Storage=8,266 cf Inflow=6.8 cfs 26,070 Discarded=0.1 cfs 7,890 cf Primary=3.1 cfs 13,591 cf Outflow=3.2 cfs 21,480	
Link AP-1:	Inflow=15.5 cfs 72,66 Primary=15.5 cfs 72,66	

# Total Runoff Area = 269,982 sf Runoff Volume = 102,496 cf Average Runoff Depth = 4.56" 55.04% Pervious = 148,611 sf 44.96% Impervious = 121,371 sf

#### Summary for Subcatchment SC1.1:

Runoff = 1.4 cfs @ 12.13 hrs, Volume= 4,714 cf, Depth> 3.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN [	Description					
	4,381	61 >	1 >75% Grass cover, Good, HSG B					
	5,583	55 N	Voods, Go	od, HSG B				
	328			ed pavemer				
	3,654			ing, HSG B				
	1,269	96 (	Gravel surfa	ace, HSG E	3			
	15,215		Veighted A					
	11,233	7	′3.83% Pei	vious Area				
	3,982			pervious Ar	ea			
	328	8	8.23% Unco	onnected				
_		~						
ŢĊ	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.4	34	0.2400	0.17		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.12"			
0.9	16	0.1875	0.28		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
1.0	83	0.0361	1.33		Shallow Concentrated Flow,			
<u> </u>	100				Short Grass Pasture Kv= 7.0 fps			
0.5	100	0.0300	3.52		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
5.8	233	Total						

# **Summary for Subcatchment SC1.2:**

Runoff = 3.0 cfs @ 12.16 hrs, Volume= 11,551 cf, Depth> 5.24"

Area (sf)	CN	Description					
6,620	61	>75% Grass cover, Good, HSG B					
2,398	55	Woods, Good, HSG B					
1,172	98	Unconnected pavement, HSG B					
8,000	98	Roofs, HSG B					
7,203	98	Paved parking, HSG B					
1,052	96	Gravel surface, HSG B					
26,444	85	Weighted Average					
10,069		38.08% Pervious Area					
16,375		61.92% Impervious Area					
1,172		7.16% Unconnected					

Page 52

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.9	41	0.2400	0.17		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.12"
	1.4	9	0.0200	0.10		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.12"
	3.4	222	0.0248	1.10		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.3	63	0.0300	3.52		Shallow Concentrated Flow,
_						Paved Kv= 20.3 fps
	9.0	335	Total			

# Summary for Subcatchment SC1.3:

Runoff	=	2.8 cfs @	12.21 hrs.	Volume=	11,836 cf,	Depth>	4.24"
i tanioni				V OTOTTTO	11,000 01,	Dopui	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN E	Description						
	15,029	61 >	>75% Grass cover, Good, HSG B						
	3,894	55 V	Voods, Go	od, HSG B					
	1,741	96 C	Gravel surfa	ace, HSG E	3				
	275	98 L	Jnconnecte	ed pavemer	nt, HSG B				
	12,548	98 F	Paved park	ing, HSG B	3				
	33,487	76 V	Veighted A	verage					
	20,664	6	51.71% Per	vious Area					
	12,823			pervious Ar	ea				
	275	2	2.14% Unco	onnected					
_									
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.2	50	0.0800	0.12		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
3.2	188	0.0372	0.96		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
2.0	160	0.0372	1.35		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.6	122	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
13.0	520	Total							

# **Summary for Subcatchment SC2.1:**

Runoff = 1.3 cfs @ 12.11 hrs, Volume= 5,041 cf, Depth> 6.52"

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description					
	595	61	>75% Gras	s cover, Go	ood, HSG B			
	423	98	Unconnecte	ed pavemer	ent, HSG B			
	4,000	98	Roofs, HSG	βB				
	4,263	98	Paved park	ing, HSG B	3			
	9,281	96	Weighted A	verage				
	595		6.41% Perv	ious Area				
	8,686		93.59% Imp	pervious Are	rea			
	423		4.87% Unco	onnected				
-				0				
Tc	Length	Slop		Capacity	•			
<u>(min)</u>	(feet)	(ft/f	i) (ft/sec)	(cfs)				
5.0					Direct Entry,			

#### Summary for Subcatchment SC2.2:

Runoff = 2.0 cfs @ 12.15 hrs, Volume	e= 7,419 cf, Depth> 4.80"
--------------------------------------	---------------------------

A	rea (sf)	CN D	escription						
	5,529		61 >75% Grass cover, Good, HSG B						
	2,637		,	od, HSG B					
	889			ed paveme	nt, HSG B				
	4,000		loofs, HSG						
	4,580			ing, HSG E					
	929	96 G	Gravel surfa	ace, HSG E	3				
	18,563		Veighted A						
	9,094			vious Area					
	9,469			pervious Ar	ea				
	889	9	.39% Unco	onnected					
т.	المربع مرالم	01		0	Description				
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	<u>(ft/ft)</u>	(ft/sec)	(cfs)					
4.6	50	0.2400	0.18		Sheet Flow,				
0.0	0	0 0000	0.74		Woods: Light underbrush n= 0.400 P2= 3.12"				
0.2	8	0.0200	0.71		Shallow Concentrated Flow,				
0.5	454	0 0000	0.00		Woodland Kv= 5.0 fps				
2.5	151	0.0200	0.99		Shallow Concentrated Flow,				
0.5	96	0.0200	2.87		Short Grass Pasture Kv= 7.0 fps				
0.5	86	0.0200	2.07		Shallow Concentrated Flow, Paved Kv= 20.3 fps				
7.0	205	Total			raveu (v-20.0 1/15				
7.8	295	Total							

#### Summary for Subcatchment SC2.3:

Runoff = 1.2 cfs @ 12.11 hrs, Volume= 3,986 cf, Depth> 4.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

	A	rea (sf)	CN E	CN Description						
		7,218	61 >	75% Gras	s cover, Go	bod, HSG B				
		293	98 L	Inconnecte	ed pavemer	nt, HSG B				
		4,029	98 F	aved park	ing, HSG B	<u>.</u>				
		11,540	75 V	Veighted A	verage					
		7,218	6	2.55% Per	vious Area					
		4,321	3	7.45% Imp	pervious Ar	ea				
		293	6	.77% Unco	onnected					
	Тс	Length	Slope	Velocity	Capacity	Description				
(	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	2.3	42	0.1430	0.31		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.12"				
	0.1	8	0.0500	1.18		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 3.12"				
	0.5	135	0.0500	4.54		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	2.9	185	Total, I	ncreased t	o minimum	Tc = 5.0 min				

# Summary for Subcatchment SC3.1:

Runoff = 1.7 cfs @ 12.11 hrs, Volume= 6,539 cf, Depth> 6.76"

Ar	ea (sf)	CN	Description						
	11,615	98	Roofs, HSG	БB					
	0	61	>75% Gras	s cover, Go	ood, HSG B				
	11,615	98	Weighted A	Weighted Average					
	0		0.00% Pervious Area						
	11,615		100.00% Im	pervious A	Area				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
5.0					Direct Entry,				

#### Summary for Subcatchment SC3.2:

Runoff = 3.6 cfs @ 12.11 hrs, Volume= 13,695 cf, Depth> 6.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN E	Description						
	2,372	61 >	75% Gras	s cover, Go	bod, HSG B				
	21,883	98 F	Paved park	ing, HSG B					
	1,422	98 L	Inconnecte	ed pavemer	nt, HSG B				
	25,677	95 V	Veighted A	verage					
	2,372	ç	.24% Perv	ious Area					
	23,305	ç	0.76% Imp	pervious Ar	ea				
	1,422	6	6.10% Unco	onnected					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
4.4	50	0.0380	0.19		Sheet Flow,				
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
5.2	118	Total							

# Summary for Subcatchment SC3.3:

Runoff = 1.1 cfs @ 12.13 hrs, Volume= 3,662 cf, Depth> 3.00"

 A	rea (sf)	CN E	Description					
	13,540		>75% Grass cover, Good, HSG B					
	1,127	96 (	Gravel surface, HSG B					
	14,666	64 V	Veighted A	verage				
	14,666	1	00.00% Pe	ervious Are	a			
Тс	Length	Slope	Velocity	Capacity	Description			
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.7	50	0.0600	0.23		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
1.9	166	0.0422	1.44		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
5.6	216	Total						

#### Summary for Subcatchment SC4.1:

Runoff = 2.2 cfs @ 12.11 hrs, Volume= 8,444 cf, Depth> 6.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

Area (sf)	CN	Description		
14,998	98	Roofs, HSG	βB	
14,998 100.00% Impervious A				Area
Tc Length (min) (feet)	Slop (ft/f	,	Capacity (cfs)	Description
5.0				Direct Entry,

# Summary for Subcatchment SC4.2:

Runoff = 1.2 cfs @ 12.11 hrs, Volume= 4,097 cf, Depth> 4.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

Area (sf) CN Description					
5,06	50	61 >7	75% Grass	s cover, Go	bod, HSG B
1,45	50	98 U	nconnecte	ed pavemer	nt, HSG B
3,97	75	98 Pa	aved parki	ing, HSG B	
10,48	34	80 W	/eighted A	verage	
5,06	50	48	3.26% Per	vious Area	
5,42	24	5	1.74% Imp	ervious Are	ea
1,450 26.72% Unconnected			6.72% Uno	connected	
Tc Len	gth	Slope	Velocity	Capacity	Description
<u>(min)</u> (fe	et)	(ft/ft)	(ft/sec)	(cfs)	
4.0	50 0	0.0500	0.21		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.4	33 0	0.0500	1.57		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.6 1	148 0	).0440	4.26		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
5.0 2	231 T	otal			

# **Summary for Subcatchment SC4.3:**

Runoff = 0.7 cfs @ 12.11 hrs, Volume= 2,497 cf, Depth> 6.40"

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

A	rea (sf)	CN	N Description					
	406	61	>75% Grass cover, Good, HSG B					
	175	98	Unconnected pavement, HSG B					
	4,101	98	Paved parking, HSG B					
	4,682	95	5 Weighted Average					
	406		8.68% Pervious Area					
	4,276		91.32% Imp	pervious Ar	rea			
	175		4.09% Unc	onnected				
_								
Тс	Length	Slop	,	Capacity	Description			
(min)	(feet)	(ft/f	) (ft/sec)	(cfs)				
5.0					Direct Entry,			

# Summary for Subcatchment SC4.4:

Runoff = 0.7 cfs @ 12.11 hrs, Volume= 2,689 cf, Depth> 6.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN	Description					
	564	61	>75% Grass cover, Good, HSG B					
	541	98	Unconnected pavement, HSG B					
	4,030	98	Paved parking, HSG B					
	5,136	94	Weighted Average					
	564		10.99% Pei	vious Area	3			
	4,571		89.01% Imp	pervious Are	rea			
	541		11.83% Un					
Т	1	01.000	Valasite.	0 it -	Description			
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

# Summary for Subcatchment SC4.5:

Runoff = 0.4 cfs @ 12.12 hrs, Volume= 1,437 cf, Depth> 2.70"

Α	rea (sf)	CN E	CN Description				
	6,391	61 >	61 >75% Grass cover, Good, HSG B				
	6,391	1	100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

#### Summary for Subcatchment SC5.1:

Runoff = 3.8 cfs @ 12.18 hrs, Volume= 14,889 cf, Depth> 2.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN D	escription					
	51,973							
	5,271 70		<ul><li>55 Woods, Good, HSG B</li><li>98 Unconnected pavement, HSG B</li></ul>					
	1,456			ing, HSG B				
	3,033	96 G	Fravel surfa	ace, HSG E				
	61,802 60,277		Veighted A	verage rvious Area				
	1,525			ervious Area				
	70		.56% Unc		_			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description			
3.8	36	0.2000	0.16		Sheet Flow,			
2.1	14	0.0200	0.11		Woods: Light underbrush n= 0.400 P2= 3.12" Sheet Flow,			
2.1	14	0.0200	0.11		Grass: Short $n= 0.150$ P2= 3.12"			
4.1	312	0.0334	1.28		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
10.0	10.0 362 Total							
	Summary for Pond CB-1:							
Inflow A	rea =	15.2 <sup>-</sup>	15 sf. 26. <sup>-</sup>	17% Imperv	/ious, Inflow Depth > 3.72" for 50-Year event			
Inflow	=	1.4 ct	fs @ 12.1	13 hrs, <sup>`</sup> Volı	ume= 4,714 cf			
Outflow	=			13 hrs, Volu				
Primary = 1.4 cfs @ 12.13 hrs, Volume= 4,714 cf								
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2								
	Peak Elev= 298.23' @ 12.13 hrs							
Flood Elev= 301.50'								
Device	Device Routing Invert Outlet Devices							
#1	Primary	2		2.0" Roun				
					PP, projecting, no headwall, Ke= 0.900 Invert= 297.50' / 293.56' S= 0.0232 '/' Cc= 0.900			
					prrugated PE, smooth interior, Flow Area= 0.79 sf			
					-			

Primary OutFlow Max=1.3 cfs @ 12.13 hrs HW=298.21' TW=295.85' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.3 cfs @ 2.26 fps)

# Summary for Pond CB-10:

Inflow Area =	10,484 sf,	51.74% Impervious,	Inflow Depth > 4.69" for 50-Year event
Inflow =	1.2 cfs @	12.11 hrs, Volume=	4,097 cf
Outflow =	1.2 cfs @	12.11 hrs, Volume=	4,097 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.2 cfs @	12.11 hrs, Volume=	4,097 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.83' @ 12.11 hrs Flood Elev= 289.00'

#1 Primary 285.00' <b>12.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 285.00' / 284.79' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=1.2 cfs @ 12.11 hrs HW=285.81' TW=285.59' (Dynamic Tailwater) ↓ 1=Culvert (Outlet Controls 1.2 cfs @ 2.39 fps)

# Summary for Pond CB-2:

Inflow Area =	26,444 sf,	61.92% Impervious,	Inflow Depth > 5.24" for 50-Year event
Inflow =	3.0 cfs @	12.16 hrs, Volume=	11,551 cf
Outflow =	3.0 cfs @	12.16 hrs, Volume=	11,551 cf, Atten= 0%, Lag= 0.0 min
Primary =	3.0 cfs @	12.16 hrs, Volume=	11,551 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 297.07' @ 12.15 hrs Flood Elev= 297.70'

Device	Routing	Invert	Outlet Devices
#1	Primary		<b>12.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.9 cfs @ 12.16 hrs HW=296.93' TW=295.96' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.9 cfs @ 3.74 fps)

# Summary for Pond CB-3:

Inflow Area	=	33,487 sf,	38.29% Impervious,	Inflow Depth > 4.24" for 50-Year event
Inflow :	=	2.8 cfs @	12.21 hrs, Volume=	11,836 cf
Outflow =	=	2.8 cfs @	12.21 hrs, Volume=	11,836 cf, Atten= 0%, Lag= 0.0 min
Primary :	=	2.8 cfs @	12.21 hrs, Volume=	11,836 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 289.44' @ 12.18 hrs Flood Elev= 291.50'

Device R	Routing	Invert	Outlet Devices
-	0	287.50'	<b>12.0" Round Culvert</b> L= 8.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.7 cfs @ 12.21 hrs HW=289.35' TW=288.50' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 2.7 cfs @ 3.49 fps)

# Summary for Pond CB-5:

Inflow Area	a =	18,563 sf,	51.01% Impervious,	Inflow Depth > 4.80" for 50-Year event
Inflow	=	2.0 cfs @	12.15 hrs, Volume=	7,419 cf
Outflow	=	2.0 cfs @	12.15 hrs, Volume=	7,419 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.0 cfs @	12.15 hrs, Volume=	7,419 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.96' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert
			L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.0 cfs @ 12.15 hrs HW=294.96' TW=290.68' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.0 cfs @ 2.63 fps)

# Summary for Pond CB-6:

Inflow Area	=	9,281 sf,	93.59% Impervious,	Inflow Depth > 6.52" for 50-Year event	
Inflow =	=	1.3 cfs @	12.11 hrs, Volume=	5,041 cf	
Outflow =	=	1.3 cfs @	12.11 hrs, Volume=	5,041 cf, Atten= 0%, Lag= 0.0 mir	n
Primary =	=	1.3 cfs @	12.11 hrs, Volume=	5,041 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 296.21' @ 12.11 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	<b>12.0" Round Culvert</b> L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.3 cfs @ 12.11 hrs HW=296.19' TW=290.59' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.3 cfs @ 2.24 fps)

# Summary for Pond CB-7:

Inflow Are	a =	4,682 sf,	91.32% Impervious,	Inflow Depth > 6.40" for 50-Year event
Inflow	=	0.7 cfs @	12.11 hrs, Volume=	2,497 cf
Outflow	=	0.7 cfs @	12.11 hrs, Volume=	2,497 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.7 cfs @	12.11 hrs, Volume=	2,497 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.87' @ 12.29 hrs Flood Elev= 292.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.00'	12.0" Round Culvert
			L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.0 cfs @ 12.11 hrs HW=289.33' TW=289.50' (Dynamic Tailwater) ☐ 1=Culvert (Controls 0.0 cfs)

# Summary for Pond CB-8:

Inflow Area =	5,136 sf,	89.01% Impervious,	Inflow Depth > 6.28" for 50-Year event
Inflow =	0.7 cfs @	12.11 hrs, Volume=	2,689 cf
Outflow =	0.7 cfs @	12.11 hrs, Volume=	2,689 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.7 cfs @	12.11 hrs, Volume=	2,689 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.86' @ 12.30 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.50'	<b>12.0" Round Culvert</b> L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary	/ OutFlow	Max=0.0 cfs @	12.11 hrs HW=289.26' TW=289.40' (Dynamic Tailwater)

**1=Culvert** (Controls 0.0 cfs)

# Summary for Pond CB-9:

Inflow Area =	11,540 sf,37.45% Ir	mpervious, Inflow Depth >	4.14" for 50-Year event
Inflow =	1.2 cfs @ 12.11 hrs	, Volume= 3,986	cf
Outflow =	1.2 cfs @ 12.11 hrs	, Volume= 3,986	cf, Atten= 0%, Lag= 0.0 min
Primary =	1.2 cfs @ 12.11 hrs	, Volume= 3,986	cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.66' @ 12.12 hrs Flood Elev= 291.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.00'	<b>12.0" Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.00' / 284.80' S= 0.0319 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.2 cfs @ 12.11 hrs HW=287.65' TW=285.59' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 1.2 cfs @ 2.16 fps)

# Summary for Pond DCB-4:

Inflow Area	=	25,677 sf,	90.76% Impervious,	Inflow Depth > 6.40"	for 50-Year event
Inflow	=	3.6 cfs @	12.11 hrs, Volume=	13,695 cf	
Outflow	=	3.6 cfs @	12.11 hrs, Volume=	13,695 cf, Att	en= 0%, Lag= 0.0 min
Primary	=	3.6 cfs @	12.11 hrs, Volume=	13,695 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.98' @ 12.12 hrs Flood Elev= 288.50'

Device Rou	uting	Invert	Outlet Devices
	V		<b>12.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.2 cfs @ 12.11 hrs HW=286.87' TW=285.70' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 3.2 cfs @ 4.12 fps)

# Summary for Pond DMH-1:

Inflow Area	a =	41,659 sf,	48.86% Impervious,	Inflow Depth > 4.69" for 50-Year event
Inflow	=	4.3 cfs @	12.15 hrs, Volume=	16,265 cf
Outflow	=	4.3 cfs @	12.15 hrs, Volume=	16,265 cf, Atten= 0%, Lag= 0.0 min
Primary	=	4.3 cfs @	12.15 hrs, Volume=	16,265 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 296.08' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	<b>12.0" Round Culvert</b> L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=4.3 cfs @ 12.15 hrs HW=296.05' TW=288.65' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 4.3 cfs @ 5.49 fps)

#### Summary for Pond DMH-2:

Inflow Area =	75,146 sf, 44.15% Impervious, Inflow Depth > 4.49" for 50-Year even	t
Inflow =	6.8 cfs @ 12.16 hrs, Volume= 28,101 cf	
Outflow =	6.8 cfs @ 12.16 hrs, Volume= 28,101 cf, Atten= 0%, Lag= 0.0 r	nin
Primary =	6.8 cfs @ 12.16 hrs, Volume= 28,101 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.68' @ 12.16 hrs Flood Elev= 291.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.90'	<b>18.0" Round Culvert</b> L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=6.7 cfs @ 12.16 hrs HW=288.64' TW=285.93' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 6.7 cfs @ 3.79 fps)

# **Summary for Pond DMH-3:**

Inflow Area	=	25,677 sf,	90.76% Impervious,	Inflow Depth > 6.40" for 50-Year event	
Inflow =	=	3.6 cfs @	12.11 hrs, Volume=	13,695 cf	
Outflow =	=	3.6 cfs @	12.11 hrs, Volume=	13,694 cf, Atten= 0%, Lag= 0.0 min	
Primary =	=	3.6 cfs @	12.11 hrs, Volume=	13,694 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.49' @ 12.54 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
<u></u> #1	Primary		<b>24.0" Round Culvert</b> L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=3.6 cfs @ 12.11 hrs HW=285.70' TW=285.58' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 3.6 cfs @ 1.33 fps)

# Summary for Pond DMH-5:

Inflow Are	a =	27,844 sf,	65.20% Impervious,	Inflow Depth > 5.37" for 50-Year event
Inflow	=	3.3 cfs @	12.13 hrs, Volume=	12,460 cf
Outflow	=	3.3 cfs @	12.13 hrs, Volume=	12,460 cf, Atten= 0%, Lag= 0.0 min
Primary	=	3.3 cfs @	12.13 hrs, Volume=	12,460 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.70' @ 12.14 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices	
#1	Primary	289.00'	12.0" Round Culvert	
			L= 88.0' CPP, projecting, no headwall, Ke= 0.900	
			Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900	
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

**Primary OutFlow** Max=2.9 cfs @ 12.13 hrs HW=290.64' TW=289.62' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 2.9 cfs @ 3.75 fps)

# Summary for Pond DMH-6:

Inflow Area =	32,526 sf,	68.96% Impervious,	Inflow Depth > 5.52" for 50-Year event
Inflow =	3.9 cfs @	12.13 hrs, Volume=	14,958 cf
Outflow =	3.9 cfs @	12.13 hrs, Volume=	14,958 cf, Atten= 0%, Lag= 0.0 min
Primary =	3.9 cfs @	12.13 hrs, Volume=	14,958 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.88' @ 12.24 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	<b>24.0" Round Culvert</b> L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf
<b>.</b> .			

Primary OutFlow Max=3.9 cfs @ 12.13 hrs HW=289.60' TW=289.50' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.9 cfs @ 1.23 fps) 203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D50-Year Rainfall=7.00"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 65

# Summary for Pond DMH-7:

Inflow Area =	5,136 sf, 89.01% Impervious,	Inflow Depth > 6.28" for 50-Year event
Inflow =	0.7 cfs @ 12.11 hrs, Volume=	2,689 cf
Outflow =	0.7 cfs @ 12.11 hrs, Volume=	2,675 cf,Atten= 0%,Lag= 0.0 min
Primary =	0.7 cfs @ 12.11 hrs, Volume=	2,675 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.85' @ 12.25 hrs Flood Elev= 292.40'

#1 Drimony 287.60' 24.0" Bound Culvert	Device	Routing	Invert	Outlet Devices
L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf	#1	Primary	287.60'	Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900

Primary OutFlow Max=0.7 cfs @ 12.11 hrs HW=289.40' TW=289.40' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 0.7 cfs @ 0.24 fps)

#### Summary for Pond DMH-9:

Inflow Area	a =	22,024 sf,	44.25% Impervious,	Inflow Depth > 4.40" for 50-Year event	
Inflow	=	2.4 cfs @	12.11 hrs, Volume=	8,083 cf	
Outflow	=	2.4 cfs @	12.11 hrs, Volume=	8,083 cf, Atten= 0%, Lag= 0.0 m	nin
Primary	=	2.4 cfs @	12.11 hrs, Volume=	8,083 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.61' @ 12.11 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.70'	<b>15.0" Round Culvert</b> L= 185.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.70' / 282.00' S= 0.0146 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.4 cfs @ 12.11 hrs HW=285.59' TW=283.48' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 2.4 cfs @ 2.53 fps)

# Summary for Pond IB-1:

Inflow Area =	89,812 sf,	36.94% Impervious,	Inflow Depth > 4.24" for 50-Year event
Inflow =	7.9 cfs @	12.16 hrs, Volume=	31,763 cf
Outflow =	7.7 cfs @	12.17 hrs, Volume=	31,494 cf, Atten= 2%, Lag= 1.1 min
Discarded =	0.0 cfs @	12.17 hrs, Volume=	984 cf
Primary =	1.4 cfs @	12.17 hrs, Volume=	22,824 cf
Secondary =	6.2 cfs @	12.17 hrs, Volume=	7,685 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 285.94' @ 12.17 hrs Surf.Area= 1,368 sf Storage= 1,344 cf

Plug-Flow detention time= 12.5 min calculated for 31,494 cf (99% of inflow) Center-of-Mass det. time= 7.5 min ( 842.8 - 835.3 )

Volume	Invert	Avail.S	Storage	Storage Description	n	
#1	284.00'	3	,366 cf	Custom Stage Da	ata (Irregular)Liste	d below (Recalc)
Elevativ		unf Anna a		In a Ctara	Curra Starra	
Elevatio		urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
284.0	00	208	72.5	0	0	208
285.0	00	661	129.3	413	413	1,126
286.0	00	1,425	189.1	1,019	1,432	2,649
287.0	00	2,492	246.3	1,934	3,366	4,643
Device	Routing	Inve	rt Outle	et Devices		
#1	Discarded	284.0	0' <b>1.02</b>	0 in/hr Exfiltration	over Surface area	a
#2	Secondary	285.50	0' <b>8.0'</b> I	long x 15.0' bread	th Broad-Crested	Rectangular Weir
	,			d (feet) 0.20 0.40		
				. (English) 2.68 2.		
#3	Primary	284.5		Round Culvert	10 2.10 2.01 2.00	2.01 2.01 2.00
110	. many	20110		0.0' CPP, projectir	na na headwall K	e= 0 900
						0.0667 '/' Cc= 0.900
						0.06677 CC = 0.900
			n= 0	.013, Flow Area= 0	1.35 ST	

**Discarded OutFlow** Max=0.0 cfs @ 12.17 hrs HW=285.93' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

**Primary OutFlow** Max=1.4 cfs @ 12.17 hrs HW=285.93' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 1.4 cfs @ 3.98 fps)

Secondary OutFlow Max=6.0 cfs @ 12.17 hrs HW=285.93' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 6.0 cfs @ 1.77 fps)

# Summary for Pond IB-2:

Inflow Area =	28,415 sf,	34.30% Impervious,	Inflow Depth > 4.02" for 50-Year event
Inflow =	2.9 cfs @	12.12 hrs, Volume=	9,520 cf
Outflow =	1.2 cfs @	12.24 hrs, Volume=	8,888 cf, Atten= 57%, Lag= 7.5 min
Primary =	1.2 cfs @	12.24 hrs, Volume=	8,888 cf
Secondary =	0.0 cfs @	0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 283.72' @ 12.24 hrs Surf.Area= 1,693 sf Storage= 2,142 cf

Plug-Flow detention time= 69.6 min calculated for 8,870 cf (93% of inflow) Center-of-Mass det. time= 33.5 min ( 872.3 - 838.8 )

Page 67

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Volume	Invert	Avai	.Storage	Storage Description	า	
#1	282.00'		4,789 cf	Custom Stage Dat	t <b>a (Irregular)</b> Listed	below (Recalc)
Elevatio (fee 282.0 283.0 284.0 285.0	≥t) 20 20 20	urf.Area (sq-ft) 834 1,315 1,855 2,452	Perim. (feet) 150.9 170.2 189.4 208.4	Inc.Store (cubic-feet) 0 1,065 1,577 2,147	Cum.Store (cubic-feet) 0 1,065 2,643 4,789	Wet.Area (sq-ft) 834 1,353 1,931 2,564
Device	Routing	Inv	vert Outle	et Devices		
#1	Secondary	284		long x 22.0' bread		
#2	Primary	282.	Coet 50' <b>8.0''</b> L= 3 Inlet	d (feet) 0.20 0.40 0 f. (English) 2.68 2.7 <b>Round Culvert</b> 7.4' CPP, projecting / Outlet Invert= 282. .013 Corrugated PE	0 2.70 2.64 2.63 g, no headwall, Ke 50' / 278.50' S= 0	2.64 2.64 2.63 = 0.900 .1070 '/' Cc= 0.900

Primary OutFlow Max=1.2 cfs @ 12.24 hrs HW=283.71' TW=0.00' (Dynamic Tailwater) ←2=Culvert (Inlet Controls 1.2 cfs @ 3.57 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=282.00' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

# Summary for Pond IC-1:

Inflow Area =	37,292 sf, 93.64% Impervious,	Inflow Depth > 6.51" for 50-Year event
Inflow =	5.3 cfs @ 12.11 hrs, Volume=	20,233 cf
Outflow =	0.9 cfs @ 12.54 hrs, Volume=	12,718 cf, Atten= 83%, Lag= 25.7 min
Discarded =	0.1 cfs @ 7.25 hrs, Volume=	7,934 cf
Primary =	0.8 cfs @ 12.54 hrs, Volume=	4,784 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.48' @ 12.54 hrs Surf.Area= 4,404 sf Storage= 8,592 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 62.4 min ( 819.3 - 756.9 )

Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
-		9 501 cf	Total Available Storage

9,501 cf Total Available Storage

Storage Group A created with Chamber Wizard

203 Ayer Road, Harvard, MA NRCC 24-hr D 50-Year Rainfall=7.00" Printed 3/9/2022 tions LLC Page 68

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
			L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.1 cfs @ 7.25 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=0.8 cfs @ 12.54 hrs HW=286.48' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 0.8 cfs of 1.6 cfs potential flow) 3=Orifice/Grate (Orifice Controls 0.8 cfs @ 2.37 fps)

# Summary for Pond IC-2:

Inflow Area =	52,660 sf,	79.76% Impervious,	Inflow Depth > 5.94" for 50-Year event
Inflow =	6.8 cfs @	12.12 hrs, Volume=	26,076 cf
Outflow =	3.2 cfs @	12.25 hrs, Volume=	21,480 cf, Atten= 53%, Lag= 7.8 min
Discarded =	0.1 cfs @	6.90 hrs, Volume=	7,890 cf
Primary =	3.1 cfs @	12.25 hrs, Volume=	13,591 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.85' @ 12.25 hrs Surf.Area= 4,361 sf Storage= 8,266 cf

Plug-Flow detention time= 129.1 min calculated for 21,436 cf (82% of inflow) Center-of-Mass det. time= 44.2 min (817.1 - 772.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows
		0.414 of	Total Available Storage

9,414 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
			L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.1 cfs @ 6.90 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=3.1 cfs @ 12.25 hrs HW=289.85' TW=0.00' (Dynamic Tailwater) -2=Culvert (Inlet Controls 3.1 cfs @ 3.89 fps) -3=Orifice/Grate (Passes 3.1 cfs of 3.4 cfs potential flow)

# Summary for Link AP-1:

Inflow Area	a =	269,982 sf,	44.96% Impervious,	Inflow Depth >	3.23"	for 50-Year event
Inflow	=	15.5 cfs @	12.18 hrs, Volume=	72,662	cf	
Primary	=	15.5 cfs @	12.18 hrs, Volume=	72,662	cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

	203 Ayer Road, Harvard, MA
POST	NRCC 24-hr D 100-Year Rainfall=8.34"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 3/9/2022
HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Sol	lutions LLC Page 70

# Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>4.87" Flow Length=233' Tc=5.8 min CN=71 Runoff=1.8 cfs 6,174 cf
SubcatchmentSC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>6.53" Flow Length=335' Tc=9.0 min CN=85 Runoff=3.7 cfs 14,386 cf
SubcatchmentSC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>5.45" Flow Length=520' Tc=13.0 min CN=76 Runoff=3.5 cfs 15,206 cf
SubcatchmentSC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>7.85" Tc=5.0 min CN=96 Runoff=1.6 cfs 6,074 cf
Subcatchment SC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>6.05" Flow Length=295' Tc=7.8 min CN=81 Runoff=2.6 cfs 9,363 cf
Subcatchment SC2.3:	Runoff Area=11,540 sf 37.45% Impervious Runoff Depth>5.34" Flow Length=185' Tc=5.0 min CN=75 Runoff=1.5 cfs 5,139 cf
SubcatchmentSC3.1:	Runoff Area=11,615 sf  100.00% Impervious  Runoff Depth>8.09" Tc=5.0 min  CN=98  Runoff=2.0 cfs  7,834 cf
SubcatchmentSC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>7.73" Flow Length=118' Slope=0.0380 '/' Tc=5.2 min CN=95 Runoff=4.4 cfs 16,547 cf
Subcatchment SC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>4.05" Flow Length=216' Tc=5.6 min CN=64 Runoff=1.5 cfs 4,948 cf
SubcatchmentSC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>8.09" Tc=5.0 min CN=98 Runoff=2.6 cfs 10,116 cf
SubcatchmentSC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>5.94" Flow Length=231' Tc=5.0 min CN=80 Runoff=1.5 cfs 5,188 cf
Subcatchment SC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>7.73" Tc=5.0 min CN=95 Runoff=0.8 cfs 3,018 cf
SubcatchmentSC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>7.61" Tc=5.0 min CN=94 Runoff=0.9 cfs 3,258 cf
SubcatchmentSC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>3.70" Tc=5.0 min CN=61 Runoff=0.6 cfs 1,971 cf
SubcatchmentSC5.1:	Runoff Area=61,802 sf 2.47% Impervious Runoff Depth>3.93" Flow Length=362' Tc=10.0 min CN=63 Runoff=5.2 cfs 20,216 cf
Pond CB-1:	Peak Elev=298.38' Inflow=1.8 cfs 6,174 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=1.8 cfs 6,174 cf

<b>POST</b> Prepared by Goldsmith, <u>HydroCAD® 10.10-4a_s/n 0</u> 2	203 Ayer Road, Harvard, MA NRCC 24-hr D 100-Year Rainfall=8.34" Prest & Ringwall, Inc. Printed 3/9/2022 1036 © 2020 HydroCAD Software Solutions LLC Page 71
Pond CB-10:	Peak Elev=286.02' Inflow=1.5 cfs 5,188 cf 12.0" Round Culvert n=0.013 L=21.0' S=0.0100 '/' Outflow=1.5 cfs 5,188 cf
Pond CB-2:	Peak Elev=298.79' Inflow=3.7 cfs 14,386 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0200 '/' Outflow=3.7 cfs 14,386 cf
Pond CB-3:	Peak Elev=290.53' Inflow=3.5 cfs 15,206 cf 12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/' Outflow=3.5 cfs 15,206 cf
Pond CB-5:	Peak Elev=295.23' Inflow=2.6 cfs 9,363 cf 12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/' Outflow=2.6 cfs 9,363 cf
Pond CB-6:	Peak Elev=296.29' Inflow=1.6 cfs 6,074 cf 12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/' Outflow=1.6 cfs 6,074 cf
Pond CB-7:	Peak Elev=290.53' Inflow=0.8 cfs 3,018 cf 12.0" Round Culvert n=0.013 L=22.0' S=0.0136 '/' Outflow=0.8 cfs 3,018 cf
Pond CB-8:	Peak Elev=290.51' Inflow=0.9 cfs 3,258 cf 12.0" Round Culvert n=0.013 L=15.0' S=0.0200 '/' Outflow=0.9 cfs 3,258 cf
Pond CB-9:	Peak Elev=287.78' Inflow=1.5 cfs 5,139 cf 12.0" Round Culvert n=0.013 L=69.0' S=0.0319 '/' Outflow=1.5 cfs 5,139 cf
Pond DCB-4:	Peak Elev=288.10' Inflow=4.4 cfs 16,547 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/' Outflow=4.4 cfs 16,547 cf
Pond DMH-1:	Peak Elev=297.28' Inflow=5.4 cfs 20,560 cf 12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/' Outflow=5.4 cfs 20,560 cf
Pond DMH-2:	Peak Elev=289.29' Inflow=8.6 cfs 35,765 cf 18.0" Round Culvert n=0.013 L=48.0' S=0.0100 '/' Outflow=8.6 cfs 35,765 cf
Pond DMH-3:	Peak Elev=287.13' Inflow=4.4 cfs 16,547 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=4.4 cfs 16,547 cf
Pond DMH-5:	Peak Elev=291.74' Inflow=4.1 cfs 15,437 cf 12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/' Outflow=4.1 cfs 15,437 cf
Pond DMH-6:	Peak Elev=290.55' Inflow=4.8 cfs 18,455 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=4.8 cfs 18,455 cf
Pond DMH-7:	Peak Elev=290.50' Inflow=0.9 cfs 3,258 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=0.9 cfs 3,254 cf
Pond DMH-9:	Peak Elev=285.76' Inflow=3.1 cfs 10,327 cf 15.0" Round Culvert n=0.013 L=185.0' S=0.0146 '/' Outflow=3.1 cfs 10,327 cf
<b>Pond IB-1:</b> Discarded=0.0 cfs 1,067 cf	Peak Elev=286.03' Storage=1,473 cf Inflow=10.0 cfs 40,713 cf Primary=1.5 cfs 27,835 cf Secondary=8.3 cfs 11,524 cf Outflow=9.8 cfs 40,426 cf

<b>POST</b> Prepared by Goldsmith, F <u>HydroCAD® 10.10-4a s/n 010</u>	203 Ayer Road, Harvard, MA NRCC 24-hr D 100-Year Rainfall=8.34" Prest & Ringwall, Inc. Printed 3/9/2022 Printed 3/9/2022 Page 72
Pond IB-2:	Peak Elev=284.03' Storage=2,693 cf Inflow=3.7 cfs 12,298 cf Primary=1.4 cfs 11,623 cf Secondary=0.1 cfs 25 cf Outflow=1.6 cfs 11,648 cf
Pond IC-1:	Peak Elev=287.12' Storage=9,501 cf Inflow=6.4 cfs 24,381 cf Discarded=0.1 cfs 8,222 cf Primary=2.2 cfs 8,512 cf Outflow=2.3 cfs 16,734 cf
Pond IC-2:	Peak Elev=290.50' Storage=9,410 cf Inflow=8.2 cfs 31,825 cf Discarded=0.1 cfs 8,163 cf Primary=3.9 cfs 18,986 cf Outflow=4.0 cfs 27,150 cf
Link AP-1:	Inflow=21.7 cfs 98,722 cf Primary=21.7 cfs 98,722 cf

# Total Runoff Area = 269,982 sf Runoff Volume = 129,438 cf Average Runoff Depth = 5.75" 55.04% Pervious = 148,611 sf 44.96% Impervious = 121,371 sf

#### Summary for Subcatchment SC1.1:

Runoff = 1.8 cfs @ 12.13 hrs, Volume= 6,174 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

A	rea (sf)	CN I	Description					
	4,381	61 >	61 >75% Grass cover, Good, HSG B					
	5,583	55 \	Noods, Go	od, HSG B				
	328	98 l	Jnconnecte	ed pavemer	nt, HSG B			
	3,654	98 I	Paved park	ing, HSG B	3			
	1,269	96 (	Gravel surfa	ace, HSG E	3			
	15,215	71 \	Neighted A	verage				
	11,233	7	73.83% Pei	vious Area				
	3,982		26.17% Imp	pervious Are	ea			
	328	8	3.23% Unco	onnected				
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.4	34	0.2400	0.17		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.12"			
0.9	16	0.1875	0.28		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
1.0	83	0.0361	1.33		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.5	100	0.0300	3.52		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
5.8	233	Total						

# **Summary for Subcatchment SC1.2:**

Runoff = 3.7 cfs @ 12.16 hrs, Volume= 14,386 cf, Depth> 6.53"

Area (sf)	CN	Description			
6,620	61	>75% Grass cover, Good, HSG B			
2,398	55	Woods, Good, HSG B			
1,172	98	Unconnected pavement, HSG B			
8,000	98	Roofs, HSG B			
7,203	98	Paved parking, HSG B			
1,052	96	Gravel surface, HSG B			
26,444	85	Weighted Average			
10,069		38.08% Pervious Area			
16,375		61.92% Impervious Area			
1,172		7.16% Unconnected			

\_

Page 74

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.9	41	0.2400	0.17		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
1.4	9	0.0200	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
3.4	222	0.0248	1.10		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	63	0.0300	3.52		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
9.0	335	Total			

# Summary for Subcatchment SC1.3:

Runoff	=	3.5 cfs @	12.21 hrs,	Volume=	15,206 cf,	Depth>	5.45"
--------	---	-----------	------------	---------	------------	--------	-------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

_	A	rea (sf)	CN	Description					
		15,029	61	61 >75% Grass cover, Good, HSG B					
		3,894	55	Woods, Go	od, HSG B				
		1,741	96	Gravel surfa	ace, HSG E	3			
		275		Unconnecte					
_		12,548	98	Paved park	ing, HSG E	}			
		33,487		Weighted A					
		20,664		61.71% Pei					
		12,823		38.29% Imp		ea			
		275	2.14% Unconnected						
	Тс	Length	Slope	e Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)		(cfs)	Description			
_	7.2	50	0.0800	0.12		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 3.12"			
	3.2	188	0.0372	0.96		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	2.0	160	0.0372	. 1.35		Shallow Concentrated Flow,			
	• •	1.0-5				Short Grass Pasture Kv= 7.0 fps			
	0.6	122	0.0300	3.52		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	13.0	520	Total						

#### 13.0 520 Total

# Summary for Subcatchment SC2.1:

Runoff	=	1.6 cfs @	12.11 hrs, Volume=	6,074 cf, Depth> 7.85"
--------	---	-----------	--------------------	------------------------

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Area (sf)	CN	Description							
595	61	>75% Grass cover, Good, HSG B							
423	98	Unconnected pavement, HSG B							
4,000	98	Roofs, HSG B							
4,263	98	Paved parking, HSG B							
9,281	96	Weighted Average							
595		6.41% Pervious Area							
8,686		93.59% Impervious Area							
423		4.87% Unconnected							
Tc Lengtl									
(min) (feet	:) (ft/	/ft) (ft/sec) (cfs)							
5.0		Direct Entry,							

#### **Summary for Subcatchment SC2.2:**

Runoff	=	2.6 cfs @	12.15 hrs, Volume=	9,363 cf, Depth> 6.05"
--------	---	-----------	--------------------	------------------------

A	rea (sf)	CN D	escription						
	5,529		61 >75% Grass cover, Good, HSG B						
	2,637			od, HSG B					
	889	98 U	Inconnecte	ed pavemer	nt, HSG B				
	4,000	98 R	loofs, HSG	βB					
	4,580	98 P	aved park	ing, HSG B	}				
	929	96 G	Fravel surfa	ace, HSG E	3				
	18,563	81 V	Veighted A	verage					
	9,094	4	8.99% Per	vious Area	l				
	9,469	5	1.01% Imp	pervious Ar	ea				
	889	9	.39% Unco	onnected					
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
4.6	50	0.2400	0.18		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
0.2	8	0.0200	0.71		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
2.5	151	0.0200	0.99		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.5	86	0.0200	2.87		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
7.8	295	Total							

#### Summary for Subcatchment SC2.3:

Runoff = 1.5 cfs @ 12.11 hrs, Volume= 5,139 cf, Depth> 5.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

Area (sf)	CN [	Description		
7,218	61 >	>75% Gras	s cover, Go	bod, HSG B
293	98 l	Jnconnecte	ed pavemei	nt, HSG B
4,029	98 F	Paved park	ing, HSG E	3
11,540	75 \	Neighted A	verage	
7,218	6	32.55% Per	vious Area	
4,321	3	37.45% Imp	pervious Ar	ea
293	6	6.77% Unco	onnected	
Tc Length	Slope		Capacity	Description
(min) (feet)	(ft/ft)	(ft/sec)	(cfs)	
2.3 42	0.1430	0.31		Sheet Flow,
				Grass: Short n= 0.150 P2= 3.12"
0.1 8	0.0500	1.18		Sheet Flow,
				Smooth surfaces n= 0.011 P2= 3.12"
0.5 135	0.0500	4.54		Shallow Concentrated Flow,
				Paved Kv= 20.3 fps
2.9 185	Total,	Increased t	o minimum	1 Tc = 5.0 min

# Summary for Subcatchment SC3.1:

Runoff = 2.0 cfs @ 12.11 hrs, Volume= 7,834 cf, Depth> 8.09"

Ar	ea (sf)	CN	Description					
	11,615	98	Roofs, HSG	БB				
	0	61	>75% Gras	s cover, Go	ood, HSG B			
	11,615	98	Weighted A	verage				
	0		0.00% Perv	ious Area				
	11,615		100.00% Im	pervious A	Area			
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description			
5.0					Direct Entry,			

#### Summary for Subcatchment SC3.2:

Runoff = 4.4 cfs @ 12.11 hrs, Volume= 16,547 cf, Depth> 7.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

A	rea (sf)	CN E	Description						
	2,372	61 >	75% Gras	s cover, Go	bod, HSG B				
	21,883	98 F	Paved park	ing, HSG B					
	1,422	98 L	Inconnecte	ed pavemer	nt, HSG B				
	25,677	95 V	Veighted A	verage					
	2,372	ç	.24% Perv	ious Area					
	23,305	ç	0.76% Imp	pervious Ar	ea				
	1,422	6	6.10% Unco	onnected					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
4.4	50	0.0380	0.19		Sheet Flow,				
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
5.2	118	Total							

# Summary for Subcatchment SC3.3:

Runoff = 1.5 cfs @ 12.13 hrs, Volume= 4,948 cf, Depth> 4.05"

 A	rea (sf)	CN E	Description					
	13,540				ood, HSG B			
	1,127	96 (	Gravel surfa	ace, HSG E	}			
	14,666	64 V	Veighted A	verage				
	14,666	1	00.00% Pe	ervious Are	a			
_								
Тс	Length	Slope	Velocity	Capacity	Description			
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.7	50	0.0600	0.23		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
1.9	166	0.0422	1.44		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
5.6	216	Total						

#### Summary for Subcatchment SC4.1:

Runoff = 2.6 cfs @ 12.11 hrs, Volume= 10,116 cf, Depth> 8.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

Area (sf)	CN	Description					
14,998	98	Roofs, HSC	βB				
14,998		100.00% Impervious Area					
Tc Length (min) (feet)	Slop (ft/fl		Capacity (cfs)				
5.0				Direct Entry,			

# Summary for Subcatchment SC4.2:

Runoff = 1.5 cfs @ 12.11 hrs, Volume= 5,188 cf, Depth> 5.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

Area (s	sf) (	CN D	Description					
5,06	50	61 >7	75% Grass	s cover, Go	bod, HSG B			
1,45	50	98 U	nconnecte	ed pavemer	nt, HSG B			
3,97	75	98 Pa	aved parki	ing, HSG B				
10,48	34	80 W	/eighted A	verage				
5,06	50	48	3.26% Per	vious Area				
5,42	24	5	1.74% Imp	ervious Are	ea			
1,45	50	26	6.72% Uno	connected				
Tc Len	gth	Slope	Velocity	Capacity	Description			
<u>(min)</u> (fe	et)	(ft/ft)	(ft/sec)	(cfs)				
4.0	50 0	0.0500	0.21		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.12"			
0.4	33 0	0.0500	1.57		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.6 1	148 0	).0440	4.26		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
5.0 2	231 T	otal						

# **Summary for Subcatchment SC4.3:**

Runoff = 0.8 cfs @ 12.11 hrs, Volume= 3,018 cf, Depth> 7.73"

Prepared by Goldsmith, Prest & Ringwall, Inc.PrintHydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPrint

A	rea (sf)	CN	Description						
	406	61	>75% Gras	s cover, Go	ood, HSG B				
	175	98	Unconnecte	ed pavemer	nt, HSG B				
	4,101	98	Paved park	ing, HSG B					
	4,682	95	Weighted A	verage					
	406		8.68% Perv	ious Area					
	4,276		91.32% Imp	pervious Ar	ea				
	175		4.09% Unc	onnected					
Тс	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft		(cfs)	ı				
5.0					Direct Entry,				

# Summary for Subcatchment SC4.4:

Runoff = 0.9 cfs @ 12.11 hrs, Volume= 3,258 cf, Depth> 7.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

Α	rea (sf)	CN	Description					
	564	61	>75% Gras	s cover, Go	ood, HSG B			
	541	98	Unconnecte	ed pavemer	ent, HSG B			
	4,030	98	Paved park	ing, HSG B	3			
	5,136	94	Weighted A	verage				
	564		10.99% Pei	vious Area	a			
	4,571		89.01% Imp	pervious Ar	rea			
	541		11.83% Un					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
5.0					Direct Entry,			

# Summary for Subcatchment SC4.5:

Runoff = 0.6 cfs @ 12.12 hrs, Volume= 1,971 cf, Depth> 3.70"

Α	rea (sf)	CN E	Description						
	6,391	61 >	>75% Grass cover, Good, HSG B						
	6,391	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

#### Summary for Subcatchment SC5.1:

Runoff = 5.2 cfs @ 12.18 hrs, Volume= 20,216 cf, Depth> 3.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

Area (sf) CN Description					
51,973 61 >75% Grass cover, Good, HSG B	61 >75% Grass cover, Good, HSG B				
	55 Woods, Good, HSG B				
70 98 Unconnected pavement, HSG B					
1,456 98 Paved parking, HSG B 3,033 96 Gravel surface, HSG B					
61,802 63 Weighted Average					
60,277 97.53% Pervious Area					
1,525 2.47% Impervious Area					
704.56% Unconnected					
Tc Length Slope Velocity Capacity Description					
(min) (feet) (ft/ft) (ft/sec) (cfs)					
3.8 36 0.2000 0.16 <b>Sheet Flow,</b>					
Woods: Light underbrush n= 0.400 P2= 3.1	2"				
2.1 14 0.0200 0.11 <b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.12"					
4.1 312 0.0334 1.28 Shallow Concentrated Flow,					
Short Grass Pasture Kv= 7.0 fps					
10.0 362 Total					
Oursement for David OD 4					
Summary for Pond CB-1:					
Inflow Area = 15,215 sf, 26.17% Impervious, Inflow Depth > 4.87" for 100-Year ev	ent				
Inflow = 1.8 cfs @ 12.13 hrs, Volume = 6,174 cf					
Outflow = 1.8 cfs @ 12.13 hrs, Volume= 6,174 cf, Atten= 0%, Lag= 0.0	min				
Primary = 1.8 cfs @ 12.13 hrs, Volume= 6,174 cf					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2					
Peak Elev= 298.38' @ 12.13 hrs					
Flood Elev= 301.50'					
Device Routing Invert Outlet Devices					
#1 Primary 297.50' <b>12.0" Round Culvert</b>					
L = 170.0' CPP, projecting, no headwall, Ke= 0.900					
Inlet / Outlet Invert= 297.50' / 293.56' S= 0.0232 '/' Cc= 0	.900				

Primary OutFlow Max=1.8 cfs @ 12.13 hrs HW=298.35' TW=296.92' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.8 cfs @ 2.48 fps)

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

# Summary for Pond CB-10:

Inflow Area =	10,484 sf, 51.74% Impervious, I	nflow Depth > 5.94" for 100-Year event
Inflow =	1.5 cfs @ 12.11 hrs, Volume=	5,188 cf
Outflow =	1.5 cfs @ 12.11 hrs, Volume=	5,188 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.5 cfs @ 12.11 hrs, Volume=	5,188 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.02' @ 12.11 hrs Flood Elev= 289.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.00'	<b>12.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 285.00' / 284.79' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.5 cfs @ 12.11 hrs HW=285.99' TW=285.74' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 1.5 cfs @ 1.91 fps)

# Summary for Pond CB-2:

Inflow Area	=	26,444 sf,	61.92% Impervious,	Inflow Depth > 6.53" for 100-Year event
Inflow	=	3.7 cfs @	12.16 hrs, Volume=	14,386 cf
Outflow	=	3.7 cfs @	12.16 hrs, Volume=	14,386 cf, Atten= 0%, Lag= 0.0 min
Primary	=	3.7 cfs @	12.16 hrs, Volume=	14,386 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 298.79' @ 12.15 hrs Flood Elev= 297.70'

<u>Device</u> R	Routing	Invert	Outlet Devices
#1 P	Primary		<b>12.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.6 cfs @ 12.16 hrs HW=298.57' TW=297.11' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.6 cfs @ 4.60 fps)

# Summary for Pond CB-3:

Inflow Area	a =	33,487 sf,	38.29% Impervious,	Inflow Depth > 5.45" for 100-Year event
Inflow	=	3.5 cfs @	12.21 hrs, Volume=	15,206 cf
Outflow	=	3.5 cfs @	12.21 hrs, Volume=	15,206 cf, Atten= 0%, Lag= 0.0 min
Primary	=	3.5 cfs @	12.21 hrs, Volume=	15,206 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 290.53' @ 12.18 hrs Flood Elev= 291.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.50'	<b>12.0" Round Culvert</b> L= 8.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.5 cfs @ 12.21 hrs HW=290.39' TW=289.01' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 3.5 cfs @ 4.46 fps)

# Summary for Pond CB-5:

Inflow Area	a =	18,563 sf,	51.01% Impervious,	Inflow Depth > 6.05" for 100-Year event
Inflow	=	2.6 cfs @	12.15 hrs, Volume=	9,363 cf
Outflow	=	2.6 cfs @	12.15 hrs, Volume=	9,363 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.6 cfs @	12.15 hrs, Volume=	9,363 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 295.23' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert
			L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.5 cfs @ 12.15 hrs HW=295.22' TW=291.71' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.5 cfs @ 3.22 fps)

# Summary for Pond CB-6:

Inflow Area	=	9,281 sf,	93.59% Impervious,	Inflow Depth > 7.85" for 100-Year e	vent
Inflow =	=	1.6 cfs @	12.11 hrs, Volume=	6,074 cf	
Outflow =	=	1.6 cfs @	12.11 hrs, Volume=	6,074 cf, Atten= 0%, Lag= 0.	0 min
Primary =	=	1.6 cfs @	12.11 hrs, Volume=	6,074 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 296.29' @ 12.11 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	<b>12.0" Round Culvert</b> L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.6 cfs @ 12.11 hrs HW=296.28' TW=291.34' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.6 cfs @ 2.37 fps)

# Summary for Pond CB-7:

Inflow Area =	4,682 sf, 91.32% Impervious,	Inflow Depth > 7.73" for 100-Year event
Inflow =	0.8 cfs @ 12.11 hrs, Volume=	3,018 cf
Outflow =	0.8 cfs @ 12.11 hrs, Volume=	3,018 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.8 cfs @ 12.11 hrs, Volume=	3,018 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.53' @ 12.29 hrs Flood Elev= 292.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.00'	12.0" Round Culvert
			L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.0 cfs @ 12.11 hrs HW=289.70' TW=289.97' (Dynamic Tailwater) ↓ 1=Culvert (Controls 0.0 cfs)

# Summary for Pond CB-8:

Inflow Area =	5,136 sf,	89.01% Impervious,	Inflow Depth > 7.61" for 100-Year event
Inflow =	0.9 cfs @	12.11 hrs, Volume=	3,258 cf
Outflow =	0.9 cfs @	12.11 hrs, Volume=	3,258 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.9 cfs @	12.11 hrs, Volume=	3,258 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.51' @ 12.29 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.50'	<b>12.0" Round Culvert</b> L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary	<b>OutFlow</b>	Max=0.0 cfs @	12.11 hrs HW=289.57' TW=289.83' (Dynamic Tailwater)

1=Culvert (Controls 0.0 cfs)

203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D 100-Year Rainfall=8.34"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 84

# Summary for Pond CB-9:

Inflow Area =	11,540 sf, 37.45% Impervious,	Inflow Depth > 5.34" for 100-Year event
Inflow =	1.5 cfs @ 12.11 hrs, Volume=	5,139 cf
Outflow =	1.5 cfs @ 12.11 hrs, Volume=	5,139 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.5 cfs @ 12.11 hrs, Volume=	5,139 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.78' @ 12.11 hrs Flood Elev= 291.00'

#1 Primary 287.00' <b>12.0'' Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.00' / 284.80' S= 0.0319 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=1.5 cfs @ 12.11 hrs HW=287.76' TW=285.74' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 1.5 cfs @ 2.34 fps)

# Summary for Pond DCB-4:

Inflow Area	=	25,677 sf,	90.76% Impervious,	Inflow Depth > 7.73" for 100-Year event
Inflow	=	4.4 cfs @	12.11 hrs, Volume=	16,547 cf
Outflow	=	4.4 cfs @	12.11 hrs, Volume=	16,547 cf, Atten= 0%, Lag= 0.0 min
Primary	=	4.4 cfs @	12.11 hrs, Volume=	16,547 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.10' @ 12.13 hrs Flood Elev= 288.50'

Device	Routing	Invert	Outlet Devices
<u></u> #1	Primary		<b>12.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
			$\mathbf{c}$

Primary OutFlow Max=3.8 cfs @ 12.11 hrs HW=288.00' TW=286.38' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.8 cfs @ 4.84 fps)

# Summary for Pond DMH-1:

Inflow Area	ı =	41,659 sf,	48.86% Impervious,	Inflow Depth > 5.92" for 100-Year event
Inflow	=	5.4 cfs @	12.15 hrs, Volume=	20,560 cf
Outflow	=	5.4 cfs @	12.15 hrs, Volume=	20,560 cf, Atten= 0%, Lag= 0.0 min
Primary	=	5.4 cfs @	12.15 hrs, Volume=	20,560 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

# 203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D100-Year Rainfall=8.34"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 85

Peak Elev= 297.28' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	<b>12.0" Round Culvert</b> L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=5.4 cfs @ 12.15 hrs HW=297.22' TW=289.24' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 5.4 cfs @ 6.86 fps)

#### Summary for Pond DMH-2:

Inflow Area =	75,146 sf, 44.15% Imperviou	us, Inflow Depth > 5.71" for 100-Year event
Inflow =	8.6 cfs @ 12.16 hrs, Volum	e= 35,765 cf
Outflow =	8.6 cfs @ 12.16 hrs, Volum	e= 35,765 cf, Atten= 0%, Lag= 0.0 min
Primary =	8.6 cfs @ 12.16 hrs, Volum	e= 35,765 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.29' @ 12.16 hrs Flood Elev= 291.70'

Device Routing Invert Outlet Devices	
#1 Primary 286.90' <b>18.0'' Round Culvert</b> L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf	

Primary OutFlow Max=8.5 cfs @ 12.16 hrs HW=289.23' TW=286.02' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 8.5 cfs @ 4.78 fps)

# **Summary for Pond DMH-3:**

Inflow Area =	25,677 sf,	90.76% Impervious,	Inflow Depth > 7.73" for 100-Year event
Inflow =	4.4 cfs @	12.11 hrs, Volume=	16,547 cf
Outflow =	4.4 cfs @	12.11 hrs, Volume=	16,547 cf, Atten= 0%, Lag= 0.0 min
Primary =	4.4 cfs @	12.11 hrs, Volume=	16,547 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.13' @ 12.25 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.10'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.2 cfs @ 12.11 hrs HW=286.38' TW=286.26' (Dynamic Tailwater) -1=Culvert (Inlet Controls 4.2 cfs @ 1.34 fps)

# Summary for Pond DMH-5:

Inflow Area =	27,844 sf, 65.20% Impervious,	Inflow Depth > 6.65" for 100-Year event
Inflow =	4.1 cfs @ 12.13 hrs, Volume=	15,437 cf
Outflow =	4.1 cfs @ 12.13 hrs, Volume=	15,437 cf, Atten= 0%, Lag= 0.0 min
Primary =	4.1 cfs @ 12.13 hrs, Volume=	15,437 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 291.74' @ 12.15 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	289.00'	<b>12.0" Round Culvert</b> L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Primary OutFlow** Max=3.5 cfs @ 12.13 hrs HW=291.57' TW=290.17' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 3.5 cfs @ 4.39 fps)

# Summary for Pond DMH-6:

Inflow Area =	32,526 sf, 6	68.96% Impervious,	Inflow Depth > 6.81" for 100-Year event
Inflow =	4.8 cfs @ 1	12.13 hrs, Volume=	18,455 cf
Outflow =	4.8 cfs @ 1	12.13 hrs, Volume=	18,455 cf, Atten= 0%, Lag= 0.0 min
Primary =	4.8 cfs @ 1	12.13 hrs, Volume=	18,455 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.55' @ 12.23 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	<b>24.0" Round Culvert</b> L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.6 cfs @ 12.13 hrs HW=290.14' TW=289.99' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 4.6 cfs @ 1.48 fps) 203 Ayer Road, Harvard, MAPOSTNRCC 24-hr D 100-Year Rainfall=8.34"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 3/9/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 87

# Summary for Pond DMH-7:

Inflow Area =	5,136 sf, 89.01% Impervious, Inflow Depth > 7.61" for 100-Ye	ear event
Inflow =	0.9 cfs @ 12.11 hrs, Volume= 3,258 cf	
Outflow =	0.9 cfs @ 12.11 hrs, Volume= 3,254 cf, Atten= 0%, Lag	g= 0.0 min
Primary =	0.9 cfs @ 12.11 hrs, Volume= 3,254 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.50' @ 12.24 hrs Flood Elev= 292.40'

#1 Drimony 287.60' 24.0" Bound Culvert	Device	Routing	Invert	Outlet Devices
L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf	#1	Primary	287.60'	Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900

Primary OutFlow Max=0.9 cfs @ 12.11 hrs HW=289.83' TW=289.82' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 0.9 cfs @ 0.27 fps)

## Summary for Pond DMH-9:

Inflow Area	a =	22,024 sf,	44.25% Impervious,	Inflow Depth > 5.63" for 100-Year event
Inflow	=	3.1 cfs @	12.11 hrs, Volume=	10,327 cf
Outflow	=	3.1 cfs @	12.11 hrs, Volume=	10,327 cf, Atten= 0%, Lag= 0.0 min
Primary	=	3.1 cfs @	12.11 hrs, Volume=	10,327 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.76' @ 12.11 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.70'	<b>15.0" Round Culvert</b> L= 185.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.70' / 282.00' S= 0.0146 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.0 cfs @ 12.11 hrs HW=285.74' TW=283.71' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.0 cfs @ 2.74 fps)

# Summary for Pond IB-1:

Inflow Area =	89,812 sf,	36.94% Impervious,	Inflow Depth > 5.44" for 100-Year event
Inflow =	10.0 cfs @	12.15 hrs, Volume=	40,713 cf
Outflow =	9.8 cfs @	12.17 hrs, Volume=	40,426 cf, Atten= 2%, Lag= 1.1 min
Discarded =	0.0 cfs @	12.17 hrs, Volume=	1,067 cf
Primary =	1.5 cfs @	12.17 hrs, Volume=	27,835 cf
Secondary =	8.3 cfs @	12.17 hrs, Volume=	11,524 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 286.03' @ 12.17 hrs Surf.Area= 1,451 sf Storage= 1,473 cf

Plug-Flow detention time= 11.1 min calculated for 40,426 cf (99% of inflow) Center-of-Mass det. time= 6.8 min ( 833.7 - 826.8 )

Volume	Invert	Avail.S	Storage	Storage Descriptio	n	
#1	284.00' 3,366 cf		Custom Stage Data (Irregular)Listed below (Recalc)			
	-		<b>.</b> .			
Elevatio		urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
284.0	00	208	72.5	0	0	208
285.0	00	661	129.3	413	413	1,126
286.0	00	1,425	189.1	1,019	1,432	2,649
287.0	00	2,492	246.3	1,934	3,366	4,643
Device	Routing	Invert Outl		et Devices		
#1	Discarded	284.0	0' <b>1.02</b>	0 in/hr Exfiltration	over Surface area	l
#2	Secondary	285.50' 8.0'		3.0' long x 15.0' breadth Broad-Crested Rectangular Weir		
	2			d (feet) 0.20 0.40		
				. (English) 2.68 2.		
#3	Primary	284.5		Round Culvert		
	,			0.0' CPP, projectin	ng, no headwall, Ke	e= 0.900
						0.0667 '/' Cc= 0.900
				.013, Flow Area= 0		

**Discarded OutFlow** Max=0.0 cfs @ 12.17 hrs HW=286.02' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

**Primary OutFlow** Max=1.4 cfs @ 12.17 hrs HW=286.02' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 1.4 cfs @ 4.14 fps)

Secondary OutFlow Max=8.0 cfs @ 12.17 hrs HW=286.02' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 8.0 cfs @ 1.94 fps)

# Summary for Pond IB-2:

Inflow Area =	28,415 sf,	34.30% Impervious,	Inflow Depth > 5.19" for 100-Year event
Inflow =	3.7 cfs @	12.11 hrs, Volume=	12,298 cf
Outflow =	1.6 cfs @	12.25 hrs, Volume=	11,648 cf, Atten= 57%, Lag= 8.1 min
Primary =	1.4 cfs @	12.25 hrs, Volume=	11,623 cf
Secondary =	0.1 cfs @	12.25 hrs, Volume=	25 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 284.03' @ 12.25 hrs Surf.Area= 1,870 sf Storage= 2,693 cf

Plug-Flow detention time= 61.3 min calculated for 11,623 cf (95% of inflow) Center-of-Mass det. time= 31.8 min ( 861.7 - 829.8 ) POST

Page 89

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Volume	Invert	Avail	.Storage	Storage Description	า	
#1	282.00'		4,789 cf	Custom Stage Dat	ta (Irregular)Listed	below (Recalc)
Elevatio (fee 282.0 283.0 284.0 285.0	et) 00 00 00	urf.Area (sq-ft) 834 1,315 1,855 2,452	Perim. (feet) 150.9 170.2 189.4 208.4	Inc.Store (cubic-feet) 0 1,065 1,577 2,147	Cum.Store (cubic-feet) 0 1,065 2,643 4,789	Wet.Area (sq-ft) 834 1,353 1,931 2,564
Device	Routing	Inv	vert Outle	et Devices		
#1	Secondary	0		long x 22.0' bread		
#2	Primary	Hea Coe 282.50' <b>8.0''</b> L= 3 Inlet		lead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63 <b>.0" Round Culvert</b> = 37.4' CPP, projecting, no headwall, Ke= 0.900 hlet / Outlet Invert= 282.50' / 278.50' S= 0.1070 '/' Cc= 0.900 = 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf		

Primary OutFlow Max=1.4 cfs @ 12.25 hrs HW=284.03' TW=0.00' (Dynamic Tailwater) ←2=Culvert (Inlet Controls 1.4 cfs @ 4.15 fps)

Secondary OutFlow Max=0.1 cfs @ 12.25 hrs HW=284.03' TW=0.00' (Dynamic Tailwater) —1=Broad-Crested Rectangular Weir (Weir Controls 0.1 cfs @ 0.44 fps)

# Summary for Pond IC-1:

Inflow Area =	37,292 sf,	93.64% Impervious,	Inflow Depth > 7.85" for 100-Year event
Inflow =	6.4 cfs @	12.11 hrs, Volume=	24,381 cf
Outflow =	2.3 cfs @	12.26 hrs, Volume=	16,734 cf, Atten= 64%, Lag= 8.6 min
Discarded =	0.1 cfs @	5.90 hrs, Volume=	8,222 cf
Primary =	2.2 cfs @	12.26 hrs, Volume=	8,512 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.12' @ 12.26 hrs Surf.Area= 4,404 sf Storage= 9,501 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 56.9 min ( 809.9 - 753.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
		9 501 cf	Total Available Storage

9,501 cf Total Available Storage

Storage Group A created with Chamber Wizard

POST

Prepared by Goldsmith, Prest & Ringwall, Inc. HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
			L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.1 cfs @ 5.90 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=2.2 cfs @ 12.26 hrs HW=287.10' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 2.2 cfs of 2.8 cfs potential flow) 3=Orifice/Grate (Orifice Controls 2.2 cfs @ 3.97 fps)

# Summary for Pond IC-2:

Inflow Area =	52,660 sf,	79.76% Impervious,	Inflow Depth > 7.25" for 100-Year event
Inflow =	8.2 cfs @	12.12 hrs, Volume=	31,825 cf
Outflow =	4.0 cfs @	12.24 hrs, Volume=	27,150 cf, Atten= 51%, Lag= 7.4 min
Discarded =	0.1 cfs @	5.55 hrs, Volume=	8,163 cf
Primary =	3.9 cfs @	12.24 hrs, Volume=	18,986 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.50' @ 12.24 hrs Surf.Area= 4,361 sf Storage= 9,410 cf

Plug-Flow detention time= 120.2 min calculated for 27,150 cf (85% of inflow) Center-of-Mass det. time= 44.6 min (813.4 - 768.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows
		0.414 of	Total Available Storage

9,414 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
			L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

**Discarded OutFlow** Max=0.1 cfs @ 5.55 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=3.9 cfs @ 12.24 hrs HW=290.49' TW=0.00' (Dynamic Tailwater) -2=Culvert (Inlet Controls 3.9 cfs @ 4.94 fps) -3=Orifice/Grate (Passes 3.9 cfs of 4.3 cfs potential flow)

# Summary for Link AP-1:

Inflow Area	a =	269,982 sf,	44.96% Impervious,	Inflow Depth > 4.39"	for 100-Year event
Inflow	=	21.7 cfs @	12.19 hrs, Volume=	98,722 cf	
Primary	=	21.7 cfs @	12.19 hrs, Volume=	98,722 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

**Pre-Development Conditions** 

203 Ayer Road, Harvard, MA Project No. 211009

		<u>Area (sf)</u>	<u>Area (Ac)</u>	
Total Subcatchment Areas		269,982	6.2	
Total Subcatchment Areas On-Site		269,982	6.2	
Total Area of Hydrolic Soil Groups On-Site	В	269,982	6.2	
Surface Type Areas				
Gras	s <u> </u>	183,615	4.2	
Woods	s <u> </u>	33,694	0.8	
Grave	el <u> </u>	52,672	1.2	
Total Impervious Area		0	0.0	
Infiltration Volume				
Inches of Recharge per Storm Event	A B C D	0.60 0.35 0.25 0.10		

Infiltration Volume =  $\sum \{ [(Total Subcatchment Area within HSG) - (Total Impervious Area within HSG) \} \}$ x (inches of Recharge Per Storm)}

Infiltration Volume

7,874	CF
-------	----

Post Development Conditions			203 Ayer Road, Harvard, MA Project No. 211009
		<u>Area (sf)</u>	<u>Area (Ac)</u>
Total Subcatchment Areas		269,982	6.2
Total Subcatchment Areas On-Site		269,982	6.2
Total Area of Hydrolic Soil Groups On-Site	В	269,982	6.2
Surface Type Areas Grass	В	119,677	2.7
 Gravel	В	9,151	0.2
Woods_	В	19,783	0.5
Unconnected pavement	В	7,036	0.2
Paved parking	В	71,721	1.6
Roofs_	В	42,613	1.0
Total Impervious Area		121,370	2.8
Infiltration Volume			
Inches of Recharge per Storm Event	A B C D	0.60 0.35 0.25 0.10	
Infiltration Volume = $\sum \{ [(Total Subcatchment Article Artic$	ea wit		mpervious Area within HSG)] hes of Recharge Per Storm)}
Natural Infiltration Volume		4,335	CF
Pre-Development Infiltration Volume		7,874	CF
Required Iniltration Volume		3,540	CF
Provided Infiltration Volume			
Infiltration Chambers (IC-1) Infiltration Chambers (IC-2) Infiltration Basin (IB-1) Infiltration Basin (IB-2)		7,531 4,409 150 473	<ul> <li>CF Volume below 286.00' Orifice</li> <li>CF Volume below 288.50' Orifice</li> <li>CF Volume below 284.50' Orifice</li> <li>CF Volume below 282.50' Orifice</li> </ul>
Total Provided Iniltration Volume		12,563	CF

#### **Infiltration Area Requirements**

#### **Drawdown Time**

(Per Massachusetts Stormwater regulations, infiltration areas must completely drain within 72 hours)

		Infiltration Chambers (IC-1)	Infiltration Chambers (IC-2)
Infiltration Area Storage Volume	cf	7,531	4,409
Design infiltration Rate	in/hr	1.02	1.02
Infiltration Bottom Area	sf	4,404	4,362

Drawdown Time = Infiltration Area Storage Volume / [Design Infiltration Rate x Infiltration Area Bottom Area]

Drawdown Time (Hrs)	20.1	11.9
---------------------	------	------

## **Mounding Analysis**

Per the Massachusetts Stormwater Handbook, mounding analysis is required when ".. The vertical separation from the bottom of an exfiltration system to seasonal high groundwater is less than four (4) feet and the recharge system is proposed to attenuate the peak discharge from a 10-year or higher 24-hour storm." The mounding analysis "... must show that the REQUIRED RECHARGE VOLUME is fully dewatered within 72 hours..."

		Infiltration Chambers (IC-1)	Infiltration Chambers (IC-2)		
Hydraulic Conductivity	ft/day	16	16		
	-	Lower Range Standard Value for "Me	edium Sand" material		
Specific Yield		0.23	0.23		
		Standard Value for "Medium Sand" material			
Initial Saturated Thickness	ft	10	10		
		Depth to bedrock			
Design Recharge Rate	ft/day	2.04	2.04		
		infiltration rate			
Time	days	3	3		
		Minimum 72 hr evaluation period			
Bottom Infiltrating Area	sf	4,404	4,362		
Length of Infiltration Area	ft	81.94	110.42		
Width of Infiltration Area	ft	53.7	39.5		
Time when Infiltration Stops	days	0.84	0.50		
		Calculated Drawdown Time (see Abo	ove)		
Maximum Water table rise at 72 hours <sup>1</sup>	ft	1.27	0.74		
	in	15 1/4	8 7/8		

#### - Resulting mound will not interfere with the full draining of the infiltration area in accordance with Mass Stormwater Standards -

<sup>1</sup>- mounding analysis calculated using the Hantush (1967) method. Automated report generated by MOUNDSOLV V2.1 (www.aqtesolv.com) by HydroSOLVE, Inc.

#### **Infiltration Area Requirements**

### **Drawdown Time**

(Per Massachusetts Stormwater regulations, infiltration areas must completely drain within 72 hours)

		Infiltration Basin (IB-1)	Infiltration Basin (IB-2)
Infiltration Area Storage Volume	cf	150	473
Design infiltration Rate	in/hr	1.02	1.02
Infiltration Bottom Area	sf	208	834

Drawdown Time = Infiltration Area Storage Volume / [Design Infiltration Rate x Infiltration Area Bottom Area]

Drawdown Time (Hrs)	8.5	6.7
---------------------	-----	-----

## **Mounding Analysis**

Per the Massachusetts Stormwater Handbook, mounding analysis is required when ".. The vertical separation from the bottom of an exfiltration system to seasonal high groundwater is less than four (4) feet and the recharge system is proposed to attenuate the peak discharge from a 10-year or higher 24-hour storm." The mounding analysis "... must show that the REQUIRED RECHARGE VOLUME is fully dewatered within 72 hours..."

		Infiltration Basin (IB-1)	Infiltration Basin (IB-2)			
Hydraulic Conductivity	ft/day	16	16			
	-	Lower Range Standard Value for "Me	dium Sand" material			
Specific Yield		0.23	0.23			
		Standard Value for "Medium Sand" material				
Initial Saturated Thickness	ft	10	10			
		Depth to bedrock				
Design Recharge Rate	ft/day	2.04	2.04			
		infiltration rate				
Time	days	3	3			
		Minimum 72 hr evaluation period				
Bottom Infiltrating Area	sf	208	834			
Length of Infiltration Area	ft	31	12.67			
Width of Infiltration Area	ft	6.7	65.8			
Time when Infiltration Stops	days	0.35	0.28			
		Calculated Drawdown Time (see Abo	ve)			
Maximum Water table rise at 72 hours <sup>1</sup>	ft	0.03	0.08			
	in	1/4	1			

### - Resulting mound will not interfere with the full draining of the infiltration area in accordance with Mass Stormwater Standards -

<sup>1</sup>- mounding analysis calculated using the Hantush (1967) method. Automated report generated by MOUNDSOLV V2.1 (www.aqtesolv.com) by HydroSOLVE, Inc.

# Stormwater Management Standard 4 WATER QUALITY RETENTION VOLUME

Parameter	Unit	Quantity	Remarks
Watershed area	sf	269,982	
Predevelopment impervious area Total impervious area added Total impervious area Total impervious area required for retention	sf sf sf sf	0 121,370 121,370 121,370	_
Runoff depth over impervious area	IN	0.5	
Required Water Quality Volume	CF	5,057	
Provided Water Quality Volume			
Infiltration Chambers (IC-1) Infiltration Chambers (IC-2) Infiltration Basin (IB-1) Infiltration Basin (IB-2)		7,531 4,409 150 473	CFVolume below 286.00' OrificeCFVolume below 288.50' OrificeCFVolume below 284.50' OrificeCFVolume below 282.50' Orifice
DESIGN VOLUME PROVIDED	CF	12,563	

## **Stormwater Management Standard 4 TSS REMOVAL**

Process Train No.	Impervious Area (SF)	ВМР Туре	TSS Removal Rate	TSS Remaining at Discharge	TSS Removed at Discharge
SC1.1, SC1.2, SC1.3, SC3.3	33,180	СВ	25%	75%	25%
		FB-1 / IB-1	80%	15%	85%
SC3.1, SC3.2	37,292	СВ	25%	75%	25%
		IC-1	80%	15%	85%
SC2.1, SC2.2, SC4.1, SC4.3, SC4.4	42,001	СВ	25%	75%	25%
		IC-2	80%	15%	85%
SC2.3, SC4.2, SC4.5	4,471	СВ	25%	75%	25%
		FB-2 / IB-2	80%	15%	85%

ABBREVIATIONS: TSS=total suspended solids; SF=square feet; SC=subcatchment; GC=grassed channel; BMP=best management practices; CB=deep CB=deep sump hooded catch basin; FB = Sediment Forebay; IB=infiltration basin; IC=infiltration chambers