Stormwater Management Report

203 Ayer Road Harvard, MA

March 2022 Revision-1 (7/25/2022)

<u>Submitted to:</u> Harvard Planning Board & Conservation Commission 13 Ayer Road Harvard, MA 01451



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> <u>Project No:</u> 211009





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"Commercial Development - Ayer Road Village Special Permit Application - 203 Ayer Road, Harvard, MA" Dated March 2022. Last Revised July 25, 2022

"Commercial Development - Notice of Intent - 203 Ayer Road, Harvard, MA" Dated March 2022.

Long-Term Pollution Prevention Plan & Stormwater System Operation and Maintenance Plan, Dated March 2022.

Section 1

Introduction and Methodology

Introduction and Methodology

This Stormwater Management Report is intended to accompany plans for the proposed Ayer Road Village Special Permit Application, Commercial Development at 203 Ayer Road in Harvard. Included in this report are calculations that support a final engineering design as required by the state's Wetlands Protection Act Regulations and the Town of Harvard's ordinances and regulations. Site specific information is presented under two scenarios, "pre-development" and "post-development" conditions, so that potential impacts due to the project can be identified, quantified and, as necessary, mitigated.

The final design intent seeks to meet the following interrelated goals:

- 1. Limit stormwater runoff rates and volume for the 2-, 10-, 50- and 100-year storm events to existing (pre-development) levels;
- Reduce the runoff rates and volume for the 2-, 10-year storm events by 5% of the existing (pre-development) levels per Harvard's Stormwater Regulations;
- 3. Maintain the volume of stormwater recharged per storm event to those of existing (pre-development) levels;
- Prevent appreciable sediment and other suspended solids and contaminants transport by trapping them on site via Best Management Practices;
- 5. Provide adequate drainage for new surfaces;
- 6. Maintain existing drainage patterns while providing a cost-effective engineering solution that addresses regulatory as well as real-world constraints.

Site Description

The proposed project is commercial development to be filed under the Town of Harvard Ayer Road Village Special Permit application. The 11.3± acre project site is located on 203 Ayer Road and designated as Parcel 8-62-002 by the Town of Harvard Assessor. The project area is a 7.95± portion located on the northeastern side of the project site.

The project area is primarily low brush vegetations with some wooded areas. There is an existing hydrologically connected Bordering Vegetated Wetlands (BVW) located at the northeastern corner of the project site. There is an existing 24" culvert located at the northeastern corner of the wetland area going under Ayer Road and allowing stormwater to discharge into a bigger wetland system east of the project site. The project area generally slopes towards the hydrologically connected Bordering Vegetated Wetlands (BVW) located at the northeastern corner of the project site.

Available NRCS soils mapping for the project shows diverse soils, ranging from Hydrologic Soil Group A-C. Hydraulic Soil Group (HSG) A consisting approximately 1/3 of the project area, designated as Merrimac fine sandy loam, is located on the southern portion of the project area. Hydraulic Soil Group B consisting of approximately 1/3 of the project area, designated as Sudbury fine sandy loam, is located on the middle portion of the project area. Hydraulic Soil Group C consisting of the remaining 1/3 of the project area, designated as Walpole sandy loam, is located on the northern portion of the project area. Onsite soil evaluations were performed, and the logs reveal that the mapping is typically consistent with the field evaluation. Although the Hydraulic Soil Group for the project is generally split into three areas as mentioned above, the area containing HSG B is the largest portion within the project area. Proposed stormwater management systems will generally be located within this area of HSG B, therefore HSG B will be used for analysis purposes.

Under the pre-development scenario, the project area will be viewed as a single subcatchment area. As shown on the plan entitled "PRE-DEVELOPMENT – WATERSHED MAP", included within the attached Appendix, subcatchment SC1.0 outlines the project area flowing towards the proposed wetland replication, as shown as Analysis Point AP-1.

Project Description

The proposed development will construct three commercial use buildings with parking, pedestrian access, and a driveway connection on Ayer Road. The three proposed commercial-use buildings, their associated parking areas and driveway access onto Ayer Road are shown in the attached Site Plan. Building A will be constructed as a recreational center specifically for the purpose of providing court space for badminton. The badminton center will have a gross floor area of 29,998 sq. ft. The proposed development will provide 120 parking spaces, five of which shall be handicap accessible parking spaces, for the recreational center. Building B and C are shown as general commercial/office-use buildings that will be subjected to final layout design once an end-user has been identified.

In order to offset the increase in stormwater runoff on site, various stormwater Best Management Practices (BMP) are proposed. On-site stormwater BMPs are designed to capture and treat stormwater, provide groundwater recharge, and reduce stormwater runoff than existing conditions.

Under the post-development scenario, the project has been divided into a total of 15 subcatchment areas, shown on the plan entitled "POST-DEVELOPMENT – WATERSHED MAP", and included in the attached Appendix, outlining runoff to the AP-1.

Subcatchment SC1.1, SC1.2, SC1.3, and SC3.3 outline stormwater runoff that will get collected by deep sump hooded catch basins and discharge into sediment forebay No. 1 prior to continuing into Infiltration Basin (IB-1). IB-1 will have an 8" HDPE discharge pipe and an emergency overflow weir to allow stormwater to continue into the resource area.

Subcatchment SC3.1 outlines stormwater runoff from the western portion of the proposed roof area that will directly discharge into Infiltration Chambers (IC-1). SC3.2 outlines runoff from the proposed pavement area that will get collected by a deep sump hooded catch basin and discharge

Subcatchment SC3.1 outlines stormwater runoff from the western portion of the proposed roof area that will directly discharge into Infiltration Chambers (IC-1). SC3.2 outlines runoff from the proposed pavement area that will get collected by a deep sump hooded catch basin and discharge into an Isolator Row for IC-1. IC-1 will have a discharge pipe to allow stormwater to continue into the resource area.

Subcatchment SC2.1, SC2.2, SC4.2 and SC4.4 outline stormwater runoff that will get collected by deep sump hooded catch basins and discharge into an Isolator Row for Infiltration Chambers (IC-2). SC4.1 outlines stormwater runoff from the eastern portion of the proposed roof area that will directly discharge into Infiltration Chambers (IC-2). IC-2 will have a discharge pipe to allow stormwater to continue into the resource area.

Subcatchment SC2.3, SC4.2 and SC4.5 outline stormwater runoff that will get collected by deep sump hooded catch basins and discharge into Infiltration Basin (IB-2). IB-2 will have a couple of discharge pipes to allow stormwater to continue into the resource area.

Subcatchment SC5.1 outlines untreated stormwater runoff from pervious surface. As mentioned above, SC5.1 will exclude the proposed wetland replication area that was previously approved by the Town of Harvard Conservation Commission.

Hydrologic and Hydraulic Computation Methodology

Runoff rates were computed using the Soil Conservation Service TR-20 Method entitled "Urban Hydrology for Small Watersheds" within the HydroCAD Stormwater Modeling software platform. The following 24-hour rainfall events from the Northeast Regional Climate Center (NRCC) Extreme Precipitation Tables database were analyzed:

Frequency (years): 2, 10, 50 and 100

As outlined above, runoff from the site has been analyzed at one point under the pre-development and post-development conditions. As a standard for comparison AP-1 is represented in both the pre and the post development cases.

Summary of Results

Peak discharge rates and volumes of the calculated runoff for both conditions analyzed are displayed in the HYDROLOGY SUMMARY that follows. As shown within the summary, the peak discharge rates and volumes at Analysis Point AP-1 for all analyzed storm events are less than or equal to those under pre-development conditions. Peak discharge rates and volumes for the 2- and 10-yr storm events under the proposed conditions are also at least five percent lower than the existing conditions, in accordance with the Town of Harvard's Stormwater Management Regulations.

The deep sump hooded catch basins, Isolator Row, infiltration chambers, sediment forebay and infiltration basins work together to provide an expected Total Suspended Solids (TSS) removal of 85% for each of the treatment train, prior to discharging into the proposed wetland replication area.

The two sets of infiltration chambers will retain and infiltrate 12,898 cubic feet of runoff prior to discharging, well in excess of the minimum required 3,540 cubic feet occurring under existing conditions and displaced by the proposed development. The proposed infiltration volume will also be in excess of the required Water Quality Volume generated by the proposed impervious area on site.

The proposed development meets the MADEP Stormwater Management Standards through the use of Best Management Practices that address groundwater recharge, water quality (first flush) retention, and suspended solids removal within sustainable BMP's. See Appendix for computed solids quantities / removal process trains, and water quality runoff volumes.

Section 2

Hydrology Summary for 24-hour Storm

HYDROLOGY SUMMARY FOR 24-HOUR STORM

203 Ayer Road, Harvard, MA Project No. 211009

PEAK DISCHARGE RATE

Pre-Development (cfs)

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	2.8	7.9	17.2	22.9
5% Reduction [1]	2.7	7.5		

Development (cfs)

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	2.1	7.0	15.8	22.0

Pre-Development vs. Developed (cfs)

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	-0.6	-0.5	-1.4	-0.9

PEAK DISCHARGE VOLUME

Pre-Development (Cubic feet)

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	14,651	35,658	74,457	99,179
5% Reduction [1]	13,918	33,875		

Development (Cubic feet)

Analysis Point	2-YR	10-YR	50-YR	100-YR
AP-1	12,110	32,146	72,751	98,834

Pre-Development vs. Developed (Cubic feet)

Analysis Point	2-YR	10-YR	25-YR	100-YR
AP-1	-1,808	-1,729	-1,706	-345

[1] Harvard's Wetlands Protection Bylaw Regulations: Chapter 147-14.C.(1) Stormwater Management: Peak runoff rates and volume under proposed conditions must be reduced by at least five percent compared to existing conditions for the 2- and 10-yr frequency storm event, and peak runoff rates and volumes under proposed conditions compared to existing conditions shall not exceed existing peak runoff rates and volume for the 50- and 100-year frequency storm events.

Section 3

Mass DEP Stormwater Management Report Checklist



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



Nulula ignature and Date

7/29/2022

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

New development

Redevelopment

Mix of New Development and Redevelopment



LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

\boxtimes	No disturbance to any Wetland Resource Areas
\boxtimes	Site Design Practices (e.g. clustered development, reduced frontage setbacks)
	Reduced Impervious Area (Redevelopment Only)
\square	Minimizing disturbance to existing trees and shrubs
	LID Site Design Credit Requested:
	Credit 1
	Credit 2
	Credit 3
	Use of "country drainage" versus curb and gutter conveyance and pipe
	Bioretention Cells (includes Rain Gardens)
	Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
	Treebox Filter
	Water Quality Swale
	Grass Channel
	Green Roof
\boxtimes	Other (describe): Direct roof recharge

Standard 1: No New Untreated Discharges

- \boxtimes No new untreated discharges
- \boxtimes Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

Calculations provided to show that post-development peak discharge rates do not exceed predevelopment rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24hour storm.

Standard 3: Recharge

Soil Analysis provided.

- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.

🖂 Static

Dynamic Field¹

 \boxtimes Runoff from all impervious areas at the site discharging to the infiltration BMP.

Simple Dynamic

Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

Recharge BMPs have been sized to infiltra	ate the Required Recharge Volume.
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Recharge BMPs have been sized to infiltrate the Required Recharge Volume only to the maximum
extent practicable for the following reason:

- Site is comprised solely of C and D soils and/or bedrock at the land surface
- M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- Solid Waste Landfill pursuant to 310 CMR 19.000
- Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- \boxtimes Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - is within the Zone II or Interim Wellhead Protection Area
 - is near or to other critical areas
 - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - involves runoff from land uses with higher potential pollutant loads.
- The Required Water Quality Volume is reduced through use of the LID site Design Credits.
- Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist	(continued))
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Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
 - The ½" or 1" Water Quality Volume or
 - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does *not* cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

Limited	Pro	ject
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- Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- Bike Path and/or Foot Path
- Redevelopment Project
- Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is *not* covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - Name of the stormwater management system owners;
 - Party responsible for operation and maintenance;
 - Schedule for implementation of routine and non-routine maintenance tasks;
 - Plan showing the location of all stormwater BMPs maintenance access areas;
 - Description and delineation of public safety features;
 - Estimated operation and maintenance budget; and
 - Operation and Maintenance Log Form.
- The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted *prior to* the discharge of any stormwater to post-construction BMPs.

Section 4

<u>Appendix</u>



I) Interest (AOI) Interest (AOI) Stony Spot Number of the stone Stony Spot Number of the stone Net Stone	The soil surveys that comprise your AOI were mapped at 1:20,000. Spot Spot Warning: Soil Map may not be valid at this scale. Enlargement of maps beyond the scale of mapping can ca misunderstanding of the detail of mapping and accuracy of ine placement. The maps do not show the small areas of contrasting soils that could have been shown at a more de scale. d Canals Please rely on the bar scale on each map sheet for map measurements. ighways Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
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rsh or swamp	projection, which preserves direction and shape but distorts
	ography Albers equal-area conic projection, should be used if more
ie or Quarry	accurate calculations of distance or area are required.
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/erely Eroded Spot	Soil map units are labeled (as space allows) for map scales
khole	
le or Slip	Date(s) aerial images were photographed: Aug 12, 2019—S
dic Spot	24, 2018

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
6A	Scarboro mucky fine sandy loam, 0 to 3 percent slopes	0.5	2.3%
31A	Walpole sandy loam, 0 to 3 percent slopes	3.4	15.2%
254A	Merrimac fine sandy loam, 0 to 3 percent slopes	4.2	18.9%
254B	Merrimac fine sandy loam, 3 to 8 percent slopes	3.9	17.5%
260B	Sudbury fine sandy loam, 3 to 8 percent slopes	8.1	36.2%
307C	Paxton fine sandy loam, 8 to 15 percent slopes, extremely stony	1.9	8.6%
311B	Woodbridge fine sandy loam, 0 to 8 percent slopes, very stony	0.3	1.3%
Totals for Area of Interest		22.3	100.0%

National Flood Hazard Layer FIRMette

°35'6"W 42°31'50"N







legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for elements do not appear: basemap imagery, flood zone labels,

regulatory purposes.

No. 211009

Date: 1/17/22

Commonwealth of Massachusetts Harvard, Massachusetts

Soil Suitability Assessment for Stormwater Management

Performed by: Limhuot Tiv, GPR Inc	Date: 1/13/22
Witnessed by: N/A	
Location Address:	Owner's Name: Wheeler Realty Trust
or Lot No. 203 Ayer Road	Address: 198 Ayer Road
Harvard, MA	Harvard, MA 01451
	Telephone No.
New Construction 🗹 Upgrade 🔲 Repa	ir 🗖
Office Review	-
Published Soil Survey Available: No 🔲 Ye	s M
Year Published Internet Publication Scale	e n/a Soil Map Unit 254A, 260B & 31A
Soil Name Merrimac fine sandy loam Soil Limitations	well drained
Soil Name Sudbury fine sandy loam Soil Limitations	well drained, shallow to groundwater
Soil Name Wapole sandy loam Soil Limitations	poorly drained, shallow to groundwater
Surficial Geologic Report Available: No Ves	
Year Published Mass Mapper Publication Scale	n/a
Geologic Material(Map Unit) Sand and gravel, Till or	bedrock
Landform Ground Morraine	
Fleed Insurance Data Many 25027C0214E	
<u>Flood insurance Rate Map:</u> 2502/C0314E	~
Within 500 Year Flood Boundary No Fe	
Within 100 Year Flood Boundary No Ver	
Within Velocity Zone No Ve	
Wetland Area	
National Wetlands Inventory Man (man unit) N/A	
Wetlands Conservancy Program Man (man unit) N/A	
Current Water Resource Conditions (USGS): Month	February
Range: Above Normal	mal
Other Reference Reviewed USGS	

Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-1	Date:	01/13/22 Time:		8:00AM	Weather:	Cloudy 27°
Location (identify on site pla	n)	See Attached Ske	etch			
Land Use vacant lot		Slope (%)	0-3%		Surfaces Stones	few
(eg woodland, agricultural fie	eld, va	cant lot etc)				
Vegatation mixed hardwood	s and p	oines				
Landform Ground Morraine	e					
Position on landscape	See at	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	age Way	>100	feet	
Possible Wet Area	>100	feet Proper	rty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

	Deep Observation Hole Log						
Hole # 122	-1	NB 14/E-31			Suface El. 301.9		
Depth from	Soil	Soil Texture	Soil Color	Soil	Other		
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,		
(inches)					Consistency, % Gravel)		
0-12	А	FSL	10YR 3/3				
12-42	C1	S	10YR 5/4				
42-85	C2	SL	2.5Y 5/3	@42"			
				10YR 6/4			
				2.5Y 6/2			
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	PROPOSED	DISPOSAL AREA		

	Depth to Bedrock: 85"	
None	Weeping from Pit Face:	None
42"		
	None 42"	Depth to Bedrock: 85" None Weeping from Pit Face: 42"

Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-2	Date:	01/13/22 Time:		8:20AM	Weather:	Cloudy 27°
Location (identify on site pla	.n) S	See Attached Ske	etch			
Land Use vacant lot	9	Slope (%)	0-3%		Surfaces Stones	few
(eg woodland, agricultural fie	eld, va	cant lot etc)				
Vegatation mixed hardwood	s and p	oines				
Landform Ground Morraine	e					
Position on landscape	See att	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	ige Way	>100	feet	
Possible Wet Area	>100 ±	feet Proper	ty Line	92±	feet	
Drinking Water Well	>100 ±	feet Other:				
					feet	

	Deep Observation Hole Log							
Hole # 122-	-2	NB 14/E-31			Suface El. 304.0			
Depth from	Soil	Soil Texture	Soil Color	Soil	Other			
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,			
(inches)					Consistency, % Gravel)			
0-12	А	FSL	10YR 3/3					
12-50	C1	S	10YR 5/4					
50-91	C2	SL	2.5Y 5/4	@50"				
				10YR 6/4				
				2.5Y 6/2				
*MI	NIMUM OF	2 HOLES REQU	VIRED AT EVERY F	ROPOSED	DISPOSAL AREA			
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >91"			

 Parent Material (geologic)
 Glacial Till
 Depth to Bedrock: >91"

 Depth to Groundwater: Standing Water in the Hole
 88"
 Weeping from Pit Face: 78"

 Estimated Seasonal High Groundwater in the Hole
 50"
 Aditional Notes

122-2

Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-3	Date:	01/13/22 Time:		8:40AM	Weather:	Cloudy 27°
Location (identify on site pl	an)	See Attached Ske	etch			
Land Use vacant lot		Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural f	ield, va	cant lot etc)				
Vegatation mixed hardwood	ls and j	pines				
Landform Ground Morrain	e					
Position on landscape	See at	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	ige Way	>100	feet	
Possible Wet Area	>100	feet Proper	ty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

	Deep Observation Hole Log						
Hole # 122	-3	NB 14/E-31			Suface El. 295.4		
Depth from	Soil	Soil Texture	Soil Color	Soil	Other		
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,		
(inches)					Consistency, % Gravel)		
0-12	А	FSL	10YR 3/3				
12-27	C1	S	10YR 6/4	@24"			
27-102	C2	FSL	2.5Y 5/4	7.5YR 5/6			
				2.5Y 6/2			
*MI	*MINIMUM OF 2 HOLES REQUIRED AT EVERY PROPOSED DISPOSAL AREA						

Parent Material (geologic) Glacial Till		Depth to Bedrock: $>102'$	'
Depth to Groundwater: Standing Water in the Hole	60"	Weeping from Pit Face:	30"
Estimated Seasonal High Groundwater in the Hole	24"		
Aditional Notes			
Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-4 I	Date: 01/1	3/22 Time:		9:00AM	Weather:	Cloudy 27°
Location (identify on site plan	n) See A	Attached Ske	tch			
Land Use vacant lot	Slop	e (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural fie	ld, vacant	lot etc)				
Vegatation mixed hardwoods	and pines					
Landform Ground Morraine						
Position on landscape S	See attache	ed Sketch				
Distances from:						
Open Water Body	>100 feet	Draina	ge Way	>100	feet	
Possible Wet Area	>100 feet	Propert	y Line	92±	feet	
Drinking Water Well	>100 feet	Other:				
					feet	

	Deep Observation Hole Log								
Hole # 122	-4	NB 14/E-31			Suface El. 289.4				
Depth from	Soil	Soil Texture	Soil Color	Soil	Other				
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,				
(inches)					Consistency, % Gravel)				
0-12	А	FSL	10YR 3/3						
12-26	Fill	LS	10YR 6/4						
26-40	Ab	FSL	10YR 4/3						
40-50	C1	FS	2.5Y 6/1	@45"					
50-102	C2	FSL	2.5Y 5/4	7.5YR 5/6					
				2.5Y 6/2					

 *MINIMUM OF 2 HOLES REQUIRED AT EVERY PROPOSED DISPOSAL AREA

 Parent Material (geologic)
 Glacial Till

 Depth to Bedrock:
 >102"

 Depth to Groundwater: Standing Water in the Hole
 90"

 Estimated Seasonal High Groundwater in the Hole
 45"

 Aditional Notes
 45"

Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-5 Date:	01/13/22 Time:	9:30AM	Weather:	Cloudy 27°
Location (identify on site plan)	See Attached Ske	etch		
Land Use vacant lot	Slope (%)	3-8%	Surfaces Stones	few
(eg woodland, agricultural field, v	acant lot etc)			
Vegatation mixed hardwoods and	pines			
Landform Ground Morraine				
Position on landscape See a	ttached Sketch			
Distances from:				
Open Water Body >100	feet Draina	age Way >10	0 feet	
Possible Wet Area >100	feet Proper	rty Line 92	feet	
Drinking Water Well >100	feet Other:			
			feet	

	Deep Observation Hole Log							
Hole # 122	-5	NB 14/E-31			Suface El. 288.3			
Depth from	Soil	Soil Texture	Soil Color	Soil	Other			
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,			
(inches)					Consistency, % Gravel)			
0-40 Fill		FSL	10YR 3/3					
40-48	C1	S	10YR 5/4					
48-108	C2	FSL	2.5Y 5/4	@50"				
				7.5YR 5/6				
				2.5Y 6/2				
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	ROPOSED	DISPOSAL AREA			
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >108"			

122-5

Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-6 I	Date: 01/13/22 Tir	ne:	10:00AM	Weather:	Cloudy 27°
Location (identify on site plan) See Attached	Sketch			
Land Use vacant lot	Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural fiel	d, vacant lot etc))			
Vegatation mixed hardwoods	and pines				
Landform Ground Morraine					
Position on landscape S	See attached Sketch				
Distances from:					
Open Water Body >	>100 feet Dra	ainage Way	>100	feet	
Possible Wet Area >	>100 feet Pro	perty Line	92±	feet	
Drinking Water Well >	>100 feet Oth	ner:			
				feet	

	Deep Observation Hole Log							
Hole # 122	-6	NB 14/E-31		Suface El. 281.2				
Depth from	Soil	Soil Texture	Soil Color	Soil	Other			
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,			
(inches)					Consistency, % Gravel)			
0-15	А	FSL	@15"					
15-80	C1	FSL	GLEY 3/N	7.5YR 5/8				
80-108	C2	FLS	2.5Y 4/3					
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	PROPOSED	DISPOSAL AREA			
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >108"			
Depth to Groun	dwater: Stan	ding Water in the	Hole 100"	Wee	eping from Pit Face: 35"			
Estimated Seaso	Estimated Seasonal High Groundwater in the Hole 15"							
Aditional Notes								

Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-7 Da	te: 01/13/22 Time:	10:30AM	Weather:	Cloudy 27°
Location (identify on site plan)	See Attached Sk	etch		
Land Use vacant lot	Slope (%)	3-8%	Surfaces Stones	few
(eg woodland, agricultural field,	vacant lot etc)			
Vegatation mixed hardwoods an	id pines			
Landform Ground Morraine				
Position on landscape See	e attached Sketch			
Distances from:				
Open Water Body >1	00 feet Draina	age Way >100	feet	
Possible Wet Area >1	00 feet Prope	rty Line 92±	feet	
Drinking Water Well >1	00 feet Other	:		
			feet	

	Deep Observation Hole Log							
Hole # 122	-7	NB 14/E-31	Suface El. 283.8					
Depth from	Soil Soil Texture Soil Color		Soil Color	Soil	Other			
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,			
(inches)					Consistency, % Gravel)			
0-15	А	FSL	7.5YR 2.5/2	@15"				
15-28	C1	S	10YR 4/2	10YR 5/6				
28-99	C2	FSL	GLEY 3/N					
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	ROPOSED	DISPOSAL AREA			
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >99"			
Depth to Groun	dwater: Stan	ding Water in the	Hole 80"	Wee	eping from Pit Face: 20"			
Estimated Seaso	onal High Gr	oundwater in the	Hole 15"					
Aditional Notes								

Location Address or Lot #: 203 Ayer Road Harvard, MA

On-Site Review

Deep Hole #: 122-8	Date:	01/13/22 Time:		11:00AM	Weather:	Cloudy 27°
Location (identify on site pla	ın)	See Attached Ske	etch		""	
Land Use vacant lot		Slope (%)	3-8%		Surfaces Stones	few
(eg woodland, agricultural fi	eld, va	cant lot etc)				
Vegatation mixed hardwood	ls and p	oines				
Landform Ground Morrain	e					
Position on landscape	See at	tached Sketch				
Distances from:						
Open Water Body	>100	feet Draina	ige Way	>100	feet	
Possible Wet Area	>100	feet Proper	ty Line	92±	feet	
Drinking Water Well	>100	feet Other:				
					feet	

	Deep Observation Hole Log								
Hole # 122	-8	NB 14/E-31		Suface El. 292.5					
Depth from	Soil	Soil Texture	Soil Color	Soil	Other				
Surface	Horizon	(USDA)	(MUNSELL)	Mottling	(Stucture, Stones, Boulders,				
(inches)					Consistency, % Gravel)				
0-12	А	FSL	10YR 3/3						
12-28	C1	LS	10YR 5/4						
28-76	C2	SL	2.5Y 5/3	@60"					
				10YR 6/4					
				2.5Y 6/2					
*MI	NIMUM OF	2 HOLES REQU	JIRED AT EVERY F	ROPOSED	DISPOSAL AREA				
Parent Material	(geologic)	Glacial Till		Depth	to Bedrock: >76"				
Depth to Groun	dwater: Stan	ding Water in the	Hole 72"	Wee	eping from Pit Face: 70"				
Estimated Seaso	onal High Gr	oundwater in the	Hole 60"						

Aditional Notes

Location Address or Lot#: 203 Ayer Road Harvard, MA

Determination for Seasonal High Water Table

Method Used:

	Depth observed standing	in observation he	ole	inches	801098101066010300101660100001066001000010000
	Depth weeping from side				
	Ground water adjustmen	t inches	See individua	al Keports	
Index Wel	Number	Reading Date		Index Well Level	
Adjustmer	t Factor	Adjusted Groun	d Water Leve		
Depth of N	aturally Occuring Pervio	us Material			
	Does at least four feet of	naturally occurir	g pervious m	aterial exist in all area	S
	observed throughout the	area proposed for	the soil abso	rption system?	Yes
	If not, what is the depth	of naturally occur	ing pervious 1	naterial?	Feet
Certificatio	<u>on</u>				
	I certify that I am current pursuant to 310 CMR 15 has been performed by n in 310 CMR 15 017. I fu	tly approved by th .017 to conduct s ne consistent with rther certify that t	ne Department oil evaluation the training,	t of Environmental Pro s and that the above a expertise and experier my soil evaluation as	otection nalysis nee described indicated
	on the attached soil evalu	ation form, are a	ccurate and in	accordance with 310	CMR
	15.100 through 15.107.	0 -	100		
	Signature	Rom Z	-	Date 2	19/22
Notes:					





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 Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	NRCC 24-hr	D	Default	24.00	1	3.13	2
2	10-Year	NRCC 24-hr	D	Default	24.00	1	4.68	2
3	50-Year	NRCC 24-hr	D	Default	24.00	1	7.00	2
4	100-Year	NRCC 24-hr	D	Default	24.00	1	8.34	2

Rainfall Events Listing

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Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
181,979	61	>75% Grass cover, Good, HSG B (SC1.0)
52,672	96	Gravel surface, HSG B (SC1.0)
35,529	55	Woods, Good, HSG B (SC1.0)
270,181	67	TOTAL AREA

203	Ayer Road, Harvard, MA
PRE Rev-1 NRCC 24-hr D	2-Year Rainfall=3.13"
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Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment SC1.0:

Runoff Area=270,181 sf 0.00% Impervious Runoff Depth=0.65" Flow Length=757' Tc=13.4 min CN=67 Runoff=2.8 cfs 14,651 cf

Link AP-1:

Inflow=2.8 cfs 14,651 cf Primary=2.8 cfs 14,651 cf

Total Runoff Area = 270,181 sf Runoff Volume = 14,651 cf Average Runoff Depth = 0.65" 100.00% Pervious = 270,181 sf 0.00% Impervious = 0 sf

Summary for Subcatchment SC1.0:

Runoff =	2.8 cfs @	12.24 hrs,	Volume=	14,651 cf,	Depth=	0.65"
----------	-----------	------------	---------	------------	--------	-------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN D	escription						
1	81,979	61 >	61 >75% Grass cover, Good, HSG B						
	35,529	55 V	Voods, Go	od, HSG B					
	52,672	96 G	Gravel surfa	ace, HSG E	}				
2	70,181	67 V	Veighted A	verage					
2	70,181	1	00.00% Pe	ervious Are	а				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
4.3	35	0.1429	0.14		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.13"				
1.0	15	0.1333	0.25		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.13"				
6.5	556	0.0420	1.43		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 tps				
0.2	27	0.1481	1.92		Shallow Concentrated Flow,				
	404	0.0450	4 40		Woodland Kv= 5.0 fps				
1.4	124	0.0450	1.48		Shallow Concentrated Flow,				
. <u> </u>					Short Grass Pasture Kv= 7.0 fps				
13.4	757	Total							

Summary for Link AP-1:

Inflow Ar	rea =	270,181 sf,	0.00% Impervious,	Inflow Depth = 0.65"	for 2-Year event
Inflow	=	2.8 cfs @	12.24 hrs, Volume=	14,651 cf	
Primary	=	2.8 cfs @	12.24 hrs, Volume=	14,651 cf, Att	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs

	203 Ayer Road, Harvard, MA
PRE Rev-1 NRC	C 24-hr D 10-Year Rainfall=4.68"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 7/13/2022
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Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.0:

Runoff Area=270,181 sf 0.00% Impervious Runoff Depth=1.58" Flow Length=757' Tc=13.4 min CN=67 Runoff=7.9 cfs 35,658 cf

Link AP-1:

Inflow=7.9 cfs 35,658 cf Primary=7.9 cfs 35,658 cf

Total Runoff Area = 270,181 sf Runoff Volume = 35,658 cf Average Runoff Depth = 1.58" 100.00% Pervious = 270,181 sf 0.00% Impervious = 0 sf

Summary for Subcatchment SC1.0:

$\pi u = 1.9 \text{ GS} (0, 12.22 \text{ HS}, V) u = 35,050 \text{ G}, Depti = 1.50$	Runoff =	7.9 cfs @	12.22 hrs, Volume=	35,658 cf, Depth= 1.58
---	----------	-----------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

Α	rea (sf)	CN D	escription						
1	81,979	61 >	61 >75% Grass cover, Good, HSG B						
	35,529	55 V	Voods, Go	od, HSG B					
	52,672	96 G	96 Gravel surface, HSG B						
2	270,181	81 67 Weighted Average							
2	70,181	1	00.00% Pe	ervious Are	а				
_									
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
4.3	35	0.1429	0.14		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.13"				
1.0	15	0.1333	0.25		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.13"				
6.5	556	0.0420	1.43		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.2	27	0.1481	1.92		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
1.4	124	0.0450	1.48		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
13.4	757	Total							

Summary for Link AP-1:

Inflow /	Area	a =	270,181 sf,	0.00% Impe	ervious,	Inflow Depth =	1.58"	for 10-	Year event
Inflow		=	7.9 cfs @	12.22 hrs, Vo	olume=	35,658 0	of		
Primar	у	=	7.9 cfs @	12.22 hrs, Vo	olume=	35,658 (cf, Atte	n= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs

	203 Ayer Road, Harvard, MA
PRE Rev-1	NRCC 24-hr D 50-Year Rainfall=7.00"
Prepared by Goldsmith, Prest & Ringwall, Inc.	Printed 7/13/2022
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Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment SC1.0:Runoff Area=270,181 sf0.00% ImperviousRunoff Depth=3.31"Flow Length=757'Tc=13.4 minCN=67Runoff=17.2 cfs74,457 cf

Link AP-1:

Inflow=17.2 cfs 74,457 cf Primary=17.2 cfs 74,457 cf

Total Runoff Area = 270,181 sf Runoff Volume = 74,457 cf Average Runoff Depth = 3.31" 100.00% Pervious = 270,181 sf 0.00% Impervious = 0 sf

Summary for Subcatchment SC1.0:

Runoff =	17.2 cfs @	12.22 hrs, Vol	ume= 74,457 cf,	Depth= 3.31"
----------	------------	----------------	-----------------	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN D	escription						
1	81,979	61 >	61 >75% Grass cover, Good, HSG B						
	35,529	55 V	55 Woods, Good, HSG B						
	52,672	96 G	96 Gravel surface, HSG B						
2	270,181	67 V	Veighted A	verage					
2	270,181	1	00.00% Pe	ervious Are	а				
_				- ··					
TC	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
4.3	35	0.1429	0.14		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.13"				
1.0	15	0.1333	0.25		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.13"				
6.5	556	0.0420	1.43		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.2	27	0.1481	1.92		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
1.4	124	0.0450	1.48		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
13.4	757	Total							

Summary for Link AP-1:

Inflow Area	a =	270,181 sf,	0.00% Impervious,	Inflow Depth = 3.31"	for 50-Year event
Inflow	=	17.2 cfs @	12.22 hrs, Volume=	74,457 cf	
Primary	=	17.2 cfs @	12.22 hrs, Volume=	74,457 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs

	20	3 Ayer Road, Ha	arvard, MA
PRE Rev-1	NRCC 24-hr D	100-Year Rain	ofall=8.34"
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Time span=0.00-26.00 hrs, dt=0.05 hrs, 521 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment SC1.0:Runoff Area=270,181 sf0.00% ImperviousRunoff Depth=4.41"Flow Length=757'Tc=13.4 minCN=67Runoff=22.9 cfs99,179 cf

Link AP-1:

Inflow=22.9 cfs 99,179 cf Primary=22.9 cfs 99,179 cf

Total Runoff Area = 270,181 sf Runoff Volume = 99,179 cf Average Runoff Depth = 4.41" 100.00% Pervious = 270,181 sf 0.00% Impervious = 0 sf

Summary for Subcatchment SC1.0:

Runoff = 22.9 cfs @ 12.21 hrs, Volume= 99,179 cf, Depth= 4.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

_	A	rea (sf)	CN D	Description					
	1	81,979	61 >	61 >75% Grass cover, Good, HSG B					
		35,529	55 V	55 Woods, Good, HSG B					
_		52,672	96 0	Gravel surfa	ace, HSG E	}			
	2	70,181	67 V	Veighted A	verage				
	2	70,181	1	00.00% Pe	ervious Are	а			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	4.3	35	0.1429	0.14		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 3.13"			
	1.0	15	0.1333	0.25		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.13"			
	6.5	556	0.0420	1.43		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.2	27	0.1481	1.92		Shallow Concentrated Flow,			
		404	0.0450	4 40		Woodland Kv= 5.0 fps			
	1.4	124	0.0450	1.48		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 tps			
	13.4	757	Total						

Summary for Link AP-1:

Inflow Area	a =	270,181 sf,	0.00% Impervious,	Inflow Depth = 4.41 "	for 100-Year event
Inflow	=	22.9 cfs @	12.21 hrs, Volume=	99,179 cf	
Primary	=	22.9 cfs @	12.21 hrs, Volume=	99,179 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.05 hrs





203 Ayer Road, Harvard, MA

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							-		
	Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
		Name				(hours)		(inches)	
_	1	2-Year	NRCC 24-hr	D	Default	24.00	1	3.13	2
	2	10-Year	NRCC 24-hr	D	Default	24.00	1	4.68	2
	3	50-Year	NRCC 24-hr	D	Default	24.00	1	7.00	2
	4	100-Year	NRCC 24-hr	D	Default	24.00	1	8.34	2

Rainfall Events Listing

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Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
119,876	61	>75% Grass cover, Good, HSG B (SC1.1, SC1.2, SC1.3, SC2.1, SC2.2, SC2.3, SC3.1, SC3.2, SC3.3, SC4.2, SC4.3, SC4.4, SC4.5, SC5.1)
9,151	96	Gravel surface, HSG B (SC1.1, SC1.2, SC1.3, SC2.2, SC3.3, SC5.1)
71,721	98	Paved parking, HSG B (SC1.1, SC1.2, SC1.3, SC2.1, SC2.2, SC2.3, SC3.2,
		SC4.2, SC4.3, SC4.4, SC5.1)
42,613	98	Roofs, HSG B (SC1.2, SC2.1, SC2.2, SC3.1, SC4.1)
7,036	98	Unconnected pavement, HSG B (SC1.1, SC1.2, SC1.3, SC2.1, SC2.2, SC2.3,
		SC3.2, SC4.2, SC4.3, SC4.4, SC5.1)
19,783	55	Woods, Good, HSG B (SC1.1, SC1.2, SC1.3, SC2.2, SC5.1)
270,181	78	TOTAL AREA

	203	Ayer Road, Ha	arvard, MA
POST Rev-1	NRCC 24-hr D	2-Year Rain	fall=3.13"
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>0.83" Flow Length=233' Tc=6.0 min CN=71 Runoff=0.3 cfs 1,058 cf
Subcatchment SC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>1.69" Flow Length=335' Tc=9.0 min CN=85 Runoff=1.0 cfs 3,732 cf
Subcatchment SC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>1.10" Flow Length=520' Tc=13.0 min CN=76 Runoff=0.7 cfs 3,064 cf
Subcatchment SC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>2.68" Tc=6.0 min CN=96 Runoff=0.6 cfs 2,071 cf
Subcatchment SC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>1.41" Flow Length=295' Tc=7.8 min CN=81 Runoff=0.6 cfs 2,182 cf
Subcatchment SC2.3:	Runoff Area=11,540 sf 37.45% Impervious Runoff Depth>1.04" Flow Length=185' Tc=6.0 min CN=75 Runoff=0.3 cfs 1,005 cf
SubcatchmentSC3.1:	Runoff Area=11,615 sf 100.00% Impervious Runoff Depth>2.89" Tc=6.0 min CN=98 Runoff=0.7 cfs 2,802 cf
SubcatchmentSC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>2.57" Flow Length=118' Slope=0.0380 '/' Tc=6.0 min CN=95 Runoff=1.5 cfs 5,506 cf
Subcatchment SC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>0.53" Flow Length=216' Tc=6.0 min CN=64 Runoff=0.2 cfs 642 cf
SubcatchmentSC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>2.89" Tc=6.0 min CN=98 Runoff=0.9 cfs 3,618 cf
SubcatchmentSC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>1.35" Flow Length=231' Tc=6.0 min CN=80 Runoff=0.3 cfs 1,176 cf
SubcatchmentSC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>2.57" Tc=6.0 min CN=95 Runoff=0.3 cfs 1,004 cf
SubcatchmentSC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>2.47" Tc=6.0 min CN=94 Runoff=0.3 cfs 1,058 cf
SubcatchmentSC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>0.41" Tc=6.0 min CN=61 Runoff=0.0 cfs 221 cf
SubcatchmentSC5.1:	Runoff Area=62,001 sf 2.46% Impervious Runoff Depth>0.49" Flow Length=362' Tc=10.0 min CN=63 Runoff=0.5 cfs 2,510 cf
Pond CB-1:	Peak Elev=297.80' Inflow=0.3 cfs 1,058 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=0.3 cfs 1,058 cf

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Pond CB-10:	Peak Elev=286.23' Inflow=0.3 cfs 1,176 cf 12.0" Round Culvert n=0.013 L=5.0' S=0.0400 '/' Outflow=0.3 cfs 1,176 cf
Pond CB-2:	Peak Elev=294.37' Inflow=1.0 cfs 3,732 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0200 '/' Outflow=1.0 cfs 3,732 cf
Pond CB-3:	Peak Elev=288.01' Inflow=0.7 cfs 3,064 cf 12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/' Outflow=0.7 cfs 3,064 cf
Pond CB-5:	Peak Elev=294.45' Inflow=0.6 cfs 2,182 cf 12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/' Outflow=0.6 cfs 2,182 cf
Pond CB-6:	Peak Elev=295.93' Inflow=0.6 cfs 2,071 cf 12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/' Outflow=0.6 cfs 2,071 cf
Pond CB-7:	Peak Elev=288.55' Inflow=0.3 cfs 1,004 cf 12.0" Round Culvert n=0.013 L=22.0' S=0.0136 '/' Outflow=0.3 cfs 1,004 cf
Pond CB-8:	Peak Elev=288.80' Inflow=0.3 cfs 1,058 cf 12.0" Round Culvert n=0.013 L=15.0' S=0.0200 '/' Outflow=0.3 cfs 1,058 cf
Pond CB-9:	Peak Elev=286.80' Inflow=0.3 cfs 1,005 cf 12.0" Round Culvert n=0.013 L=40.0' S=0.0200 '/' Outflow=0.3 cfs 1,005 cf
Pond DCB-4:	Peak Elev=285.27' Inflow=1.5 cfs 5,506 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/' Outflow=1.5 cfs 5,506 cf
Pond DMH-1:	Peak Elev=294.15' Inflow=1.3 cfs 4,790 cf 12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/' Outflow=1.3 cfs 4,790 cf
Pond DMH-2:	Peak Elev=287.62' Inflow=1.9 cfs 7,855 cf 18.0" Round Culvert n=0.013 L=48.0' S=0.0100 '/' Outflow=1.9 cfs 7,855 cf
Pond DMH-3:	Peak Elev=284.74' Inflow=1.5 cfs 5,506 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=1.5 cfs 5,506 cf
Pond DMH-5:	Peak Elev=289.65' Inflow=1.2 cfs 4,253 cf 12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/' Outflow=1.2 cfs 4,253 cf
Pond DMH-6:	Peak Elev=288.55' Inflow=1.4 cfs 5,257 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=1.4 cfs 5,257 cf
Pond DMH-7:	Peak Elev=288.66' Inflow=0.3 cfs 1,058 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=0.3 cfs 1,051 cf
Pond DMH-9:	Peak Elev=285.87' Inflow=0.6 cfs 2,181 cf 15.0" Round Culvert n=0.013 L=197.0' S=0.0175 '/' Outflow=0.6 cfs 2,181 cf
Pond IB-1: Discarded=0.0 cfs 647 d	Peak Elev=285.46' Storage=790 cf Inflow=2.1 cfs 8,497 cf f Primary=1.5 cfs 7,644 cf Secondary=0.0 cfs 0 cf Outflow=1.6 cfs 8,291 cf

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Pond IB-2:	Peak Elev=282.74' Storage Primary=0.2 cfs 1,827 cf Secondary=0.0 cf	e=765 cf Inflow=0.7 cfs 2,401 cf is 0 cf Outflow=0.2 cfs 1,827 cf
Pond IC-1:	Peak Elev=284.74' Storage= Discarded=0.1 cfs 6,382 cf Primary=0.0 cf	3,540 cf Inflow=2.2 cfs 8,308 cf is 0 cf Outflow=0.1 cfs 6,382 cf
Pond IC-2:	Peak Elev=288.55' Storage= Discarded=0.1 cfs 6,350 cf Primary=0.0 cfs	4,576 cf Inflow=2.7 cfs 9,926 cf 129 cf Outflow=0.1 cfs 6,478 cf
Link AP-1:		Inflow=2.1 cfs 12,110 cf Primary=2.1 cfs 12,110 cf

Total Runoff Area = 270,181 sf Runoff Volume = 31,650 cf Average Runoff Depth = 1.41" 55.08% Pervious = 148,810 sf 44.92% Impervious = 121,371 sf

Summary for Subcatchment SC1.1:

Runoff	=	0.3 cfs @	12.14 hrs,	Volume=	1.058 cf, Depth>	0.83"
			,		.,	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN I	Description						
	4,381	61 :	61 >75% Grass cover, Good, HSG B						
	5,583	55	Noods, Go	od, HSG B					
	328	98	Jnconnecte	ed pavemer	nt, HSG B				
	3,654	98	Paved parking, HSG B						
	1,269	96	6 Gravel surface, HSG B						
	15,215	71	71 Weighted Average						
	11,233	-	73.83% Pei	vious Area					
	3,982		26.17% Imp	pervious Ar	ea				
	328	ä	3.23% Unco	onnected					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.4	34	0.2400	0.17		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
0.9	16	0.1875	0.28		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.12"				
1.0	83	0.0361	1.33		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.5	100	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
5.8	233	Total,	Increased t	o minimum	1 Tc = 6.0 min				

Summary for Subcatchment SC1.2:

Runoff = 1.0 cfs @ 12.16 hrs, Volume= 3,732 cf, Depth> 1.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Area (sf)	CN	Description
6,620	61	>75% Grass cover, Good, HSG B
2,398	55	Woods, Good, HSG B
1,172	98	Unconnected pavement, HSG B
8,000	98	Roofs, HSG B
7,203	98	Paved parking, HSG B
1,052	96	Gravel surface, HSG B
26,444	85	Weighted Average
10,069		38.08% Pervious Area
16,375		61.92% Impervious Area
1,172		7.16% Unconnected
	Area (sf) 6,620 2,398 1,172 8,000 7,203 1,052 26,444 10,069 16,375 1,172	Area (sf) CN 6,620 61 2,398 55 1,172 98 8,000 98 7,203 98 1,052 96 26,444 85 10,069 16,375 1,172 1,172

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Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
3.9	41	0.2400	0.17		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
1.4	9	0.0200	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
3.4	222	0.0248	1.10		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	63	0.0300	3.52		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
9.0	335	Total			

Summary for Subcatchment SC1.3:

Runoff = 0.7 cfs @ 12.22 hrs, Volume= 3,064 cf, Depth> 1.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN [Description		
	15,029	61 >	>75% Gras	s cover, Go	bod, HSG B
	3,894	55 \	Voods, Go	od, HSG B	
	1,741	96 (Gravel surfa	ace, HSG E	3
	275	98 l	Jnconnecte	ed pavemer	nt, HSG B
	12,548	98 F	Paved park	ing, HSG B	
	33,487	76 \	Veighted A	verage	
	20,664	6	61.71% Pei	vious Area	
	12,823	3	38.29% Imp	pervious Ar	ea
	275	2	2.14% Unco	onnected	
_		~			
TC	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.2	50	0.0800	0.12		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
3.2	188	0.0372	0.96		Shallow Concentrated Flow,
0.0	400	0 0070	4.05		Woodland Kv= 5.0 fps
2.0	160	0.0372	1.35		Shallow Concentrated Flow,
0.0	400	0 0000	0.50		Short Grass Pasture KV= 7.0 tps
0.6	122	0.0300	3.52		Shallow Concentrated Flow,
40.0	500	T . 4 . 1			raveu nv-20.0 1ps

13.0 520 Total

Summary for Subcatchment SC2.1:

Runoff = 0.6 cfs @ 12.13 hrs, Volume= 2,071 cf, Depth> 2.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

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203 Ayer Road, Harvard, MA NRCC 24-hr D 2-Year Rainfall=3.13" Printed 7/13/2022 HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC Page 9

A	rea (sf)	CN	Description						
	595	61	>75% Grass	s cover, Go	lood, HSG B				
	423	98	Unconnecte	ed pavemer	ent, HSG B				
	4,000	98	Roofs, HSG	B					
	4,263	98	Paved parki	ing, HSG B	В				
	9,281	96	Weighted A	verage					
	595		6.41% Perv	6.41% Pervious Area					
	8,686		93.59% Imp	ervious Are	rea				
	423		4.87% Unco	onnected					
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	i) (ft/sec)	(cts)					
5.0					Direct Entry,				
5.0	0	Total,	Increased to	o minimum	n Tc = 6.0 min				

Summary for Subcatchment SC2.2:

0.6 cfs @ 12.15 hrs, Volume= 2,182 cf, Depth> 1.41" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Α	rea (sf)	CN E	Description						
	5,529	61 >	61 >75% Grass cover, Good, HSG B						
	2,637	55 V	Voods, Go	od, HSG B					
	889	98 L	Inconnecte	ed pavemer	nt, HSG B				
	4,000	98 F	Roofs, HSC	βB					
	4,580	98 F	Paved park	ing, HSG B					
	929	96 (Gravel surfa	ace, HSG E	}				
	18,563	81 V	Veighted A	verage					
	9,094	4	8.99% Pe	vious Area					
	9,469	5	51.01% Imp	pervious Ar	ea				
	889	ç	.39% Unc	onnected					
-				o "					
	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(CfS)					
4.6	50	0.2400	0.18		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
0.2	8	0.0200	0.71		Shallow Concentrated Flow,				
o -	4 - 4				Woodland Kv= 5.0 fps				
2.5	151	0.0200	0.99		Shallow Concentrated Flow,				
<u> </u>	~~~		o o 7		Short Grass Pasture Kv= 7.0 fps				
0.5	86	0.0200	2.87		Shallow Concentrated Flow,				
					Paved Kv= 20.3 tps				
7.8	295	Total							

Summary for Subcatchment SC2.3:

Runoff = 0.3 cfs @ 12.14 hrs, Volume= 1,005 cf, Depth> 1.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Α	rea (sf)	CN [Description						
	7,218	61 >	61 >75% Grass cover, Good, HSG B						
	293	98 l	Jnconnecte	ed pavemer	nt, HSG B				
	4,029	98 F	Paved park	ing, HSG B	3				
	11,540	75 \	Veighted A	verage					
	7,218	6	62.55% Per	vious Area					
	4,321	3	37.45% Imp	pervious Ar	ea				
	293	6	6.77% Unco	onnected					
_									
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
2.3	42	0.1430	0.31		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.12"				
0.1	8	0.0500	1.18		Sheet Flow,				
					Smooth surfaces n= 0.011 P2= 3.12"				
0.5	135	0.0500	4.54		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
2.9	185	Total,	Increased t	o minimum	ı Tc = 6.0 min				

Summary for Subcatchment SC3.1:

Runoff = 0.7 cfs @ 12.13 hrs, Volume= 2,802 cf, Depth> 2.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Ar	rea (sf)	CN I	Description						
	11,615	98	Roofs, HSG	βB					
	0	61 :	>75% Gras	s cover, Go	bod, HSG B				
	11,615	98	8 Weighted Average						
	0	(0.00% Pervious Area						
	11,615		100.00% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				
5.0	0	Total,	Total, Increased to minimum Tc = 6.0 min						

Summary for Subcatchment SC3.2:

Page 11

Runoff 1.5 cfs @ 12.13 hrs, Volume= 5,506 cf, Depth> 2.57" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Α	rea (sf)	CN [Description						
	2,372	61 >	61 >75% Grass cover, Good, HSG B						
	21,883	98 F	Paved park	ing, HSG B					
	1,422	98 l	Jnconnecte	ed pavemer	nt, HSG B				
	25,677	95 V	Veighted A	verage					
	2,372	ç	9.24% Perv	ious Area					
	23,305	ç	90.76% Imp	pervious Are	ea				
	1,422	6	6.10% Unco	onnected					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
4.4	50	0.0380	0.19		Sheet Flow,				
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
5.2	118	Total, I	ncreased t	o minimum	Tc = 6.0 min				

Summary for Subcatchment SC3.3:

0.2 cfs @ 12.15 hrs, Volume= 642 cf, Depth> 0.53" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Α	rea (sf)	CN [Description		
	13,540	61 >	>75% Gras	s cover, Go	ood, HSG B
	1,127	96 (Gravel surfa	ace, HSG E	3
	14,666	64 \	Veighted A	verage	
	14,666		100.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.7	50	0.0600	0.23		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
1.9	166	0.0422	1.44		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
5.6	216	Total,	ncreased t	o minimum	Tc = 6.0 min

DOOT							203 Ay	er Road, Harvard, MA		
POST F	Rev-1					NRCC	24-hr D 2	-Year Rainfall=3.13"		
Prepare	d by Gol	dsmith,	Prest & Ri	ngwall, Ind				Printed 7/13/2022		
HydroCAI	D® 10.10-4	4a s/n 01	1036 © 2020) HydroCAL	Software Solution	ons LLC		Page 12		
	Summary for Subcatchment SC4.1:									
Runoff	=	0.9 c	fs @ 12.1	3 hrs, Volu	ume=	3,618 cf,	Depth> 2.	89"		
Runoff by NRCC 24	y SCS TR 4-hr D 2-	R-20 met Year Rai	hod, UH=S infall=3.13"	CS, Weigh	ted-CN, Time S	Span= 0.0	0-24.00 hrs	, dt= 0.05 hrs		
A	rea (sf)	CN E	Description							
	14,998	98 F	Roofs, HSG	В						
	14,998	1	00.00% Im	pervious A	rea					
Tc (min)	Length	Slope	Velocity	Capacity	Description					
5.0	(icct)	(1011)	(10/300)	(013)	Direct Entry					
5.0	0	Total.	ncreased to	o minimum	Tc = 6.0 min					
	-	, .								
	Summary for Subcatchment SC4.2:									
Runoff	=	0.3 c	fs @ 12.1	3 hrs, Volu	ume=	1,176 cf,	Depth> 1.	35"		
Runoff by NRCC 24	y SCS TR 4-hr D 2-	R-20 met Year Rai	hod, UH=S infall=3.13"	CS, Weigh	ted-CN, Time S	Span= 0.0	0-24.00 hrs	, dt= 0.05 hrs		
A	rea (sf)	CN E	Description							
	5,060	61 >	75% Grass	s cover, Go	od, HSG B					
	1,450	98 L	Inconnecte	d pavemer	nt, HSG B					
	3,975	<u>98 F</u>	aved parki	ng, HSG B						
	10,484	80 V	Veighted A	verage						
	5,000 5,424	4	8.20% Per	vious Area	22					
	1 450	2	26 72% Und	connected	5a					
	1,-100	-	0.7270 0110							
Тс	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	<u>(ft/ft)</u>	(ft/sec)	(cfs)						
4.0	50	0.0500	0.21		Sheet Flow,	n = 0.150	D2- 3 12			
04	33	0 0500	1 57		Shallow Con	centrated	FZ= 3.12			
0.1		2.2000			Short Grass P	asture k	(v= 7.0 fps			
0.6	148	0.0440	4.26		Shallow Con	centrated	I Flow,			
		<u> </u>			Paved Kv= 2	20.3 fps				
5.0	231	iotal, I	ncreased to	o minimum	1 c = 6.0 min					

Summary for Subcatchment SC4.3:

Runoff = 0.3 cfs @ 12.13 hrs, Volume= 1,004 cf, Depth> 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

203 Ayer Road, Harvard, MA NRCC 24-hr D 2-Year Rainfall=3.13" Printed 7/13/2022 ns LLC Page 13

POST Rev-1

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A	rea (sf)	CN	Description		
	406	61	>75% Grass	s cover, Go	ood, HSG B
	175	98	Unconnecte	ed pavemen	ent, HSG B
	4,101	98	Paved park	ing, HSG B	В
Ta	4,682 406 4,276 175	95 Slop	Weighted A 8.68% Perv 91.32% Imp 4.09% Unco	verage ious Area pervious Are onnected	
(min)	(feet)	Siop (ft/fl	t) (ft/sec)	Capacity (cfs)	Description
5.0	· · · ·	•	<u> </u>	Y	Direct Entry,
5.0	0	Total,	Increased t	o minimum	n Tc = 6.0 min

Summary for Subcatchment SC4.4:

Runoff = 0.3 cfs @ 12.13 hrs, Volume= 1,058 cf, Depth> 2.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

A	rea (sf)	CN	Description						
	564	61	>75% Grass	s cover, Go	ood, HSG B				
	541	98	Unconnecte	ed pavemer	ent, HSG B				
	4,030	98	Paved park	ing, HSG B	В				
	5,136	94	Weighted A	verage					
	564		10.99% Per	vious Area	a				
	4,571		89.01% Imp	89.01% Impervious Area					
	541		11.83% Und	connected					
Tc (min)	Length (feet)	Slop (ft/ft	e Velocity t) (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				
5.0	0	Total,	Increased t	o minimum	n Tc = 6.0 min				

Summary for Subcatchment SC4.5:

Runoff = 0.0 cfs @ 12.15 hrs, Volume= 221 cf, Depth> 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-Year Rainfall=3.13"

Area (sf)	CN	Description							
6,391	61	>75% Grass cover, Good, HSG B							
6,391		100.00% Pervious Area							
POST	Rev-1					NRCC	203 Ayer 24-hr D 2-Y	Road, Har e <i>ar Rainfa</i>	vard, MA all=3.13"
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Prepare	ed by Gol	dsmith, I	Prest & Ri	ingwall, Ind	C.			Printed 7	/13/2022
HydroCA	D® 10.10-	4a_s/n_01	036 © 202	0 HydroCAD) Software Solut	tions LLC			Page 14
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0	(1001)	(14,14)	(1	(0.0)	Direct Entry	,			
5.0	0	Total, Ir	ncreased t	o minimum	Tc = 6.0 min				
			Sum	mary for	Subcatchm	ent SC5.1	:		
Runoff	=	0.5 cf	s@ 12.2	20 hrs, Volu	ume=	2,510 cf,	Depth> 0.49	"	
Runoff b NRCC 2	oy SCS TF 24-hr D 2-	R-20 meth Year Rai	iod, UH=S nfall=3.13"	CS, Weigh	ted-CN, Time	Span= 0.00	-24.00 hrs, d	t= 0.05 hrs	
Α	vrea (sf)	CN D	escription						
	52,172 5,271 70 1,456 3 033	61 > 55 W 98 U 98 P 96 G	75% Grass /oods, Goo nconnecte aved park ravel surf	s cover, Go od, HSG B ed pavemer ing, HSG B ace, HSG B	ood, HSG B nt, HSG B				
	62,001 60,476 1,525 70	63 W 9 2 4	/eighted A 7.54% Per .46% Impe .56% Unco	verage vious Area ervious Area onnected	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
3.8	36	0.2000	0.16		Sheet Flow, Woods: Light	underbrus	n n= 0.400	P2= 3.12"	
2.1	14	0.0200	0.11		Sheet Flow, Grass: Short	n= 0.150	P2= 3.12"		
4.1	312	0.0334	1.28		Shallow Cor Short Grass	n centrated Pasture Kv	Flow, /= 7.0 fps		
10.0	362	Total							
				Summar	y for Pond (CB-1:			
Inflow A Inflow Outflow Primary	rea = = = =	15,2 [,] 0.3 cf 0.3 cf 0.3 cf	I5 sf, 26.1 s @ 12.1 s @ 12.1 s @ 12.1	17% Imperv 4 hrs, Volu 4 hrs, Volu 4 hrs, Volu	vious, Inflow D ume= ume= ume=	0epth > 0.8 1,058 cf 1,058 cf, 1,058 cf, 1,058 cf	33" for 2-Ye Atten= 0%, L	ear event ₋ag= 0.0 m	in
Routing Peak Ele Flood El	by Dyn-S ev= 297.8 ev= 301.5	tor-Ind m 0' @ 12.1 50'	ethod, Tim 4 hrs	ie Span= 0.	.00-24.00 hrs,	dt= 0.05 hrs	s/2		
Device	Routing		Invert O	utlet Device	es				
#1	Primary	2	97.50' 1 : L: In n:	2.0" Roun = 170.0' C ilet / Outlet = 0.013 Co	d Culvert CPP, projecting Invert= 297.50 prrugated PE, s	, no headwa)' / 293.56' smooth inte	all, Ke= 0.90 S= 0.0232 '/ rior, Flow Ar	0 ' Cc= 0.90 ea= 0.79 s	00 f

Primary OutFlow Max=0.3 cfs @ 12.14 hrs HW=297.80' TW=294.13' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.3 cfs @ 1.46 fps)

Summary for Pond CB-10:

Inflow Are	ea =	10,484 sf,	51.74% Impervious,	Inflow Depth > 1.3	35" for 2-Year event
Inflow	=	0.3 cfs @	12.13 hrs, Volume=	1,176 cf	
Outflow	=	0.3 cfs @	12.13 hrs, Volume=	1,176 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	0.3 cfs @	12.13 hrs, Volume=	1,176 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.23' @ 12.13 hrs Flood Elev= 289.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.90'	12.0" Round Culvert
			L= 5.0 CPP, projecting, no neadwall, $Ke= 0.900$ Inlet / Outlet Invert= 285.90' / 285.70' S= 0.0400 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.3 cfs @ 12.13 hrs HW=286.22' TW=285.86' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.3 cfs @ 1.53 fps)

Summary for Pond CB-2:

Inflow Area	a =	26,444 sf,	61.92% Impervious,	Inflow Depth > 1.	.69" for 2-Year event
Inflow	=	1.0 cfs @	12.16 hrs, Volume=	3,732 cf	
Outflow	=	1.0 cfs @	12.16 hrs, Volume=	3,732 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.0 cfs @	12.16 hrs, Volume=	3,732 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.37' @ 12.16 hrs Flood Elev= 297.70'

Device	Routing	Invert	Outlet Devices					
#1	Primary	293.70'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf					
D!	Drive and OutFlass May 10 of a 12 10 hrs. UN/-201 201 TM/-201 141. (Dynamia Tailyyatar)							

Primary OutFlow Max=1.0 cfs @ 12.16 hrs HW=294.36' TW=294.14' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.0 cfs @ 1.79 fps) 203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D2-Year Rainfall=3.13"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 16

Summary for Pond CB-3:

Inflow Are	a =	33,487 sf,	38.29% Impervious,	Inflow Depth > 1.10" for 2-Year event	
Inflow	=	0.7 cfs @	12.22 hrs, Volume=	3,064 cf	
Outflow	=	0.7 cfs @	12.22 hrs, Volume=	3,064 cf, Atten= 0%, Lag= 0.0 min	
Primary	=	0.7 cfs @	12.22 hrs, Volume=	3,064 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.01' @ 12.22 hrs Flood Elev= 291.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.50'	12.0" Round Culvert L= 8.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.7 cfs @ 12.22 hrs HW=288.00' TW=287.58' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.7 cfs @ 2.55 fps)

Summary for Pond CB-5:

Inflow Area	a =	18,563 sf,	51.01% Impervious,	Inflow Depth >	1.41"	for 2-Year event
Inflow	=	0.6 cfs @	12.15 hrs, Volume=	2,182 c	f	
Outflow	=	0.6 cfs @	12.15 hrs, Volume=	2,182 c	f, Atter	n= 0%, Lag= 0.0 min
Primary	=	0.6 cfs @	12.15 hrs, Volume=	2,182 c	f	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.45' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.6 cfs @ 12.15 hrs HW=294.45' TW=289.64' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.6 cfs @ 1.80 fps)

Summary for Pond CB-6:

Inflow Area	a =	9,281 sf,	93.59% Impervious,	Inflow Depth >	2.68"	for 2-Year event
Inflow	=	0.6 cfs @	12.13 hrs, Volume=	2,071 c	f	
Outflow	=	0.6 cfs @	12.13 hrs, Volume=	2,071 c	f, Atten	= 0%, Lag= 0.0 min
Primary	=	0.6 cfs @	12.13 hrs, Volume=	2,071 c	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 295.93' @ 12.13 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	12.0" Round Culvert L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.5 cfs @ 12.13 hrs HW=295.92' TW=289.63' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.5 cfs @ 1.73 fps)

Summary for Pond CB-7:

Inflow Area	a =	4,682 sf,	91.32% Impervious,	Inflow Depth > 2.5	57" for 2-Year event
Inflow	=	0.3 cfs @	12.13 hrs, Volume=	1,004 cf	
Outflow	=	0.3 cfs @	12.13 hrs, Volume=	1,004 cf, /	Atten= 0%, Lag= 0.0 min
Primary	=	0.3 cfs @	12.13 hrs, Volume=	1,004 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.55' @ 15.00 hrs Flood Elev= 292.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.00'	12.0" Round Culvert L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
			5

Primary OutFlow Max=0.3 cfs @ 12.13 hrs HW=288.33' TW=288.17' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.3 cfs @ 1.75 fps)

Summary for Pond CB-8:

Inflow Area	a =	5,136 sf,	89.01% Impervious,	Inflow Depth > 2	2.47" for 2-Year ev	/ent
Inflow	=	0.3 cfs @	12.13 hrs, Volume=	1,058 cf		
Outflow	=	0.3 cfs @	12.13 hrs, Volume=	1,058 cf	, Atten= 0%, Lag= 0	0.0 min
Primary	=	0.3 cfs @	12.13 hrs, Volume=	1,058 cf	-	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.80' @ 12.13 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.50'	12.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.3 cfs @ 12.13 hrs HW=288.80' TW=287.92' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.3 cfs @ 1.46 fps)

Summary for Pond CB-9:

Inflow Area	a =	11,540 sf,	37.45% Impervious,	Inflow Depth > 1.	.04" for 2-Year event
Inflow	=	0.3 cfs @	12.14 hrs, Volume=	1,005 cf	
Outflow	=	0.3 cfs @	12.14 hrs, Volume=	1,005 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	0.3 cfs @	12.14 hrs, Volume=	1,005 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.80' @ 12.14 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.50'	12.0" Round Culvert
			L= 40.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 286.50' / 285.70' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.3 cfs @ 12.14 hrs HW=286.79' TW=285.86' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.3 cfs @ 1.46 fps)

Summary for Pond DCB-4:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth > 2.5	57" for 2-Year event
Inflow	=	1.5 cfs @	12.13 hrs, Volume=	5,506 cf	
Outflow	=	1.5 cfs @	12.13 hrs, Volume=	5,506 cf, <i>i</i>	Atten= 0%, Lag= 0.0 min
Primary	=	1.5 cfs @	12.13 hrs, Volume=	5,506 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.27' @ 12.13 hrs Flood Elev= 288.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.50'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
D		Max-1 E afa	42.42 hrs. $ A = 205.24$ T $ A = 204.60$ (Dynamia Tailyyatar)

Primary OutFlow Max=1.5 cfs @ 12.13 hrs HW=285.24' TW=284.69' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.5 cfs @ 2.32 fps)

Summary for Pond DMH-1:

Inflow Area	a =	41,659 sf,	48.86% Impervious,	Inflow Depth > 1.38	' for 2-Year event
Inflow	=	1.3 cfs @	12.15 hrs, Volume=	4,790 cf	
Outflow	=	1.3 cfs @	12.15 hrs, Volume=	4,790 cf, At	ten= 0%, Lag= 0.0 min
Primary	=	1.3 cfs @	12.15 hrs, Volume=	4,790 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.15' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	12.0" Round Culvert L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.3 cfs @ 12.15 hrs HW=294.14' TW=287.61' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.3 cfs @ 2.22 fps)

Summary for Pond DMH-2:

Inflow Are	a =	75,146 sf,	44.15% Impervious,	Inflow Depth > 1	.25" for 2-Year event
Inflow	=	1.9 cfs @	12.17 hrs, Volume=	7,855 cf	
Outflow	=	1.9 cfs @	12.17 hrs, Volume=	7,855 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.9 cfs @	12.17 hrs, Volume=	7,855 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.62' @ 12.17 hrs Flood Elev= 291.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.90'	18.0" Round Culvert L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.9 cfs @ 12.17 hrs HW=287.61' TW=285.34' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.9 cfs @ 2.26 fps)

Summary for Pond DMH-3:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth >	2.57"	for 2-Year event
Inflow	=	1.5 cfs @	12.13 hrs, Volume=	5,506 c	of	
Outflow	=	1.5 cfs @	12.13 hrs, Volume=	5,506 c	of, Atter	n= 0%, Lag= 0.0 min
Primary	=	1.5 cfs @	12.13 hrs, Volume=	5,506 c	of	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 284.74' @ 14.67 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.10'	24.0" Round Culvert L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.5 cfs @ 12.13 hrs HW=284.69' TW=284.26' (Dynamic Tailwater) ☐ 1=Culvert (Barrel Controls 1.5 cfs @ 2.79 fps)

Summary for Pond DMH-5:

Inflow Area	ı =	27,844 sf,	65.20% Impervious,	Inflow Depth > 1	.83" for 2-Year event
Inflow	=	1.2 cfs @	12.14 hrs, Volume=	4,253 cf	
Outflow	=	1.2 cfs @	12.14 hrs, Volume=	4,253 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.2 cfs @	12.14 hrs, Volume=	4,253 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.65' @ 12.14 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	289.00'	12.0" Round Culvert L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
			Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.1 cfs @ 12.14 hrs HW=289.64' TW=288.18' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.1 cfs @ 2.14 fps)

Summary for Pond DMH-6:

Inflow Area	a =	32,526 sf,	68.96% Impervious,	Inflow Depth >	1.94" 1	for 2-Year event
Inflow	=	1.4 cfs @	12.14 hrs, Volume=	5,257 c	f	
Outflow	=	1.4 cfs @	12.14 hrs, Volume=	5,257 c	f, Atten	= 0%, Lag= 0.0 min
Primary	=	1.4 cfs @	12.14 hrs, Volume=	5,257 c	f	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.55' @ 14.95 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert
	-		L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.4 cfs @ 12.14 hrs HW=288.18' TW=287.88' (Dynamic Tailwater) -1=Culvert (Barrel Controls 1.4 cfs @ 2.77 fps)

Summary for Pond DMH-7:

Inflow Are	a =	5,136 sf,	89.01% Impervious,	Inflow Depth > 2	2.47" for 2-Year event
Inflow	=	0.3 cfs @	12.13 hrs, Volume=	1,058 cf	
Outflow	=	0.3 cfs @	12.13 hrs, Volume=	1,051 cf	, Atten= 0%, Lag= 0.0 min
Primary	=	0.3 cfs @	12.13 hrs, Volume=	1,051 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.66' @ 14.67 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.3 cfs @ 12.13 hrs HW=287.92' TW=287.85' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.3 cfs @ 1.39 fps)

Summary for Pond DMH-9:

Inflow Are	a =	22,024 sf,	44.25% Impervious,	Inflow Depth > 1.19	9" for 2-Year event
Inflow	=	0.6 cfs @	12.13 hrs, Volume=	2,181 cf	
Outflow	=	0.6 cfs @	12.13 hrs, Volume=	2,181 cf, A	Atten= 0%, Lag= 0.0 min
Primary	=	0.6 cfs @	12.13 hrs, Volume=	2,181 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.87' @ 12.13 hrs Flood Elev= 290.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.45'	15.0" Round Culvert L= 197.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 285.45' / 282.00' S= 0.0175 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
D .			40.40 km 10.01 005.00 10.00 540 $(D.m.m.i.t.T.i.kt.m)$

Primary OutFlow Max=0.6 cfs @ 12.13 hrs HW=285.86' TW=282.54' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.6 cfs @ 1.73 fps) 203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D2-Year Rainfall=3.13"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 22

Summary for Pond IB-1:

Inflow Area =	89,812 sf, 36.94% Impervious,	Inflow Depth > 1.14" for 2-Year event
Inflow =	2.1 cfs @ 12.17 hrs, Volume=	8,497 cf
Outflow =	1.6 cfs @ 12.26 hrs, Volume=	8,291 cf, Atten= 24%, Lag= 5.6 min
Discarded =	0.0 cfs @ 12.26 hrs, Volume=	647 cf
Primary =	1.5 cfs @ 12.26 hrs, Volume=	7,644 cf
Secondary =	0.0 cfs $\overline{@}$ 0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.46' @ 12.26 hrs Surf.Area= 978 sf Storage= 790 cf

Plug-Flow detention time= 24.1 min calculated for 8,291 cf (98% of inflow) Center-of-Mass det. time= 10.8 min (891.0 - 880.2)

<u>Volume</u>	Invert	Avail.	Storage	Storage Descriptio	n		
#1	284.00'	3	3,366 cf	Custom Stage Da	ta (Irregular)Liste	d below (Recalc)	
Elevatio	on Su	ırf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>	
284.0	00	208	72.5	0	0	208	
285.0	00	661	129.3	413	413	1,126	
286.0	00	1,425	189.1	1,019	1,432	2,649	
287.0	00	2,492	246.3	1,934	3,366	4,643	
Device	Routing	Inve	ert Outle	et Devices			
#1	Discarded	284.0	0' 1.02	0 in/hr Exfiltration	over Surface area	a	
#2	Secondary	285.5	0' 8.0'	long x 15.0' bread	th Broad-Crested	Rectangular Weir	
			Head	d (feet) 0.20 0.40	0.60 0.80 1.00 1	.20 1.40 1.60	
			Coet	f. (English) 2.68 2.	70 2.70 2.64 2.63	3 2.64 2.64 2.63	
#3	Primary	284.5	0' 10.0	" Round Culvert			
			L= 3	0.0' CPP, projectin	ıg, no headwall, K	e= 0.900	
			Inlet	/ Outlet Invert= 284	.50' / 282.50' S=	0.0667 '/' Cc= 0.900	
			n= 0	.013, Flow Area= 0	.55 sf		

Discarded OutFlow Max=0.0 cfs @ 12.26 hrs HW=285.46' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=1.5 cfs @ 12.26 hrs HW=285.46' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 1.5 cfs @ 2.79 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=284.00' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

Summary for Pond IB-2:

Inflow Area =	28,415 sf,	34.30% Impervious,	Inflow Depth > 1.	01" for 2-Year event
Inflow =	0.7 cfs @	12.14 hrs, Volume=	2,401 cf	
Outflow =	0.2 cfs @	12.42 hrs, Volume=	1,827 cf,	Atten= 74%, Lag= 17.1 min
Primary =	0.2 cfs @	12.42 hrs, Volume=	1,827 cf	-
Secondary =	0.0 cfs @	0.00 hrs, Volume=	0 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 282.74' @ 12.42 hrs Surf.Area= 1,201 sf Storage= 765 cf

Plug-Flow detention time= 175.5 min calculated for 1,827 cf (76% of inflow) Center-of-Mass det. time= 71.7 min (959.6 - 887.8)

Volume	Invert A	vail.Storage	Storage Description	on	
#1	282.00'	4,792 cf	Custom Stage Da	ata (Irregular) Liste	ed below (Recalc)
Elevation	Surf.Are	a Perim.	Inc.Store	Cum.Store	Wet.Area
282.00	87	0 143.0	0	0	870
283.00 284.00	1,32 1,84	8 162.0 3 181.0	1,091 1,578	1,091 2,669	1,356 1,902
285.00	2,41	4 200.0	2,122	4,792	2,509

Device	Routing	Invert	Outlet Devices
#1	Secondary	284.00'	10.0' long x 22.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	282.50'	10.0" Round Culvert L= 37.4' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 282.50' / 278.50' S= 0.1070 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=0.2 cfs @ 12.42 hrs HW=282.74' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Inlet Controls 0.2 cfs @ 1.32 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=282.00' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

Summary for Pond IC-1:

Inflow Area	1 =	37,292 sf,	93.64% Impervious,	Inflow Depth >	2.67"	for 2-Ye	ear event
Inflow	=	2.2 cfs @	12.13 hrs, Volume=	8,308 0	of		
Outflow	=	0.1 cfs @	10.85 hrs, Volume=	6,382 c	of, Atten	= 95%,	Lag= 0.0 min
Discarded	=	0.1 cfs @	10.85 hrs, Volume=	6,382 c	of		-
Primary	=	0.0 cfs @	0.00 hrs, Volume=	0 0	of		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 284.74' @ 14.68 hrs Surf.Area= 4,404 sf Storage= 3,540 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 137.6 min (918.2 - 780.6)

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Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
		9,501 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
	-		L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 10.85 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=283.50' TW=0.00' (Dynamic Tailwater) -2=Culvert (Controls 0.0 cfs) -3=Orifice/Grate (Controls 0.0 cfs)

Summary for Pond IC-2:

Inflow Area	a =	52,660 sf,	79.76% Impervi	ous, Inflow Dept	h > 2.26"	for 2-Ye	ar event
Inflow	=	2.7 cfs @	12.13 hrs, Volu	me= 9,9	926 cf		
Outflow	=	0.1 cfs @	14.95 hrs, Volu	me= 6,4	78 cf, Atte	n= 95%,	Lag= 169.3 min
Discarded	=	0.1 cfs @	10.75 hrs, Volu	me= 6,3	350 cf		-
Primary	=	0.0 cfs @	14.95 hrs, Volu	me= ´	29 cf		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.55' @ 14.95 hrs Surf.Area= 4,361 sf Storage= 4,576 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 119.8 min (915.3 - 795.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows

9,414 cf Total Available Storage

Storage Group A created with Chamber Wizard

203 Ayer Road, Harvard, MA NRCC 24-hr D 2-Year Rainfall=3.13" Printed 7/13/2022

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203 Ayer Road, Harvard, MAPOST Rev-1NRCC 24-hr D2-Year Rainfall=3.13"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 25

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
	-		L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 10.75 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=0.0 cfs @ 14.95 hrs HW=288.55' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 0.0 cfs of 0.2 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.0 cfs @ 0.76 fps)

Summary for Link AP-1:

Inflow A	Area	=	270,181 s	sf,	44.92% Im	pervious,	Inflow Depth >	> 0	.54"	for 2-	Year eve	ent
Inflow	=	=	2.1 cfs @	D	12.25 hrs,	Volume=	12,11	0 cf				
Primary	y =	=	2.1 cfs @	Ď	12.25 hrs,	Volume=	12,11	0 cf,	Atter	ו= 0%,	Lag= 0	.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

	203 Ayer Road, Harvard, MA
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>1.87" Flow Length=233' Tc=6.0 min CN=71 Runoff=0.7 cfs 2,377 cf
SubcatchmentSC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>3.07" Flow Length=335' Tc=9.0 min CN=85 Runoff=1.8 cfs 6,758 cf
SubcatchmentSC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>2.26" Flow Length=520' Tc=13.0 min CN=76 Runoff=1.5 cfs 6,320 cf
Subcatchment SC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>4.21" Tc=6.0 min CN=96 Runoff=0.9 cfs 3,256 cf
SubcatchmentSC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>2.70" Flow Length=295' Tc=7.8 min CN=81 Runoff=1.2 cfs 4,175 cf
Subcatchment SC2.3:	Runoff Area=11,540 sf 37.45% Impervious Runoff Depth>2.19" Flow Length=185' Tc=6.0 min CN=75 Runoff=0.6 cfs 2,105 cf
SubcatchmentSC3.1:	Runoff Area=11,615 sf 100.00% Impervious Runoff Depth>4.44" Tc=6.0 min CN=98 Runoff=1.1 cfs 4,297 cf
Subcatchment SC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>4.10" Flow Length=118' Slope=0.0380 '/' Tc=6.0 min CN=95 Runoff=2.3 cfs 8,769 cf
Subcatchment SC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>1.37" Flow Length=216' Tc=6.0 min CN=64 Runoff=0.5 cfs 1,679 cf
Subcatchment SC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>4.44" Tc=6.0 min CN=98 Runoff=1.4 cfs 5,548 cf
Subcatchment SC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>2.61" Flow Length=231' Tc=6.0 min CN=80 Runoff=0.7 cfs 2,282 cf
Subcatchment SC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>4.10" Tc=6.0 min CN=95 Runoff=0.4 cfs 1,599 cf
Subcatchment SC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>3.99" Tc=6.0 min CN=94 Runoff=0.5 cfs 1,707 cf
Subcatchment SC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>1.18" Tc=6.0 min CN=61 Runoff=0.2 cfs 628 cf
SubcatchmentSC5.1:	Runoff Area=62,001 sf 2.46% Impervious Runoff Depth>1.30" Flow Length=362' Tc=10.0 min CN=63 Runoff=1.6 cfs 6,742 cf
Pond CB-1:	Peak Elev=297.98' Inflow=0.7 cfs 2,377 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=0.7 cfs 2,377 cf

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Dond CP 10,	Deak Elev-286 37	' Inflow=0.7 cfc 2.282 cf
Pond CB-10:	12.0" Round Culvert n=0.013 L=5.0' S=0.0400 '/	Outflow=0.7 cfs 2,282 cf
		,
Pond CB-2:	Peak Elev=295.01	' Inflow=1.8 cfs 6,758 cf
	12.0 Round Culvent 11-0.013 L-7.0 3-0.0200 /	
Pond CB-3:	Peak Elev=288.30	' Inflow=1.5 cfs 6,320 cf
	12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/	Outflow=1.5 cfs 6,320 cf
Pond CB-5:	Peak Elev=294.65	' Inflow=1.2 cfs 4,175 cf
	12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/'	Outflow=1.2 cfs 4,175 cf
Bond CB 6:	Peak Elev-206 04	' Inflow-0.0 cfc 3.256 cf
	12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/'	Outflow=0.9 cfs 3,256 cf
Pond CB-7:	Peak Elev=288.93 12 0" Round Culvert n=0.013 L=22 0' S=0.0136 '/'	' Inflow=0.4 cfs $1,599$ cf
		041101 0.4 010 1,000 01
Pond CB-8:	Peak Elev=288.93	' Inflow=0.5 cfs 1,707 cf
	12.0" Round Cuivert n=0.013 L=15.0 S=0.0200 7	Outflow=0.5 cts 1,707 ct
Pond CB-9:	Peak Elev=286.95	' Inflow=0.6 cfs 2,105 cf
	12.0" Round Culvert n=0.013 L=40.0' S=0.0200 '/'	Outflow=0.6 cfs 2,105 cf
Pond DCB-4:	Peak Elev=285.71	' Inflow=2.3 cfs 8.769 cf
	12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/'	Outflow=2.3 cfs 8,769 cf
Dand DML 4.	Dook Elov=204 65	' Inflow-2 5 of 0 124 of
Pond DMH-1:	12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/'	Outflow=2.5 cfs 9,134 cf
Pond DMH-2:	Peak Elev=287.98' 18.0" Round Culvert n=0.013 L=48.0' S=0.0100.1/' (Inflow=3.8 cfs 15,454 cf Outflow=3.8 cfs 15,454 cf
		Juliow 0.0 013 10,404 01
Pond DMH-3:	Peak Elev=285.71	' Inflow=2.3 cfs 8,769 cf
	24.0" Round Culvert n=0.013 L=5.0 S=0.0200 7	
Pond DMH-5:	Peak Elev=289.95	' Inflow=2.0 cfs 7,431 cf
	12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/'	Outflow=2.0 cfs 7,431 cf
Pond DMH-6:	Peak Elev=288.93	' Inflow=2.4 cfs 9,030 cf
	24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/'	Outflow=2.4 cfs 9,030 cf
Pond DMH-7	Deak Flov=288 02	' Inflow=0.5 cfs 1.707 cf
	24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/'	Outflow=0.5 cfs 1,697 cf
Pona DMH-9:	Peak Elev=286.08 15.0" Round Culvert n=0.013 L=197 0' S=0.0175 '/'	Outflow=1.3 cfs 4,387 cf
Pond IB-1:	Peak Elev=285.72' Storage=1,069 cf	Inflow=4.3 cfs 17,133 cf

Pond IB-1: Peak Elev=285.72' Storage=1,069 cf Inflow=4.3 cfs 17,133 cf Discarded=0.0 cfs 788 cf Primary=1.9 cfs 14,604 cf Secondary=2.2 cfs 1,511 cf Outflow=4.1 cfs 16,903 cf

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Pond IB-2:	Peak Elev=283.11' Storage=1,24 Primary=0.9 cfs 4,410 cf Secondary=0.0 cfs 0	43 cf Inflow=1.5 cfs 5,014 cf) cf Outflow=0.9 cfs 4,410 cf
Pond IC-1:	Peak Elev=285.71' Storage=6,71 Discarded=0.1 cfs 7,157 cf Primary=0.0 cfs 0	0 cf Inflow=3.4 cfs 13,060 cf) cf Outflow=0.1 cfs 7,157 cf
Pond IC-2: Dis	Peak Elev=288.92' Storage=5,77 carded=0.1 cfs 7,126 cf Primary=1.0 cfs 4,880	6 cf Inflow=4.3 cfs 16,275 cf cf Outflow=1.1 cfs 12,006 cf
Link AP-1:		Inflow=7.0 cfs 32,146 cf Primary=7.0 cfs 32,146 cf

Total Runoff Area = 270,181 sf Runoff Volume = 58,240 cf Average Runoff Depth = 2.59" 55.08% Pervious = 148,810 sf 44.92% Impervious = 121,371 sf

Summary for Subcatchment SC1.1:

Runoff = 0.7 cfs @ 12.13 hrs, Volume=	2,377 cf, Depth>	1.87"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN	Description						
	4,381	61	31 >75% Grass cover, Good, HSG B						
	5,583	55	Woods, Go	od, HSG B					
	328	98	Unconnecte	ed pavemer	nt, HSG B				
	3,654	98	Paved park	ing, HSG B					
	1,269	96	Gravel surfa	ace, HSG E	3				
	15,215	71	Weighted A	verage					
	11,233		73.83% Per	vious Area					
	3,982		26.17% Imp	pervious Are	ea				
	328		8.23% Unco	onnected					
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
3.4	34	0.2400	0.17		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
0.9	16	0.1875	5 0.28		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.12"				
1.0	83	0.036	l 1.33		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.5	100	0.0300) 3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
5.8	233	Total,	Increased t	o minimum	Tc = 6.0 min				

Summary for Subcatchment SC1.2:

Runoff = 1.8 cfs @ 12.16 hrs, Volume= 6,758 cf, Depth> 3.07"

Area (sf)	CN	Description
6,620	61	>75% Grass cover, Good, HSG B
2,398	55	Woods, Good, HSG B
1,172	98	Unconnected pavement, HSG B
8,000	98	Roofs, HSG B
7,203	98	Paved parking, HSG B
1,052	96	Gravel surface, HSG B
26,444	85	Weighted Average
10,069		38.08% Pervious Area
16,375		61.92% Impervious Area
1,172		7.16% Unconnected
	Area (sf) 6,620 2,398 1,172 8,000 7,203 1,052 26,444 10,069 16,375 1,172	Area (sf) CN 6,620 61 2,398 55 1,172 98 8,000 98 7,203 98 1,052 96 26,444 85 10,069 16,375 1,172 1,172

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.9	41	0.2400	0.17		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
1.4	9	0.0200	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
3.4	222	0.0248	1.10		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	63	0.0300	3.52		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
9.0	335	Total			

Summary for Subcatchment SC1.3:

Runoff	=	1.5 cfs @	12.21 hrs,	Volume=	6,320 cf, Depth>	2.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN I	Description					
	15,029	61 >	61 >75% Grass cover, Good, HSG B					
	3,894	55 \	Noods, Go	od, HSG B				
	1,741	96 (Gravel surfa	ace, HSG E	3			
	275	98 l	Jnconnecte	ed pavemer	nt, HSG B			
	12,548	98 I	Paved park	ing, HSG B				
	33,487	76 \	Neighted A	verage				
	20,664	6	61.71% Pei	vious Area				
	12,823	3	38.29% Imp	pervious Are	ea			
	275		2.14% Unco	onnected				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.2	50	0.0800	0.12		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.12"			
3.2	188	0.0372	0.96		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
2.0	160	0.0372	1.35		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.6	122	0.0300	3.52		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
13.0	520	Total						

Summary for Subcatchment SC2.1:

Runoff	=	0.9 cfs @	12.13 hrs,	Volume=	3,256 cf,	Depth> 4.21"
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203 Ayer Road, Harvard, MA NRCC 24-hr D 10-Year Rainfall=4.68" Printed 7/13/2022 HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC Page 31

Area (sf) CN Description >75% Grass cover, Good, HSG B 595 61 Unconnected pavement, HSG B 423 98 Roofs, HSG B 4,000 98 4,263 98 Paved parking, HSG B Weighted Average 9,281 96 6.41% Pervious Area 595 8,686 93.59% Impervious Area 4.87% Unconnected 423 Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, 5.0 0 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment SC2.2:

Runoff 1.2 cfs @ 12.15 hrs, Volume= 4,175 cf, Depth> 2.70" =

Α	rea (sf)	CN [Description						
	5,529	61 >	61 >75% Grass cover, Good, HSG B						
	2,637	55 N	Voods, Go	od, HSG B					
	889	98 l	Jnconnecte	ed pavemer	nt, HSG B				
	4,000	98 F	Roofs, HSC	βB					
	4,580	98 F	Paved park	ing, HSG B					
	929	96 (Gravel surfa	ace, HSG E	3				
	18,563	81 V	Veighted A	verage					
	9,094	Z	l8.99% Pei	vious Area					
	9,469	5	51.01% Imp	pervious Ar	ea				
	889	ç	9.39% Unc	onnected					
_									
TC	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
4.6	50	0.2400	0.18		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
0.2	8	0.0200	0.71		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
2.5	151	0.0200	0.99		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.5	86	0.0200	2.87		Shallow Concentrated Flow,				
					Paved Kv= 20.3 tps				
7.8	295	Total							

Summary for Subcatchment SC2.3:

Runoff = 0.6 cfs @ 12.13 hrs, Volume= 2,105 cf, Depth> 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

Α	rea (sf)	CN [Description		
	7,218	61 >	75% Gras	s cover, Go	ood, HSG B
	293	98 l	Jnconnecte	ed pavemer	nt, HSG B
	4,029	98 F	Paved park	ing, HSG B	
	11,540	75 V	Veighted A	verage	
	7,218	6	62.55% Per	vious Area	
	4,321	3	87.45% Imp	pervious Ar	ea
	293	6	6.77% Unco	onnected	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
2.3	42	0.1430	0.31		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.1	8	0.0500	1.18		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 3.12"
0.5	135	0.0500	4.54		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
2.9	185	Total, I	ncreased t	o minimum	1 Tc = 6.0 min

Summary for Subcatchment SC3.1:

Runoff = 1.1 cfs @ 12.13 hrs, Volume= 4,297 cf, Depth> 4.44"

Ar	rea (sf)	CN I	Description					
	11,615	98	Roofs, HSG	βB				
	0	61 :	>75% Gras	s cover, Go	bod, HSG B			
	11,615	98	98 Weighted Average					
	0	(0.00% Perv	ious Area				
	11,615		100.00% Impervious Area					
Тс	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0		Direct Entry,						
5.0	0	Total, Increased to minimum Tc = 6.0 min						

Summary for Subcatchment SC3.2:

Runoff = 2.3 cfs @ 12.13 hrs, Volume= 8,769 cf, Depth> 4.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN Description					
	2,372	61 :	>75% Gras	s cover, Go	ood, HSG B		
	21,883	98 I	Paved park	ing, HSG B			
	1,422	98	Jnconnecte	ed pavemer	nt, HSG B		
	25,677	95 \	Neighted A	verage			
	2,372	ę	9.24% Perv	ious Area			
	23,305	ę	90.76% Imp	pervious Are	ea		
	1,422	(6.10% Unco	onnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
4.4	50	0.0380	0.19		Sheet Flow,		
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
5.2	118	Total,	Increased t	o minimum	Tc = 6.0 min		

Summary for Subcatchment SC3.3:

Runoff = 0.5 cfs @ 12.14 hrs, Volume= 1,679 cf, Depth> 1.37"

A	rea (sf)	CN I	Description		
	13,540	61 ;	>75% Gras	s cover, Go	ood, HSG B
	1,127	96 (Gravel surfa	ace, HSG E	3
	14,666	64	Neighted A	verage	
	14,666		100.00% Pe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.7	50	0.0600	0.23		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
1.9	166	0.0422	1.44		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
5.6	216	Total,	Increased t	o minimum	Tc = 6.0 min

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PUSI I	≺ev- 1 d by Col	domith I	Dreat 9 Di	novell in	-	NRUU Z	4-nr D	Drinted 7/12/2022
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ITYUIUCA		4a 5/1101	030 @ 2020		Soltware Solu			Fage 34
			Sum	mary for	Subcatchm	ent SC4.	1:	
Runoff	=	1.4 ct	fs @ 12.1	3 hrs, Volu	ume=	5,548 cf,	Depth>	4.44"
Runoff b NRCC 2	y SCS TF 4-hr D 10	R-20 meth)-Year Ra	nod, UH=S ainfall=4.68	CS, Weigh 5"	ted-CN, Time	Span= 0.00	0-24.00 h	rs, dt= 0.05 hrs
A	rea (sf)	CN D	escription					
	14,998	98 R	Roofs, HSG	В				
	14,998	1	00.00% Im	pervious A	rea			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0			· · ·		Direct Entry	•		
5.0	0	Total, I	ncreased to	o minimum	Tc = 6.0 min	•		
		,						
			Sum	mary for	Subcatchm	ent SC4.	2:	
Runoff	=	0.7 ct	fs @ 12.1	3 hrs, Volu	ume=	2,282 cf,	Depth>	2.61"
Runoff b NRCC 2	y SCS TF 4-hr D 1(R-20 metł)-Year Ra	nod, UH=S ainfall=4.68	CS, Weigh 5"	ted-CN, Time	Span= 0.00	0-24.00 h	rs, dt= 0.05 hrs
A	rea (sf)	CN D	escription					
	5,060	61 >	75% Grass	s cover, Go	od, HSG B			
	1,450	98 U	Inconnecte	d pavemer	nt, HSG B			
	3,975	98 P	aved parki	ng, HSG B	1			
	10,484	80 V	Veighted A	verage				
	5,060	4	8.26% Per	vious Area				
	5,424	5	1.74% Imp	ervious Are	ea			
	1,450	2	0.72% 010	onnecleu				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.0	50	0.0500	0.21		Sheet Flow,	n = 0.150	D0- 2 /	10"
0.4	33	0.0500	1 57		Shallow Cor	UCI.U –II Contrated	PZ-J.	12
0.4	00	0.0000	1.07		Short Grass	Pasture K	(v= 7.0 fp	S
0.6	148	0.0440	4.26		Shallow Cor Paved Kv=	ncentrated 20.3 fps	I Flow,	
	004	Tatal				•		

5.0 231 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment SC4.3:

Runoff = 0.4 cfs @ 12.13 hrs, Volume= 1,599 cf, Depth> 4.10"

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Are	ea (sf)	CN	Description	
	406	61	>75% Grass cover, Good, HSG B	
	175	98	Unconnected pavement, HSG B	
	4,101	98	Paved parking, HSG B	
	4,682	95	Weighted Average	
	4 276		91.32% Impervious Area	
	175		4.09% Unconnected	
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity Capacity Description t) (ft/sec) (cfs)	
5.0			Direct Entry,	
5.0	0	Total,	Increased to minimum Tc = 6.0 min	

Summary for Subcatchment SC4.4:

Runoff = 0.5 cfs @ 12.13 hrs, Volume= 1,707 cf, Depth> 3.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 10-Year Rainfall=4.68"

A	rea (sf)	CN	Description		
	564	61	>75% Grass	s cover, Go	ood, HSG B
	541	98	Unconnecte	ed pavemer	nt, HSG B
	4,030	98	Paved park	ing, HSG B	3
	5,136	94	Weighted A	verage	
	564		10.99% Per	vious Area	9
	4,571		89.01% Imp	pervious Are	ea
	541		11.83% Und	connected	
-		0		0	
IC	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/fi	t) (ft/sec)	(cts)	
5.0					Direct Entry,
5.0	0	Total,	Increased t	o minimum	n Tc = 6.0 min

Summary for Subcatchment SC4.5:

Runoff = 0.2 cfs @ 12.14 hrs, Volume= 628 cf, Depth> 1.18"

Area (sf)	CN	Description
6,391	61	>75% Grass cover, Good, HSG B
6,391		100.00% Pervious Area

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HydroCA	D® 10.10-	4a s/n 01	036 © 202	0 HydroCAE	c.) Software Solu	tions LLC	Plinted 7/13/2022 Page 36
Tc (min)	Length	Slope	Velocity	Capacity	Description		
5.0	(1001)	(1011)	(10300)	(013)	Direct Entry	· · · · · · · · · · · · · · · · · · ·	
5.0	0	Total, Ir	ncreased t	o minimum	Tc = 6.0 min	,	
			Sum	mary for	Subcatchm	ent SC5.1:	
Runoff	=	1.6 cf	^r s @ 12.1	9 hrs, Volu	ume=	6,742 cf, Depth> 1.3	30"
Runoff b NRCC 2	y SCS TF 4-hr D 10	R-20 meth)-Year Ra	nod, UH=S ainfall=4.68	CS, Weigh 3"	ted-CN, Time	Span= 0.00-24.00 hrs,	dt= 0.05 hrs
A	rea (sf)	CN D	escription				
	52,172 5,271 70 1,456 3,033	61 > 55 W 98 U 98 P 96 G	75% Gras /oods, Go nconnecte aved park avel surfa	s cover, Go od, HSG B ed pavemer ing, HSG B ace, HSG B	ood, HSG B nt, HSG B		
	62,001 60,476 1,525 70	63 W 9 2 4	/eighted A 7.54% Per .46% Impe .56% Unco	verage vious Area ervious Area onnected	а		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
3.8	36	0.2000	0.16		Sheet Flow,	•	
2.1	14	0.0200	0.11		Sheet Flow,	n = 0.150 P2= 3.12"	J P2- 3.12
4.1	312	0.0334	1.28		Shallow Cor Short Grass	ncentrated Flow, Pasture Kv= 7.0 fps	
10.0	362	Total				·	
				Summar	y for Pond	CB-1:	
Inflow Ai Inflow Outflow Primary	rea = = = =	15,2 [,] 0.7 cf 0.7 cf 0.7 cf	15 sf, 26. s @ 12.1 s @ 12.1 s @ 12.1	17% Imperv 13 hrs, Volu 13 hrs, Volu 13 hrs, Volu	/ious, Inflow E ume= ume= ume=	Depth > 1.87" for 10 2,377 cf 2,377 cf, Atten= 0%, 2,377 cf)-Year event Lag= 0.0 min
Routing Peak Ele Flood El	by Dyn-Si ev= 297.9 ev= 301.5	tor-Ind m 8' @ 12.1 50'	ethod, Tim 3 hrs	ie Span= 0.	.00-24.00 hrs,	dt= 0.05 hrs / 2	
Device	Routing		Invert O	utlet Devic	es		
#1	Primary	2	97.50' 1 : L: Ir n:	2.0" Roun = 170.0' C nlet / Outlet = 0.013 Co	d Culvert PP, projecting Invert= 297.50 prrugated PE, s	, no headwall, Ke= 0. 0' / 293.56' S= 0.0232 smooth interior, Flow /	900 ? '/' Cc= 0.900 Area= 0.79 sf

Primary OutFlow Max=0.7 cfs @ 12.13 hrs HW=297.97' TW=294.58' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 0.7 cfs @ 1.85 fps)

Summary for Pond CB-10:

Inflow Area	a =	10,484 sf,	51.74% Impervious,	Inflow Depth > 2	2.61" for 10-Year event
Inflow	=	0.7 cfs @	12.13 hrs, Volume=	2,282 cf	
Outflow	=	0.7 cfs @	12.13 hrs, Volume=	2,282 cf	, Atten= 0%, Lag= 0.0 min
Primary	=	0.7 cfs @	12.13 hrs, Volume=	2,282 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.37' @ 12.13 hrs Flood Elev= 289.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.90'	12.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.90' / 285.70' S= 0.0400 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.6 cfs @ 12.13 hrs HW=286.36' TW=286.06' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.6 cfs @ 1.83 fps)

Summary for Pond CB-2:

Inflow Area	a =	26,444 sf,	61.92% Impervious,	Inflow Depth > 3.	.07" for 10-Year event
Inflow	=	1.8 cfs @	12.16 hrs, Volume=	6,758 cf	
Outflow	=	1.8 cfs @	12.16 hrs, Volume=	6,758 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.8 cfs @	12.16 hrs, Volume=	6,758 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 295.01' @ 12.15 hrs Flood Elev= 297.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.70'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
D		Max-1 0 ata	12.16 hrs. $ $

Primary OutFlow Max=1.8 cfs @ 12.16 hrs HW=294.96' TW=294.61' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.8 cfs @ 2.24 fps)

Summary for Pond CB-3:

Inflow Are	a =	33,487 sf,	38.29% Impervious,	Inflow Depth > 2.26" for 10-Year event	
Inflow	=	1.5 cfs @	12.21 hrs, Volume=	6,320 cf	
Outflow	=	1.5 cfs @	12.21 hrs, Volume=	6,320 cf, Atten= 0%, Lag= 0.0 m	nin
Primary	=	1.5 cfs @	12.21 hrs, Volume=	6,320 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.30' @ 12.21 hrs Flood Elev= 291.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.50'	12.0" Round Culvert L= 8.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.5 cfs @ 12.21 hrs HW=288.29' TW=287.92' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 1.5 cfs @ 3.01 fps)

Summary for Pond CB-5:

Inflow Are	a =	18,563 sf,	51.01% Impervious,	Inflow Depth > 2.70	" for 10-Year event
Inflow	=	1.2 cfs @	12.15 hrs, Volume=	4,175 cf	
Outflow	=	1.2 cfs @	12.15 hrs, Volume=	4,175 cf, A	tten= 0%, Lag= 0.0 min
Primary	=	1.2 cfs @	12.15 hrs, Volume=	4,175 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.65' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.2 cfs @ 12.15 hrs HW=294.65' TW=289.94' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.2 cfs @ 2.17 fps)

Summary for Pond CB-6:

Inflow Area	a =	9,281 sf,	93.59% Impervious,	Inflow Depth > 4	.21" for 10-Year event
Inflow	=	0.9 cfs @	12.13 hrs, Volume=	3,256 cf	
Outflow	=	0.9 cfs @	12.13 hrs, Volume=	3,256 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	0.9 cfs @	12.13 hrs, Volume=	3,256 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 296.04' @ 12.13 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	12.0" Round Culvert L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.8 cfs @ 12.13 hrs HW=296.03' TW=289.91' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.8 cfs @ 1.95 fps)

Summary for Pond CB-7:

Inflow Area	a =	4,682 sf,	91.32% Impervious,	Inflow Depth > 4.10)" for 10-Year event
Inflow	=	0.4 cfs @	12.13 hrs, Volume=	1,599 cf	
Outflow	=	0.4 cfs @	12.13 hrs, Volume=	1,599 cf, A	tten= 0%, Lag= 0.0 min
Primary	=	0.4 cfs @	12.13 hrs, Volume=	1,599 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.93' @ 12.42 hrs Flood Elev= 292.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.00'	12.0" Round Culvert L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.0 cfs @ 12.13 hrs HW=288.56' TW=288.62' (Dynamic Tailwater) **1=Culvert** (Controls 0.0 cfs)

Summary for Pond CB-8:

Inflow Area	a =	5,136 sf,	89.01% Impervious,	Inflow Depth > 3.	.99" for 10-Year event
Inflow	=	0.5 cfs @	12.13 hrs, Volume=	1,707 cf	
Outflow	=	0.5 cfs @	12.13 hrs, Volume=	1,707 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	0.5 cfs @	12.13 hrs, Volume=	1,707 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.93' @ 12.42 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.50'	12.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.4 cfs @ 12.13 hrs HW=288.88' TW=288.48' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.4 cfs @ 1.65 fps)

Summary for Pond CB-9:

Inflow Area	a =	11,540 sf,	37.45% Impervious,	Inflow Depth > 2	2.19" for 10-Year event
Inflow	=	0.6 cfs @	12.13 hrs, Volume=	2,105 cf	
Outflow	=	0.6 cfs @	12.13 hrs, Volume=	2,105 cf	Atten= 0%, Lag= 0.0 min
Primary	=	0.6 cfs @	12.13 hrs, Volume=	2,105 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.95' @ 12.13 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.50'	12.0" Round Culvert
			L= 40.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 286.50' / 285.70' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.6 cfs @ 12.13 hrs HW=286.94' TW=286.06' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.6 cfs @ 1.79 fps)

Summary for Pond DCB-4:

Inflow Are	a =	25,677 sf,	90.76% Impervious,	Inflow Depth > 4.10"	for 10-Year event
Inflow	=	2.3 cfs @	12.13 hrs, Volume=	8,769 cf	
Outflow	=	2.3 cfs @	12.13 hrs, Volume=	8,769 cf, Atter	n= 0%, Lag= 0.0 min
Primary	=	2.3 cfs @	12.13 hrs, Volume=	8,769 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.71' @ 16.83 hrs Flood Elev= 288.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.50'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.2 cfs @ 12.13 hrs HW=285.57' TW=284.94' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.2 cfs @ 2.86 fps)

Summary for Pond DMH-1:

Inflow Are	ea =	41,659 sf,	48.86% Impervious,	Inflow Depth > 2.63" for 10-Year event	
Inflow	=	2.5 cfs @	12.15 hrs, Volume=	9,134 cf	
Outflow	=	2.5 cfs @	12.15 hrs, Volume=	9,134 cf, Atten= 0%, Lag= 0.0 min	
Primary	=	2.5 cfs @	12.15 hrs, Volume=	9,134 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.65' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	12.0" Round Culvert L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.5 cfs @ 12.15 hrs HW=294.64' TW=287.97' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.5 cfs @ 3.14 fps)

Summary for Pond DMH-2:

Inflow Are	ea =	75,146 sf,	44.15% Impervious,	Inflow Depth > 2.4	7" for 10-Year event
Inflow	=	3.8 cfs @	12.17 hrs, Volume=	15,454 cf	
Outflow	=	3.8 cfs @	12.17 hrs, Volume=	15,454 cf, A	Atten= 0%, Lag= 0.0 min
Primary	=	3.8 cfs @	12.17 hrs, Volume=	15,454 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.98' @ 12.17 hrs Flood Elev= 291.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.90'	18.0" Round Culvert L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.7 cfs @ 12.17 hrs HW=287.96' TW=285.71' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.7 cfs @ 2.77 fps)

Summary for Pond DMH-3:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth > 4	.10" for 10-Year event
Inflow	=	2.3 cfs @	12.13 hrs, Volume=	8,769 cf	
Outflow	=	2.3 cfs @	12.13 hrs, Volume=	8,763 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	2.3 cfs @	12.13 hrs, Volume=	8,763 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

Peak Elev= 285.71' @ 16.79 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.10'	24.0" Round Culvert L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.3 cfs @ 12.13 hrs HW=284.94' TW=284.71' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.3 cfs @ 1.82 fps)

Summary for Pond DMH-5:

Inflow Area	=	27,844 sf,	65.20% Impervious,	Inflow Depth >	3.20" f	or 10-Year event
Inflow	=	2.0 cfs @	12.14 hrs, Volume=	7,431 c	f	
Outflow	=	2.0 cfs @	12.14 hrs, Volume=	7,431 c	f, Atten=	= 0%, Lag= 0.0 min
Primary	=	2.0 cfs @	12.14 hrs, Volume=	7,431 c	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.95' @ 12.14 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	289.00'	12.0" Round Culvert L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.0 cfs @ 12.14 hrs HW=289.92' TW=288.66' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.0 cfs @ 2.58 fps)

Summary for Pond DMH-6:

Inflow Area	a =	32,526 sf,	68.96% Impervious,	Inflow Depth > 3	3.33" for 10-Year event
Inflow	=	2.4 cfs @	12.14 hrs, Volume=	9,030 cf	
Outflow	=	2.4 cfs @	12.14 hrs, Volume=	9,030 cf.	, Atten= 0%, Lag= 0.0 min
Primary	=	2.4 cfs @	12.14 hrs, Volume=	9,030 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.93' @ 12.37 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.4 cfs @ 12.14 hrs HW=288.65' TW=288.52' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.4 cfs @ 1.41 fps)

Summary for Pond DMH-7:

Inflow Are	ea =	5,136 sf,	89.01% Impervious,	Inflow Depth > 3.9	9" for 10-Year event
Inflow	=	0.5 cfs @	12.13 hrs, Volume=	1,707 cf	
Outflow	=	0.5 cfs @	12.13 hrs, Volume=	1,697 cf, /	Atten= 0%, Lag= 0.0 min
Primary	=	0.5 cfs @	12.13 hrs, Volume=	1,697 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.92' @ 12.38 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.5 cfs @ 12.13 hrs HW=288.48' TW=288.47' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.5 cfs @ 0.34 fps)

Summary for Pond DMH-9:

Inflow Are	ea =	22,024 sf,	44.25% Impervious,	Inflow Depth > 2.3	39" for 10-Year event
Inflow	=	1.3 cfs @	12.13 hrs, Volume=	4,387 cf	
Outflow	=	1.3 cfs @	12.13 hrs, Volume=	4,387 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.3 cfs @	12.13 hrs, Volume=	4,387 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.08' @ 12.13 hrs Flood Elev= 290.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.45'	15.0" Round Culvert L= 197.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 285.45' / 282.00' S= 0.0175 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
D '			40.40 km 10.01 000.001 $10.000.001$ (Demonstration)

Primary OutFlow Max=1.2 cfs @ 12.13 hrs HW=286.06' TW=283.02' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.2 cfs @ 2.10 fps) 203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D10-Year Rainfall=4.68"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 44

Summary for Pond IB-1:

Inflow Area =	89,812 sf,	36.94% Impervious,	Inflow Depth > 2.29"	for 10-Year event
Inflow =	4.3 cfs @	12.16 hrs, Volume=	17,133 cf	
Outflow =	4.1 cfs @	12.19 hrs, Volume=	16,903 cf, Atte	en= 3%, Lag= 1.5 min
Discarded =	0.0 cfs @	12.19 hrs, Volume=	788 cf	
Primary =	1.9 cfs @	12.19 hrs, Volume=	14,604 cf	
Secondary =	2.2 cfs @	12.19 hrs, Volume=	1,511 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.72' @ 12.19 hrs Surf.Area= 1,183 sf Storage= 1,069 cf

Plug-Flow detention time= 16.1 min calculated for 16,868 cf (98% of inflow) Center-of-Mass det. time= 8.4 min (864.8 - 856.4)

<u>Volume</u>	Invert	Avail.St	orage	Storage Description	า	
#1	284.00'	3,5	366 cf	Custom Stage Dat	ta (Irregular) Listed	d below (Recalc)
Elevatio	on Su	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
284.0	00	208	72.5	0	0	208
285.0	00	661	129.3	413	413	1,126
286.0	00	1,425	189.1	1,019	1,432	2,649
287.0	00	2,492	246.3	1,934	3,366	4,643
Device	Routing	Inver	t Outle	et Devices		
#1	Discarded	284.00	' 1.02	0 in/hr Exfiltration of	over Surface area	1
#2	Secondary	285.50	' 8. 0'	long x 15.0' breadt	h Broad-Crested	Rectangular Weir
			Head	d (feet) 0.20 0.40 ().60 0.80 1.00 1.	20 1.40 1.60
			Coel	f. (English) 2.68 2.7	0 2.70 2.64 2.63	3 2.64 2.64 2.63
#3	Primary	284.50	10.0	" Round Culvert		
			L= 3	0.0' CPP, projecting	g, no headwall, Ko	e= 0.900
			Inlet	/ Outlet Invert= 284.	.50' / 282.50' S= (0.0667 '/' Cc= 0.900
			n= 0	.013, Flow Area= 0.	.55 sf	

Discarded OutFlow Max=0.0 cfs @ 12.19 hrs HW=285.72' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=1.9 cfs @ 12.19 hrs HW=285.72' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 1.9 cfs @ 3.40 fps)

Secondary OutFlow Max=2.2 cfs @ 12.19 hrs HW=285.72' TW=0.00' (Dynamic Tailwater) —2=Broad-Crested Rectangular Weir (Weir Controls 2.2 cfs @ 1.25 fps)

Summary for Pond IB-2:

Inflow Area =	28,415 sf, 34.30% Impervi	ous, Inflow Depth > 2.12"	for 10-Year event
Inflow =	1.5 cfs @ 12.13 hrs, Volu	me= 5,014 cf	
Outflow =	0.9 cfs @ 12.22 hrs, Volu	me= 4,410 cf, Atte	en= 39%, Lag= 5.2 min
Primary =	0.9 cfs @ 12.22 hrs, Volu	me= 4,410 cf	-
Secondary =	0.0 cfs @ 0.00 hrs, Volu	me= 0 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 283.11' @ 12.22 hrs Surf.Area= 1,382 sf Storage= 1,243 cf

Plug-Flow detention time= 100.9 min calculated for 4,400 cf (88% of inflow) Center-of-Mass det. time= 40.9 min (902.8 - 861.9)

#1	282.00'	4,792 cf	Custom Stage Dat	ta (Irregular) Listed	below (Recald
Elevation (feet)	Surf.Are (sq-1	ea Perim. ft) (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
282.00	87	0 143.0	0	0	870
283.00	1,32	162.0	1,091	1,091	1,356
284.00	1,84	3 181.0	1,578	2,669	1,902
285.00	2,41	4 200.0	2,122	4,792	2,509

Device	Rouling	mven	Outlet Devices
#1	Secondary	284.00'	10.0' long x 22.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	282.50'	10.0" Round Culvert L= 37.4' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 282.50' / 278.50' S= 0.1070 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=0.9 cfs @ 12.22 hrs HW=283.11' TW=0.00' (Dynamic Tailwater) 2=Culvert (Inlet Controls 0.9 cfs @ 2.09 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=282.00' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

Summary for Pond IC-1:

Inflow Area	n =	37,292 sf,	93.64% Impervious,	Inflow Depth > 4	.20" for 10-ነ	ear event
Inflow	=	3.4 cfs @	12.13 hrs, Volume=	13,060 cf		
Outflow	=	0.1 cfs @	9.60 hrs, Volume=	7,157 cf,	, Atten= 97%,	Lag= 0.0 min
Discarded	=	0.1 cfs @	9.60 hrs, Volume=	7,157 cf		
Primary	=	0.0 cfs @	0.00 hrs, Volume=	0 cf		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.71' @ 16.79 hrs Surf.Area= 4,404 sf Storage= 6,710 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 91.6 min (859.4 - 767.8)

203 Ayer Road, Harvard, MA NRCC 24-hr D 10-Year Rainfall=4.68" Printed 7/13/2022

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POST Rev-1

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Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
		9.501 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
	-		L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 9.60 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=283.50' TW=0.00' (Dynamic Tailwater) -2=Culvert (Controls 0.0 cfs) -3=Orifice/Grate (Controls 0.0 cfs)

Summary for Pond IC-2:

Inflow Area	ı =	52,660 sf,	79.76% Impervious	, Inflow Depth > 3	.71" for 10-Year event
Inflow	=	4.3 cfs @	12.13 hrs, Volume	= 16,275 cf	
Outflow	=	1.1 cfs @	12.38 hrs, Volume	= 12,006 cf,	Atten= 74%, Lag= 14.8 min
Discarded	=	0.1 cfs @	9.30 hrs, Volume	= 7,126 cf	-
Primary	=	1.0 cfs @	12.38 hrs, Volume	= 4,880 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.92' @ 12.38 hrs Surf.Area= 4,361 sf Storage= 5,776 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 57.8 min (842.0 - 784.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows

9,414 cf Total Available Storage

Storage Group A created with Chamber Wizard

203 Ayer Road, Harvard, MAPOST Rev-1NRCC 24-hr D10-Year Rainfall=4.68"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 47

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
	·		L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 9.30 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=1.0 cfs @ 12.38 hrs HW=288.92' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 1.0 cfs of 1.1 cfs potential flow) -3=Orifice/Grate (Orifice Controls 1.0 cfs @ 2.20 fps)

Summary for Link AP-1:

Inflow .	Area	1 =	270,181 sf,	44.92% Im	pervious,	Inflow Depth >	1.43	3" for 10	-Year event
Inflow		=	7.0 cfs @	12.20 hrs,	Volume=	32,146	cf		
Primar	y	=	7.0 cfs @	12.20 hrs,	Volume=	32,146	cf, A	tten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

	203 Ayer Road, Harvard, MA
POST Rev-1	NRCC 24-hr D 50-Year Rainfall=7.00"
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HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solu	tions LLC Page 48

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>3.72" Flow Length=233' Tc=6.0 min CN=71 Runoff=1.4 cfs 4,714 cf
Subcatchment SC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>5.24" Flow Length=335' Tc=9.0 min CN=85 Runoff=3.0 cfs 11,551 cf
Subcatchment SC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>4.24" Flow Length=520' Tc=13.0 min CN=76 Runoff=2.8 cfs 11,836 cf
Subcatchment SC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>6.52" Tc=6.0 min CN=96 Runoff=1.3 cfs 5,040 cf
Subcatchment SC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>4.80" Flow Length=295' Tc=7.8 min CN=81 Runoff=2.0 cfs 7,419 cf
Subcatchment SC2.3:	Runoff Area=11,540 sf 37.45% Impervious Runoff Depth>4.14" Flow Length=185' Tc=6.0 min CN=75 Runoff=1.2 cfs 3,984 cf
Subcatchment SC3.1:	Runoff Area=11,615 sf 100.00% Impervious Runoff Depth>6.75" Tc=6.0 min CN=98 Runoff=1.6 cfs 6,537 cf
Subcatchment SC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>6.40" Flow Length=118' Slope=0.0380 '/' Tc=6.0 min CN=95 Runoff=3.6 cfs 13,692 cf
Subcatchment SC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>3.00" Flow Length=216' Tc=6.0 min CN=64 Runoff=1.1 cfs 3,662 cf
Subcatchment SC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>6.75" Tc=6.0 min CN=98 Runoff=2.1 cfs 8,442 cf
Subcatchment SC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>4.69" Flow Length=231' Tc=6.0 min CN=80 Runoff=1.2 cfs 4,096 cf
Subcatchment SC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>6.40" Tc=6.0 min CN=95 Runoff=0.7 cfs 2,497 cf
Subcatchment SC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>6.28" Tc=6.0 min CN=94 Runoff=0.7 cfs 2,688 cf
Subcatchment SC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>2.70" Tc=6.0 min CN=61 Runoff=0.4 cfs 1,436 cf
Subcatchment SC5.1:	Runoff Area=62,001 sf 2.46% Impervious Runoff Depth>2.89" Flow Length=362' Tc=10.0 min CN=63 Runoff=3.8 cfs 14,937 cf
Pond CB-1:	Peak Elev=298.23' Inflow=1.4 cfs 4,714 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=1.4 cfs 4,714 cf

203 Ayer Road, Harvard, MA NRCC 24-hr D 50-Year Rainfall=7.00" **POST Rev-1** Prepared by Goldsmith, Prest & Ringwall, Inc. Printed 7/13/2022 HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC Page 49 Peak Elev=286.61' Inflow=1.2 cfs 4,096 cf Pond CB-10: 12.0" Round Culvert n=0.013 L=5.0' S=0.0400 '/' Outflow=1.2 cfs 4,096 cf Peak Elev=297.08' Inflow=3.0 cfs 11,551 cf Pond CB-2: 12.0" Round Culvert n=0.013 L=7.0' S=0.0200 '/' Outflow=3.0 cfs 11,551 cf Peak Elev=289.45' Inflow=2.8 cfs 11,836 cf Pond CB-3: 12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/' Outflow=2.8 cfs 11,836 cf Pond CB-5: Peak Elev=294.96' Inflow=2.0 cfs 7,419 cf 12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/' Outflow=2.0 cfs 7,419 cf Pond CB-6: Peak Elev=296.19' Inflow=1.3 cfs 5,040 cf 12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/' Outflow=1.3 cfs 5,040 cf Peak Elev=289.87' Inflow=0.7 cfs 2,497 cf Pond CB-7: 12.0" Round Culvert n=0.013 L=22.0' S=0.0136 '/' Outflow=0.7 cfs 2,497 cf Pond CB-8: Peak Elev=289.85' Inflow=0.7 cfs 2,688 cf 12.0" Round Culvert n=0.013 L=15.0' S=0.0200 '/' Outflow=0.7 cfs 2,688 cf Peak Elev=287.15' Inflow=1.2 cfs 3,984 cf Pond CB-9: 12.0" Round Culvert n=0.013 L=40.0' S=0.0200 '/' Outflow=1.2 cfs 3,984 cf Pond DCB-4: Peak Elev=286.90' Inflow=3.6 cfs 13,692 cf 12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/' Outflow=3.6 cfs 13,692 cf Peak Elev=296.08' Inflow=4.3 cfs 16,264 cf Pond DMH-1: 12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/' Outflow=4.3 cfs 16,264 cf Pond DMH-2: Peak Elev=288.69' Inflow=6.8 cfs 28,101 cf 18.0" Round Culvert n=0.013 L=48.0' S=0.0100 '/' Outflow=6.8 cfs 28,101 cf Peak Elev=286.49' Inflow=3.6 cfs 13,692 cf Pond DMH-3: 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=3.6 cfs 13,684 cf Peak Elev=290.72' Inflow=3.3 cfs 12,459 cf Pond DMH-5: 12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/' Outflow=3.3 cfs 12,459 cf Peak Elev=289.87' Inflow=4.0 cfs 14,956 cf Pond DMH-6: 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=4.0 cfs 14,956 cf Pond DMH-7: Peak Elev=289.85' Inflow=0.7 cfs 2,688 cf 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=0.7 cfs 2,680 cf Peak Elev=286.34' Inflow=2.4 cfs 8,080 cf Pond DMH-9: 15.0" Round Culvert n=0.013 L=197.0' S=0.0175 '/' Outflow=2.4 cfs 8,080 cf Peak Elev=285.90' Storage=1,300 cf Inflow=7.9 cfs 31,762 cf Pond IB-1:

Discarded=0.0 cfs 946 cf Primary=2.1 cfs 24,858 cf Secondary=5.6 cfs 5,699 cf Outflow=7.6 cfs 31,502 cf
POST Rev-1 Prepared by Goldsmith, F <u>HydroCAD® 10.10-4a</u> s/n 010	Prest & Ringwall, Inc. 136 © 2020 HydroCAD Softwa	NRCC 24	203 Ayer Road, H 4-hr D 50-Year Rain Printed	arvard, MA nfall=7.00″ 7/13/2022 Page 50
Pond IB-2:	Peak E	lev=283.53' Storage	e=1,860 cf Inflow=2.8	cfs 9,517 cf
	Primary=1.6 cfs_8,876	of Secondary=0.0	cfs 0 cf Outflow=1.6	cfs 8,876 cf
Pond IC-1:	Peak Ele	ev=286.49' Storage=	=8,594 cf Inflow=5.2 c	fs 20,221 cf
	Discarded=0.1 cfs 7,929 cf	f Primary=0.8 cfs 4	,784 cf Outflow=0.9 c	fs 12,713 cf
Pond IC-2:	Peak Ele	ev=289.85' Storage=	=8,264 cf Inflow=6.8 c	fs 26,078 cf
	Discarded=0.1 cfs 7,884 cf	Primary=3.1 cfs 13	,596 cf Outflow=3.2 c	fs 21,480 cf
Link AP-1:			Inflow=15.8 c Primary=15.8 c	fs 72,751 cf fs 72,751 cf

Total Runoff Area = 270,181 sf Runoff Volume = 102,532 cf Average Runoff Depth = 4.55" 55.08% Pervious = 148,810 sf 44.92% Impervious = 121,371 sf

Summary for Subcatchment SC1.1:

Runoff = 1.4 cfs @ 12.13 hrs, Volume= 4,714 cf, Depth> 3.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN	Description							
	4,381	61	61 >75% Grass cover, Good, HSG B							
	5,583	55	Woods, Good, HSG B							
	328	98	Unconnecte	ed pavemer	nt, HSG B					
	3,654	98	Paved park	ing, HSG B						
	1,269	96	Gravel surfa	ace, HSG E	3					
	15,215	71	Weighted A	verage						
	11,233		73.83% Pei	vious Area						
	3,982		26.17% Imp	pervious Ar	ea					
	328		8.23% Unc	onnected						
_				_						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
3.4	34	0.2400	0.17		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.12"					
0.9	16	0.1875	0.28		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.12"					
1.0	83	0.0361	1.33		Shallow Concentrated Flow,					
o -	400		0.50		Short Grass Pasture Kv= 7.0 fps					
0.5	100	0.0300	3.52		Shallow Concentrated Flow,					
					Paved Kv= 20.3 tps					
5.8	233	Total,	Increased t	o minimum	Tc = 6.0 min					

Summary for Subcatchment SC1.2:

Runoff = 3.0 cfs @ 12.16 hrs, Volume= 11,551 cf, Depth> 5.24"

Are	ea (sf)	CN	Description				
	6,620	61	>75% Grass cover, Good, HSG B				
	2,398	55	Woods, Good, HSG B				
	1,172	98	Inconnected pavement, HSG B				
	8,000	98	coofs, HSG B				
	7,203	98	vaved parking, HSG B				
	1,052	96	Gravel surface, HSG B				
2	6,444	85	Weighted Average				
1	0,069		38.08% Pervious Area				
1	6,375		61.92% Impervious Area				
	1,172		7.16% Unconnected				

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.9	41	0.2400	0.17		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
1.4	9	0.0200	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
3.4	222	0.0248	1.10		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	63	0.0300	3.52		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
9.0	335	Total			

Summary for Subcatchment SC1.3:

Runoff	=	2 8 cfs @	12 21 hrs	Volume=	11 836 cf	Denth>	4 24"
RUHUH	-	2.0 US @	12.211115,	volume-	11,000 01,	Depui/	4.24

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN [Description								
	15,029	61 >	51 >75% Grass cover, Good, HSG B								
	3,894	55 \	Noods, Go	loods, Good, HSG B							
	1,741	96 (Gravel surfa	ace, HSG E	3						
	275	98 l	Jnconnecte	ed pavemer	nt, HSG B						
	12,548	98 F	Paved park	ing, HSG B							
	33,487	76 \	Neighted A	verage							
	20,664	6	61.71% Pei	vious Area							
	12,823	3	38.29% Imp	pervious Are	ea						
	275	2	2.14% Unco	onnected							
Тс	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
7.2	50	0.0800	0.12		Sheet Flow,						
					Woods: Light underbrush n= 0.400 P2= 3.12"						
3.2	188	0.0372	0.96		Shallow Concentrated Flow,						
					Woodland Kv= 5.0 fps						
2.0	160	0.0372	1.35		Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
0.6	122	0.0300	3.52		Shallow Concentrated Flow,						
					Paved Kv= 20.3 tps						
13.0	520	Total									

Summary for Subcatchment SC2.1:

Runoff = 1.3 cfs @ 12.13 hrs, Volume= 5,040 cf, Depth> 6.52"

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A	rea (sf)	CN	Description	Description							
	595	61	>75% Gras	s cover, Go	ood, HSG B						
	423	98	Unconnecte	ed pavemer	ent, HSG B						
	4,000	98	Roofs, HSG	B							
	4,263	98	Paved park	ing, HSG B	В						
	9,281	96	Weighted A	verage							
	595		6.41% Perv	ious Area							
	8,686		93.59% Imp	pervious Are	rea						
	423		4.87% Unco	onnected							
Tc	Length	Slope	e Velocity	Capacity	Description						
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)							
5.0					Direct Entry,						
5.0	0	Total,	Increased t	o minimum	n Tc = 6.0 min						

Summary for Subcatchment SC2.2:

2.0 cfs @ 12.15 hrs, Volume= 7,419 cf, Depth> 4.80" Runoff =

A	rea (sf)	CN I	Description							
	5,529	61 >	61 >75% Grass cover, Good, HSG B							
	2,637	55 \	Noods, Go	od, HSG B						
	889	98 l	Jnconnecte	ed pavemer	nt, HSG B					
	4,000	98 I	Roofs, HSG	βB						
	4,580	98 I	Paved park	ing, HSG B	5					
	929	96 (Gravel surfa	ace, HSG E	3					
	18,563	81 \	Neighted A	verage						
	9,094	4	18.99% Pei	vious Area						
	9,469	Ę	51.01% Imp	pervious Ar	ea					
	889	ę	9.39% Unc	onnected						
Тс	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
4.6	50	0.2400	0.18		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.12"					
0.2	8	0.0200	0.71		Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps					
2.5	151	0.0200	0.99		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
0.5	86	0.0200	2.87		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
7.8	295	Total								

Summary for Subcatchment SC2.3:

Runoff = 1.2 cfs @ 12.13 hrs, Volume= 3,984 cf, Depth> 4.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

Α	rea (sf)	CN [Description							
	7,218	61 >	61 >75% Grass cover, Good, HSG B							
	293	98 l	Unconnected pavement, HSG B							
	4,029	98 F	Paved parking, HSG B							
	11,540	75 \	Veighted A	verage						
	7,218	6	62.55% Per	vious Area						
	4,321	3	37.45% Imp	pervious Ar	ea					
	293	6	6.77% Unco	onnected						
_										
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
2.3	42	0.1430	0.31		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.12"					
0.1	8	0.0500	1.18		Sheet Flow,					
					Smooth surfaces n= 0.011 P2= 3.12"					
0.5	135	0.0500	4.54		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
2.9	185	Total,	Increased t	o minimum	ı Tc = 6.0 min					

Summary for Subcatchment SC3.1:

Runoff = 1.6 cfs @ 12.13 hrs, Volume= 6,537 cf, Depth> 6.75"

Ar	rea (sf)	CN I	Description							
	11,615	98	Roofs, HSG B							
	0	61 :	>75% Gras	s cover, Go	bod, HSG B					
	11,615	98	98 Weighted Average							
	0	(0.00% Pervious Area							
	11,615		100.00% Impervious Area							
Тс	Length	Slope	Velocity	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
5.0			Direct Entry,							
5.0	0	Total,	Total, Increased to minimum Tc = 6.0 min							

Summary for Subcatchment SC3.2:

Runoff = 3.6 cfs @ 12.13 hrs, Volume= 13,692 cf, Depth> 6.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN [Description							
	2,372	61 >	61 >75% Grass cover, Good, HSG B							
	21,883	98 F	Paved park	ing, HSG B						
	1,422	98 l	Inconnecte	ed pavemer	nt, HSG B					
	25,677	95 V	Veighted A	verage						
	2,372	ç).24% Perv	ious Area						
	23,305	ç	0.76% Imp	pervious Are	ea					
	1,422	6	6.10% Unco	onnected						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
4.4	50	0.0380	0.19		Sheet Flow,					
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps					
5.2	118	Total, I	ncreased t	o minimum	Tc = 6.0 min					

Summary for Subcatchment SC3.3:

Runoff = 1.1 cfs @ 12.13 hrs, Volume= 3,662 cf, Depth> 3.00"

Α	rea (sf)	CN [Description		
	13,540	61 >	>75% Gras	s cover, Go	ood, HSG B
	1,127	96 (Gravel surfa	ace, HSG E	3
	14,666	64 \	Veighted A	verage	
	14,666		100.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.7	50	0.0600	0.23		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
1.9	166	0.0422	1.44		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
5.6	216	Total,	ncreased t	o minimum	Tc = 6.0 min

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<u></u>			Sum	mary for	Subcatchmen	nt SC4.1:	<u> </u>
Runoff	=	2.1 ct	fs @ 12.1	3 hrs, Volu	ume= 8	,442 cf, Depth>	≥ 6.75"
Runoff b NRCC 24	y SCS TF 4-hr D 50	R-20 meth)-Year Ra	nod, UH=S ainfall=7.00	CS, Weigh)"	ted-CN, Time Sp	an= 0.00-24.00	hrs, dt= 0.05 hrs
A	rea (sf)	CN D	escription				
	14,998	98 R	loofs, HSG	В			
	14,998	1	00.00% Im	pervious A	rea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		
5.0	0	Total, li	ncreased t	o minimum	Tc = 6.0 min		
			Sum	mary for	Subcatchmen	ot SC4.2:	
Runoff	=	1.2 ct	fs @ 12.1	3 hrs, Volu	ume= 4	,096 cf, Depth>	▶ 4.69"
Runoff b NRCC 24	y SCS TF 4-hr D 50	R-20 meth)-Year Ra	nod, UH=S ainfall=7.00	CS, Weigh)"	ted-CN, Time Sp	an= 0.00-24.00	hrs, dt= 0.05 hrs
A	rea (sf)	CN D	escription				
	5,060	61 >	75% Grass	s cover, Go	od, HSG B		
	1,450	98 U	Inconnecte	ed pavemer	nt, HSG B		
	<u> </u>	<u>90 F</u> 80 W	Aveu park	verade			
	5.060	4	8.26% Per	vious Area			
	5,424	5	1.74% Imp	ervious Are	ea		
	1,450	2	6.72% Uno	connected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
4.0	50	0.0500	0.21		Sheet Flow,		
0.4	00	0.0500	A 67		Grass: Short n	= 0.150 P2= 3	8.12"
0.4	33	0.0500	1.57		Short Grass Pa	sture Ky= 7 0	fns
0.6	148	0.0440	4.26		Shallow Conce Paved Kv= 20	entrated Flow, .3 fps	,F.2

5.0 231 Total, Increased to minimum Tc = 6.0 min

Summary for Subcatchment SC4.3:

Runoff = 0.7 cfs @ 12.13 hrs, Volume= 2,497 cf, Depth> 6.40"

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A	rea (sf)	CN	Description
	406	61	>75% Grass cover, Good, HSG B
	175	98	Unconnected pavement, HSG B
	4,101	98	Paved parking, HSG B
Tc	4,682 406 4,276 175 Length	95 Slop	Weighted Average 8.68% Pervious Area 91.32% Impervious Area 4.09% Unconnected be Velocity Capacity Description
<u> (IIIII)</u>		(101	
5.0			Direct Entry,
5.0	0	Total,	, Increased to minimum Tc = 6.0 min

Summary for Subcatchment SC4.4:

Runoff = 0.7 cfs @ 12.13 hrs, Volume= 2,688 cf, Depth> 6.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-Year Rainfall=7.00"

A	rea (sf)	CN	Description	
	564	61	>75% Grass cover, Good, HSG B	
	541	98	Unconnected pavement, HSG B	
	4,030	98	Paved parking, HSG B	
	5,136	94	Weighted Average	
	564		10.99% Pervious Area	
	4,571		89.01% Impervious Area	
	541		11.83% Unconnected	
Tc (min)	Length (feet)	Slop (ft/f	e Velocity Capacity Description t) (ft/sec) (cfs)	
5.0			Direct Entry,	
5.0	0	Total,	Increased to minimum Tc = 6.0 min	

Summary for Subcatchment SC4.5:

Runoff = 0.4 cfs @ 12.13 hrs, Volume= 1,436 cf, Depth> 2.70"

Area (sf)	CN	Description
6,391	61	>75% Grass cover, Good, HSG B
6,391		100.00% Pervious Area

POST I Prepare	Rev-1 d by Gol	dsmith, I	Prest & R	ingwall, In	с.	203 NRCC 24-hr D	Ayer Road, Harvard, MA 50-Year Rainfall=7.00" Printed 7/13/2022
HydroCA	D® 10.10-	4a_s/n 01	036 © 202	0 HydroCAE) Software Sol	utions LLC	Page 58
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entr	y,	
5.0	0	Total, li	ncreased t	o minimum	Tc = 6.0 min		
			Sum	mary for	Subcatchn	nent SC5.1:	
Runoff	=	3.8 cf	fs @ 12.1	8 hrs, Volu	ume=	14,937 cf, Depth>	2.89"
Runoff b NRCC 24	y SCS TF 4-hr D 50	R-20 meth)-Year Ra	nod, UH=S ainfall=7.00	CS, Weigh)"	ted-CN, Time	e Span= 0.00-24.00	hrs, dt= 0.05 hrs
А	rea (sf)	CN D	escription				
	52,172 5,271 70 1,456 3,033	61 > 55 W 98 U 98 P 96 G	75% Grass Voods, Go Inconnecte aved park Gravel surfa	s cover, Gc od, HSG B ed pavemer ing, HSG B ace, HSG B	ood, HSG B nt, HSG B		
	62,001 60,476 1,525 70	63 V 9 2 4	Veighted A 7.54% Per .46% Impe .56% Unco	verage vious Area ervious Area onnected	a		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
3.8	36	0.2000	0.16		Sheet Flow	',	
2.1	14	0.0200	0.11		Woods: Ligi Sheet Flow	nt underbrush n= 0 /, 	.400 P2= 3.12"
4.1	312	0.0334	1.28		Shallow Co Short Grass	ncentrated Flow, Pasture Ky= 7.0 f	ns
10.0	362	Total					
				Summar	y for Pond	CB-1:	
Inflow Ar Inflow Outflow Primary	rea = = = =	15,2 ⁻ 1.4 ct 1.4 ct 1.4 ct	15 sf, 26.′ fs @ 12.1 fs @ 12.1 fs @ 12.1 fs @ 12.1	17% Imperv 3 hrs, Volu 3 hrs, Volu 3 hrs, Volu	vious, Inflow ume= ume= ume=	Depth > 3.72" fo 4,714 cf 4,714 cf, Atten= 4,714 cf	r 50-Year event 0%, Lag= 0.0 min
Routing Peak Ele Flood Ele	by Dyn-Si ev= 298.23 ev= 301.5	tor-Ind m 3' @ 12.1 50'	ethod, Tim I3 hrs	ne Span= 0.	.00-24.00 hrs	, dt= 0.05 hrs / 2	
Device	Routina		Invert O	utlet Devic	es		
#1	Primary	2	97.50' 1 ; L: Ir n:	2.0'' Roun = 170.0' C hlet / Outlet = 0.013 Cc	d Culvert CPP, projectin Invert= 297.5 prrugated PE,	g, no headwall, Ke 50' / 293.56' S= 0.0 smooth interior, Fl	= 0.900 232 '/' Cc= 0.900 ow Area= 0.79 sf

Primary OutFlow Max=1.3 cfs @ 12.13 hrs HW=298.21' TW=295.87' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.3 cfs @ 2.26 fps)

Summary for Pond CB-10:

Inflow Area	a =	10,484 sf,	51.74% Impervious,	Inflow Depth >	4.69" fo	or 50-Year event
Inflow	=	1.2 cfs @	12.13 hrs, Volume=	4,096 ct	f	
Outflow	=	1.2 cfs @	12.13 hrs, Volume=	4,096 ct	f, Atten=	0%, Lag= 0.0 min
Primary	=	1.2 cfs @	12.13 hrs, Volume=	4,096 ct	f	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.61' @ 12.13 hrs Flood Elev= 289.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.90'	12.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.90' / 285.70' S= 0.0400 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.1 cfs @ 12.13 hrs HW=286.59' TW=286.32' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.1 cfs @ 1.98 fps)

Summary for Pond CB-2:

Inflow Area	a =	26,444 sf,	61.92% Impervious,	Inflow Depth > 5	.24" for 50-Year event
Inflow	=	3.0 cfs @	12.16 hrs, Volume=	11,551 cf	
Outflow	=	3.0 cfs @	12.16 hrs, Volume=	11,551 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	3.0 cfs @	12.16 hrs, Volume=	11,551 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 297.08' @ 12.15 hrs Flood Elev= 297.70'

Device	Routing	Invert	Outlet Devices				
#1	Primary	293.70'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf				

Primary OutFlow Max=2.9 cfs @ 12.16 hrs HW=296.94' TW=295.97' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.9 cfs @ 3.74 fps)

Summary for Pond CB-3:

Inflow Are	ea =	33,487 sf,	38.29% Impervious,	Inflow Depth > 4.2	24" for 50-Year event
Inflow	=	2.8 cfs @	12.21 hrs, Volume=	11,836 cf	
Outflow	=	2.8 cfs @	12.21 hrs, Volume=	11,836 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	2.8 cfs @	12.21 hrs, Volume=	11,836 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.45' @ 12.18 hrs Flood Elev= 291.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.50'	12.0" Round Culvert L= 8.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.7 cfs @ 12.21 hrs HW=289.36' TW=288.51' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.7 cfs @ 3.49 fps)

Summary for Pond CB-5:

Inflow Are	a =	18,563 sf,	51.01% Impervious,	Inflow Depth > 4	.80" for 50-Year event
Inflow	=	2.0 cfs @	12.15 hrs, Volume=	7,419 cf	
Outflow	=	2.0 cfs @	12.15 hrs, Volume=	7,419 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	2.0 cfs @	12.15 hrs, Volume=	7,419 cf	_

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 294.96' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.0 cfs @ 12.15 hrs HW=294.96' TW=290.71' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.0 cfs @ 2.63 fps)

Summary for Pond CB-6:

Inflow Area	a =	9,281 sf,	93.59% Impervious,	Inflow Depth > 6	6.52" for 50	-Year event
Inflow	=	1.3 cfs @	12.13 hrs, Volume=	5,040 cf	:	
Outflow	=	1.3 cfs @	12.13 hrs, Volume=	5,040 cf	, Atten= 0%,	Lag= 0.0 min
Primary	=	1.3 cfs @	12.13 hrs, Volume=	5,040 cf	:	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

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Peak Elev= 296.19' @ 12.13 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	12.0" Round Culvert L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.2 cfs @ 12.13 hrs HW=296.17' TW=290.61' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.2 cfs @ 2.21 fps)

Summary for Pond CB-7:

Inflow Area	a =	4,682 sf,	91.32% Impervious,	Inflow Depth >	6.40"	for 50-Year event
Inflow	=	0.7 cfs @	12.13 hrs, Volume=	2,497 c	of	
Outflow	=	0.7 cfs @	12.13 hrs, Volume=	2,497 c	of, Atter	n= 0%, Lag= 0.0 min
Primary	=	0.7 cfs @	12.13 hrs, Volume=	2,497 c	of	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.87' @ 12.31 hrs Flood Elev= 292.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.00'	12.0" Round Culvert L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.0 cfs @ 12.13 hrs HW=289.35' TW=289.53' (Dynamic Tailwater) 1=Culvert (Controls 0.0 cfs)

Summary for Pond CB-8:

Inflow Area	ı =	5,136 sf,	89.01% Impervious,	Inflow Depth > 6	6.28" for	50-Year event
Inflow	=	0.7 cfs @	12.13 hrs, Volume=	2,688 cf	f	
Outflow	=	0.7 cfs @	12.13 hrs, Volume=	2,688 cf	f, Atten= 0%	%, Lag= 0.0 min
Primary	=	0.7 cfs @	12.13 hrs, Volume=	2,688 cf	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.85' @ 12.31 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.50'	12.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.0 cfs @ 12.13 hrs HW=289.27' TW=289.43' (Dynamic Tailwater) ↓ 1=Culvert (Controls 0.0 cfs)

Summary for Pond CB-9:

Inflow Area	a =	11,540 sf,	37.45% Impervious,	Inflow Depth > 4.14	for 50-Year event
Inflow	=	1.2 cfs @	12.13 hrs, Volume=	3,984 cf	
Outflow	=	1.2 cfs @	12.13 hrs, Volume=	3,984 cf, At	ten= 0%, Lag= 0.0 min
Primary	=	1.2 cfs @	12.13 hrs, Volume=	3,984 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.15' @ 12.13 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.50'	12.0" Round Culvert
			L= 40.0° CPP, projecting, no neadwall, Ke= 0.900
			Inlet / Outlet Invert= 286.50' / 285.70' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.1 cfs @ 12.13 hrs HW=287.13' TW=286.32' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.1 cfs @ 2.14 fps)

Summary for Pond DCB-4:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth >	6.40"	for 50-Year event
Inflow	=	3.6 cfs @	12.13 hrs, Volume=	13,692 c	f	
Outflow	=	3.6 cfs @	12.13 hrs, Volume=	13,692 c	f, Atter	n= 0%, Lag= 0.0 min
Primary	=	3.6 cfs @	12.13 hrs, Volume=	13,692 c	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.90' @ 12.14 hrs Flood Elev= 288.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.50'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.1 cfs @ 12.13 hrs HW=286.78' TW=285.69' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.1 cfs @ 3.97 fps)

Summary for Pond DMH-1:

Inflow Are	ea =	41,659 sf,	48.86% Impervious,	Inflow Depth > 4.68" for 50-Year event	
Inflow	=	4.3 cfs @	12.15 hrs, Volume=	16,264 cf	
Outflow	=	4.3 cfs @	12.15 hrs, Volume=	16,264 cf, Atten= 0%, Lag= 0.0 mir	۱
Primary	=	4.3 cfs @	12.15 hrs, Volume=	16,264 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 296.08' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	12.0" Round Culvert L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=4.3 cfs @ 12.15 hrs HW=296.06' TW=288.66' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 4.3 cfs @ 5.51 fps)

Summary for Pond DMH-2:

Inflow Are	ea =	75,146 sf,	44.15% Impervious,	Inflow Depth >	4.49" fc	or 50-Year event
Inflow	=	6.8 cfs @	12.16 hrs, Volume=	28,101 cl	f	
Outflow	=	6.8 cfs @	12.16 hrs, Volume=	28,101 ct	f, Atten=	0%, Lag= 0.0 min
Primary	=	6.8 cfs @	12.16 hrs, Volume=	28,101 ct	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.69' @ 12.16 hrs Flood Elev= 291.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.90'	18.0" Round Culvert L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=6.7 cfs @ 12.16 hrs HW=288.64' TW=285.90' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 6.7 cfs @ 3.79 fps)

Summary for Pond DMH-3:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth >	6.40"	for 50-Year event
Inflow	=	3.6 cfs @	12.13 hrs, Volume=	13,692 c	f	
Outflow	=	3.6 cfs @	12.13 hrs, Volume=	13,684 c	f, Atten	= 0%, Lag= 0.0 min
Primary	=	3.6 cfs @	12.13 hrs, Volume=	13,684 c	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

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Peak Elev= 286.49' @ 12.55 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.10'	24.0" Round Culvert L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=3.5 cfs @ 12.13 hrs HW=285.69' TW=285.58' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 3.5 cfs @ 1.29 fps)

Summary for Pond DMH-5:

Inflow Area	a =	27,844 sf,	65.20% Impervious,	Inflow Depth >	5.37"	for 50-Year event
Inflow	=	3.3 cfs @	12.14 hrs, Volume=	12,459 c	f	
Outflow	=	3.3 cfs @	12.14 hrs, Volume=	12,459 c	f, Atten	= 0%, Lag= 0.0 min
Primary	=	3.3 cfs @	12.14 hrs, Volume=	12,459 c	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.72' @ 12.14 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	289.00'	12.0" Round Culvert L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.0 cfs @ 12.14 hrs HW=290.67' TW=289.60' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 3.0 cfs @ 3.84 fps)

Summary for Pond DMH-6:

Inflow Area	=	32,526 sf,	68.96% Impervious,	Inflow Depth > 4	5.52" for 50-Ye	ear event
Inflow	=	4.0 cfs @	12.14 hrs, Volume=	14,956 cf		
Outflow	=	4.0 cfs @	12.14 hrs, Volume=	14,956 cf	, Atten= 0%, La	ig= 0.0 min
Primary	=	4.0 cfs @	12.14 hrs, Volume=	14,956 cf		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.87' @ 12.25 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=3.9 cfs @ 12.14 hrs HW=289.59' TW=289.48' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.9 cfs @ 1.24 fps)

Summary for Pond DMH-7:

Inflow Area	a =	5,136 sf,	89.01% Impervious,	Inflow Depth > 6	.28" for 50-Year event
Inflow	=	0.7 cfs @	12.13 hrs, Volume=	2,688 cf	
Outflow	=	0.7 cfs @	12.13 hrs, Volume=	2,680 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	0.7 cfs @	12.13 hrs, Volume=	2,680 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.85' @ 12.26 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.7 cfs @ 12.13 hrs HW=289.43' TW=289.43' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.7 cfs @ 0.23 fps)

Summary for Pond DMH-9:

Inflow Area	a =	22,024 sf,	44.25% Impervious,	Inflow Depth > 4	.40" for 50-Year event
Inflow	=	2.4 cfs @	12.13 hrs, Volume=	8,080 cf	
Outflow	=	2.4 cfs @	12.13 hrs, Volume=	8,080 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	2.4 cfs @	12.13 hrs, Volume=	8,080 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.34' @ 12.13 hrs Flood Elev= 290.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.45'	15.0" Round Culvert L= 197.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 285.45' / 282.00' S= 0.0175 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
.			

Primary OutFlow Max=2.3 cfs @ 12.13 hrs HW=286.32' TW=283.37' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.3 cfs @ 2.50 fps) 203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D50-Year Rainfall=7.00"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 66

Summary for Pond IB-1:

Inflow Area =	89,812 sf,	36.94% Impervious,	Inflow Depth > 4.24"	for 50-Year event
Inflow =	7.9 cfs @	12.16 hrs, Volume=	31,762 cf	
Outflow =	7.6 cfs @	12.18 hrs, Volume=	31,502 cf, Atte	en= 3%, Lag= 1.2 min
Discarded =	0.0 cfs @	12.18 hrs, Volume=	946 cf	
Primary =	2.1 cfs @	12.18 hrs, Volume=	24,858 cf	
Secondary =	5.6 cfs @	12.18 hrs, Volume=	5,699 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 285.90' @ 12.18 hrs Surf.Area= 1,340 sf Storage= 1,300 cf

Plug-Flow detention time= 11.5 min calculated for 31,437 cf (99% of inflow) Center-of-Mass det. time= 6.7 min (842.0 - 835.4)

Volume	Invert	Avail.	Storage	Storage Description	n			
#1	284.00'	3	3,366 cf	Custom Stage Dat	ta (Irregular) Liste	d below (Recalc)		
Elevatio	on Su	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
(166	:()	(Sq-II)	(leet)	(Cubic-leet)		(sq-it)		
284.0	00	208	72.5	0	0	208		
285.0	00	661	129.3	413	413	1,126		
286.0	00	1,425	189.1	1,019	1,432	2,649		
287.0	00	2,492	246.3	1,934	3,366	4,643		
Device	Routing	Inve	ert Outle	et Devices				
#1	Discarded	284.0	0' 1.02	0 in/hr Exfiltration	over Surface area	a		
#2	Secondarv	285.5	0' 8.0'	8.0' long x 15.0' breadth Broad-Crested Rectangular Weir				
	J		Head	d (feet) 0.20 0.40 (0.60 0.80 1.00 1	20 1.40 1.60		
			Coef	(English) 268 27	70 2 70 2 64 2 6	3 2 64 2 64 2 63		
#3	Primary	284.5	0' 10.0 '	" Round Culvert	a no headwall K	e= 0.900		
			Inlet n= 0	/ Outlet Invert= 284. .013, Flow Area= 0.	.50' / 282.50' S= .55 sf	0.0667 '/' Cc= 0.900		

Discarded OutFlow Max=0.0 cfs @ 12.18 hrs HW=285.90' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=2.1 cfs @ 12.18 hrs HW=285.90' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 2.1 cfs @ 3.76 fps)

Secondary OutFlow Max=5.4 cfs @ 12.18 hrs HW=285.90' TW=0.00' (Dynamic Tailwater) —2=Broad-Crested Rectangular Weir (Weir Controls 5.4 cfs @ 1.70 fps)

Summary for Pond IB-2:

Inflow Area =	28,415 sf, 34.30% Impervious	, Inflow Depth > 4.0	2" for 50-Year event
Inflow =	2.8 cfs @ 12.13 hrs, Volume=	= 9,517 cf	
Outflow =	1.6 cfs @ 12.22 hrs, Volume=	= 8,876 cf, A	Atten= 42%, Lag= 5.5 min
Primary =	1.6 cfs @ 12.22 hrs, Volume=	= 8,876 cf	
Secondary =	0.0 cfs $\overline{@}$ 0.00 hrs, Volume=	= 0 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 283.53' @ 12.22 hrs Surf.Area= 1,589 sf Storage= 1,860 cf

Plug-Flow detention time= 67.8 min calculated for 8,876 cf (93% of inflow) Center-of-Mass det. time= 31.2 min (870.7 - 839.5)

#1	282.00'	4,792 cf	Custom Stage Da	ta (Irregular) Listed	l below (Recald
Elevation (feet)	Surf.Are (sq-	ea Perim. ft) (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
282.00	8.	70 143.0	0	0	870
283.00	1,3	28 162.0	1,091	1,091	1,356
284.00	1,84	43 181.0	1,578	2,669	1,902
285.00	2,4	14 200.0	2,122	4,792	2,509

Device	Rouling	Inven	Outlet Devices
#1	Secondary	284.00'	10.0' long x 22.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	282.50'	10.0" Round Culvert L= 37.4' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 282.50' / 278.50' S= 0.1070 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=1.6 cfs @ 12.22 hrs HW=283.52' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Inlet Controls 1.6 cfs @ 2.94 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=282.00' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

Summary for Pond IC-1:

Inflow Area	1 =	37,292 sf,	93.64% Im	pervious,	Inflow Depth >	6.	51" fo	r 50-1	∕ear e∖	/ent
Inflow	=	5.2 cfs @	12.13 hrs,	Volume=	20,221	cf				
Outflow	=	0.9 cfs @	12.55 hrs,	Volume=	12,713	Scf,	Atten=	83%,	Lag= 2	25.5 min
Discarded	=	0.1 cfs @	7.25 hrs,	Volume=	7,929) cf			-	
Primary	=	0.8 cfs @	12.55 hrs,	Volume=	4,784	cf				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.49' @ 12.55 hrs Surf.Area= 4,404 sf Storage= 8,594 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 62.6 min (819.9 - 757.3)

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Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
		9,501 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
	-		L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 7.25 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=0.8 cfs @ 12.55 hrs HW=286.48' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 0.8 cfs of 1.6 cfs potential flow) **3=Orifice/Grate** (Orifice Controls 0.8 cfs @ 2.37 fps)

Summary for Pond IC-2:

Inflow Area	a =	52,660 sf,	79.76% Impervious,	Inflow Depth >	5.94"	for 50-Y	'ear event
Inflow	=	6.8 cfs @	12.13 hrs, Volume=	26,078 0	of		
Outflow	=	3.2 cfs @	12.26 hrs, Volume=	21,480 0	cf, Atter	า= 53%,	Lag= 7.7 min
Discarded	=	0.1 cfs @	6.95 hrs, Volume=	7,884 0	cf		-
Primary	=	3.1 cfs @	12.26 hrs, Volume=	13,596 (of		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.85' @ 12.26 hrs Surf.Area= 4,361 sf Storage= 8,264 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 44.2 min (817.8 - 773.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows

9,414 cf Total Available Storage

Storage Group A created with Chamber Wizard

203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D50-Year Rainfall=7.00"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 69

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
	-		L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 6.95 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=3.0 cfs @ 12.26 hrs HW=289.84' TW=0.00' (Dynamic Tailwater) -2=Culvert (Inlet Controls 3.0 cfs @ 3.87 fps) -3=Orifice/Grate (Passes 3.0 cfs of 3.4 cfs potential flow)

Summary for Link AP-1:

Inflow A	Area	=	270,181 sf,	44.92% In	npervious,	Inflow Depth >	3.2	23" for 50	-Year event
Inflow		=	15.8 cfs @	12.19 hrs,	Volume=	72,751	cf		
Primar	у	=	15.8 cfs @	12.19 hrs,	Volume=	72,751	cf, A	Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

	203	3 Ayer Road, Ha	arvard, MA
POST Rev-1	NRCC 24-hr D	100-Year Rain	nfall=8.34"
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 2 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentSC1.1:	Runoff Area=15,215 sf 26.17% Impervious Runoff Depth>4.87" Flow Length=233' Tc=6.0 min CN=71 Runoff=1.8 cfs 6,173 cf
Subcatchment SC1.2:	Runoff Area=26,444 sf 61.92% Impervious Runoff Depth>6.53" Flow Length=335' Tc=9.0 min CN=85 Runoff=3.7 cfs 14,386 cf
Subcatchment SC1.3:	Runoff Area=33,487 sf 38.29% Impervious Runoff Depth>5.45" Flow Length=520' Tc=13.0 min CN=76 Runoff=3.5 cfs 15,206 cf
Subcatchment SC2.1:	Runoff Area=9,281 sf 93.59% Impervious Runoff Depth>7.85" Tc=6.0 min CN=96 Runoff=1.6 cfs 6,073 cf
Subcatchment SC2.2:	Runoff Area=18,563 sf 51.01% Impervious Runoff Depth>6.05" Flow Length=295' Tc=7.8 min CN=81 Runoff=2.6 cfs 9,363 cf
Subcatchment SC2.3:	Runoff Area=11,540 sf 37.45% Impervious Runoff Depth>5.34" Flow Length=185' Tc=6.0 min CN=75 Runoff=1.5 cfs 5,137 cf
Subcatchment SC3.1:	Runoff Area=11,615 sf 100.00% Impervious Runoff Depth>8.09" Tc=6.0 min CN=98 Runoff=2.0 cfs 7,832 cf
Subcatchment SC3.2:	Runoff Area=25,677 sf 90.76% Impervious Runoff Depth>7.73" Flow Length=118' Slope=0.0380 '/' Tc=6.0 min CN=95 Runoff=4.3 cfs 16,544 cf
Subcatchment SC3.3:	Runoff Area=14,666 sf 0.00% Impervious Runoff Depth>4.05" Flow Length=216' Tc=6.0 min CN=64 Runoff=1.5 cfs 4,947 cf
SubcatchmentSC4.1:	Runoff Area=14,998 sf 100.00% Impervious Runoff Depth>8.09" Tc=6.0 min CN=98 Runoff=2.5 cfs 10,114 cf
Subcatchment SC4.2:	Runoff Area=10,484 sf 51.74% Impervious Runoff Depth>5.94" Flow Length=231' Tc=6.0 min CN=80 Runoff=1.5 cfs 5,187 cf
Subcatchment SC4.3:	Runoff Area=4,682 sf 91.32% Impervious Runoff Depth>7.73" Tc=6.0 min CN=95 Runoff=0.8 cfs 3,017 cf
Subcatchment SC4.4:	Runoff Area=5,136 sf 89.01% Impervious Runoff Depth>7.61" Tc=6.0 min CN=94 Runoff=0.9 cfs 3,258 cf
Subcatchment SC4.5:	Runoff Area=6,391 sf 0.00% Impervious Runoff Depth>3.70" Tc=6.0 min CN=61 Runoff=0.6 cfs 1,970 cf
Subcatchment SC5.1:	Runoff Area=62,001 sf 2.46% Impervious Runoff Depth>3.93" Flow Length=362' Tc=10.0 min CN=63 Runoff=5.2 cfs 20,281 cf
Pond CB-1:	Peak Elev=298.38' Inflow=1.8 cfs 6,173 cf 12.0" Round Culvert n=0.013 L=170.0' S=0.0232 '/' Outflow=1.8 cfs 6,173 cf

	203 Ayer Road, Harvard NRCC 24-br D_100-Year Bainfall=	d, MA
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Pond CB-10:	,Peak Elev=286.78' Inflow=1.5 cfs 5 12.0" Round Culvert n=0.013 L=5.0' S=0.0400 '/' Outflow=1.5 cfs 5	187 cf 187 cf
Pond CB-2:	,Peak Elev=298.79' Inflow=3.7 cfs 14 12.0" Round Culvert n=0.013 L=7.0' S=0.0200 '/' Outflow=3.7 cfs 14	386 cf 386 cf
Pond CB-3:	,Peak Elev=290.54' Inflow=3.5 cfs 15 12.0" Round Culvert n=0.013 L=8.0' S=0.0125 '/' Outflow=3.5 cfs 15	206 cf 206 cf
Pond CB-5:	,9, Peak Elev=295.23' Inflow=2.6 cfs 12.0" Round Culvert n=0.013 L=86.0' S=0.0570 '/' Outflow=2.6 cfs	363 cf 363 cf
Pond CB-6:	Peak Elev=296.28' Inflow=1.6 cfs 6, 12.0" Round Culvert n=0.013 L=58.0' S=0.1103 '/' Outflow=1.6 cfs 6,	073 cf 073 cf
Pond CB-7:	Peak Elev=290.53' Inflow=0.8 cfs 3, 12.0" Round Culvert n=0.013 L=22.0' S=0.0136 '/' Outflow=0.8 cfs 3,	017 cf 017 cf
Pond CB-8:	,? Peak Elev=290.50' Inflow=0.9 cfs 12.0" Round Culvert n=0.013 L=15.0' S=0.0200 '/' Outflow=0.9 cfs	258 cf 258 cf
Pond CB-9:	Peak Elev=287.26' Inflow=1.5 cfs 5, 12.0" Round Culvert n=0.013 L=40.0' S=0.0200 '/' Outflow=1.5 cfs 5,	137 cf 137 cf
Pond DCB-4:	,Peak Elev=288.05' Inflow=4.3 cfs 16 12.0" Round Culvert n=0.013 L=7.0' S=0.0429 '/' Outflow=4.3 cfs 16	544 cf 544 cf
Pond DMH-1:	,Peak Elev=297.29' Inflow=5.4 cfs 20 با 12.0" Round Culvert n=0.013 L=188.0' S=0.0322 '/' Outflow=5.4 cfs 20	560 cf 560 cf
Pond DMH-2:	Peak Elev=289.30' Inflow=8.6 cfs 35, 18.0" Round Culvert n=0.013 L=48.0' S=0.0100 '/' Outflow=8.6 cfs 35,	765 cf 765 cf
Pond DMH-3:	Peak Elev=287.18' Inflow=4.3 cfs 16, 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=4.3 cfs 16,	544 cf 544 cf
Pond DMH-5:	Peak Elev=291.73' Inflow=4.1 cfs 15,4 12.0" Round Culvert n=0.013 L=88.0' S=0.0148 '/' Outflow=4.1 cfs 15,4	436 cf 436 cf
Pond DMH-6:	،Peak Elev=290.54' Inflow=4.8 cfs 18 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=4.8 cfs 18	453 cf 453 cf
Pond DMH-7:	,? Peak Elev=290.50' Inflow=0.9 cfs 24.0" Round Culvert n=0.013 L=5.0' S=0.0200 '/' Outflow=0.9 cfs	258 cf 256 cf
Pond DMH-9:	,Peak Elev=286.49' Inflow=3.0 cfs 10 ;15.0" Round Culvert n=0.013 L=197.0' S=0.0175 '/' Outflow=3.0 cfs 10	324 cf 324 cf
Pond IB-1:	Peak Elev=286.00' Storage=1,430 cf Inflow=10.0 cfs 40,	712 cf

Discarded=0.0 cfs 1,020 cf Primary=2.2 cfs 30,646 cf Secondary=7.6 cfs 8,770 cf Outflow=9.8 cfs 40,437 cf

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Pond IB-2:	Peak Elev=283.78' Storage=2,273 cf Inflow=3.6 cfs Primary=1.9 cfs 11,636 cf Secondary=0.0 cfs 0 cf Outflow=1.9 cfs	12,294 cf 11,636 cf
Pond IC-1:	Peak Elev=287.17' Storage=9,501 cf Inflow=6.2 cfs Discarded=0.1 cfs 8,217 cf Primary=2.3 cfs 8,512 cf Outflow=2.4 cfs	24,376 cf 16,729 cf
Pond IC-2:	Peak Elev=290.50' Storage=9,407 cf Inflow=8.2 cfs Discarded=0.1 cfs 8,158 cf Primary=3.9 cfs 18,988 cf Outflow=4.0 cfs	31,823 cf 27,146 cf
Link AP-1:	Inflow=22.0 cfs Primary=22.0 cfs	98,834 cf 98,834 cf

Total Runoff Area = 270,181 sf Runoff Volume = 129,488 cf Average Runoff Depth = 5.75" 55.08% Pervious = 148,810 sf 44.92% Impervious = 121,371 sf

Summary for Subcatchment SC1.1:

Runoff = 1.8 cfs @ 12.13 hrs, Volume= 6,173 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

A	rea (sf)	CN	Description							
	4,381	61	61 >75% Grass cover, Good, HSG B							
	5,583	55	55 Woods, Good, HSG B							
	328	98	Unconnecte	ed pavemer	nt, HSG B					
	3,654	98	Paved park	ing, HSG B						
	1,269	96	Gravel surfa	ace, HSG E	3					
	15,215	71	Weighted A	verage						
	11,233		73.83% Pei	vious Area						
	3,982		26.17% Imp	pervious Ar	ea					
	328		8.23% Unc	onnected						
Тс	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
3.4	34	0.2400	0.17		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.12"					
0.9	16	0.1875	0.28		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.12"					
1.0	83	0.0361	1.33		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
0.5	100	0.0300	3.52		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
5.8	233	Total,	Increased t	o minimum	Tc = 6.0 min					

Summary for Subcatchment SC1.2:

Runoff = 3.7 cfs @ 12.16 hrs, Volume= 14,386 cf, Depth> 6.53"

Area (sf)	CN	Description
6,620	61	>75% Grass cover, Good, HSG B
2,398	55	Woods, Good, HSG B
1,172	98	Unconnected pavement, HSG B
8,000	98	Roofs, HSG B
7,203	98	Paved parking, HSG B
1,052	96	Gravel surface, HSG B
26,444	85	Weighted Average
10,069		38.08% Pervious Area
16,375		61.92% Impervious Area
1,172		7.16% Unconnected
	Area (sf) 6,620 2,398 1,172 8,000 7,203 1,052 26,444 10,069 16,375 1,172	Area (sf) CN 6,620 61 2,398 55 1,172 98 8,000 98 7,203 98 1,052 96 26,444 85 10,069 16,375 1,172 98

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.9	41	0.2400	0.17		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
1.4	9	0.0200	0.10		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
3.4	222	0.0248	1.10		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	63	0.0300	3.52		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
9.0	335	Total			

Summary for Subcatchment SC1.3:

Runoff	=	3.5 cfs @	12.21 hrs,	Volume=	15,206 cf,	Depth>	5.45"
			· _ · _ · · · · •,		,,		

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

A	rea (sf)	CN [Description						
	15,029	61 >	61 >75% Grass cover, Good, HSG B						
	3,894	55 V	Voods, Go	od, HSG B					
	1,741	96 C	Gravel surfa	ace, HSG E	3				
	275	98 l	Jnconnecte	ed pavemer	nt, HSG B				
	12,548	98 F	Paved park	ing, HSG B					
	33,487	76 V	Veighted A	verage					
	20,664	6	61.71% Pei	vious Area					
	12,823	3	38.29% Imp	pervious Are	ea				
	275	2	2.14% Unco	onnected					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.2	50	0.0800	0.12		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.12"				
3.2	188	0.0372	0.96		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
2.0	160	0.0372	1.35		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.6	122	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
13.0	520	Total							

Summary for Subcatchment SC2.1:

Runoff	=	1.6 cfs @	12.13 hrs,	Volume=	6,073 cf,	Depth>	7.85"
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A	rea (sf)	CN	Description					
	595	61	>75% Gras	s cover, Go	bood, HSG B			
	423	98	Unconnecte	ed pavemer	ent, HSG B			
	4,000	98	Roofs, HSG	βB				
	4,263	98	Paved park	ing, HSG B	В			
	9,281	96	Weighted A	verage				
	595		6.41% Pervious Area					
	8,686		93.59% Impervious Area					
	423		4.87% Unconnected					
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
5.0					Direct Entry,			
5.0	0	Total,	Increased t	o minimum	n Tc = 6.0 min			

Summary for Subcatchment SC2.2:

2.6 cfs @ 12.15 hrs, Volume= 9,363 cf, Depth> 6.05" Runoff =

A	rea (sf)	CN [Description		
	5,529	61 >	75% Gras	s cover, Go	ood, HSG B
	2,637	55 V	Voods, Go	od, HSG B	
	889	98 l	Jnconnecte	ed pavemer	nt, HSG B
	4,000	98 F	Roofs, HSC	βB	
	4,580	98 F	Paved park	ing, HSG B	5
	929	96 (Gravel surfa	ace, HSG E	3
	18,563	81 V	Veighted A	verage	
	9,094	2	8.99% Pe	vious Area	
	9,469	5	51.01% Imp	pervious Ar	ea
	889	ç	.39% Unc	onnected	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.6	50	0.2400	0.18		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.12"
0.2	8	0.0200	0.71		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
2.5	151	0.0200	0.99		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.5	86	0.0200	2.87		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
7.8	295	Total			

Summary for Subcatchment SC2.3:

Runoff = 1.5 cfs @ 12.13 hrs, Volume= 5,137 cf, Depth> 5.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

A	rea (sf)	CN [Description		
	7,218	61 >	>75% Gras	s cover, Go	ood, HSG B
	293	98 l	Jnconnecte	ed pavemer	nt, HSG B
	4,029	98 F	Paved park	ing, HSG B	
	11,540	75 V	Veighted A	verage	
	7,218	6	62.55% Per	vious Area	
	4,321	3	37.45% Imp	pervious Are	ea
	293	6	6.77% Unco	onnected	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
2.3	42	0.1430	0.31		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.12"
0.1	8	0.0500	1.18		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 3.12"
0.5	135	0.0500	4.54		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
2.9	185	Total, I	Increased t	o minimum	1 Tc = 6.0 min

Summary for Subcatchment SC3.1:

Runoff = 2.0 cfs @ 12.13 hrs, Volume= 7,832 cf, Depth> 8.09"

A	rea (sf)	CN	Description			
	11,615	98	Roofs, HSG	βB		
	0	61	>75% Gras	s cover, Go	ood, HSG B	
	11,615	98	Weighted A	verage		
	0		0.00% Perv	ious Area		
	11,615	100.00% Impervious Area				
Tc	Length	Slope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cts)		
5.0					Direct Entry,	
5.0	0	Total,	Increased t	o minimum	n Tc = 6.0 min	

Summary for Subcatchment SC3.2:

Runoff = 4.3 cfs @ 12.13 hrs, Volume= 16,544 cf, Depth> 7.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

A	rea (sf)	CN I	Description		
	2,372	61 :	>75% Gras	s cover, Go	ood, HSG B
	21,883	98 I	Paved park	ing, HSG B	
	1,422	98	Jnconnecte	ed pavemer	nt, HSG B
	25,677	95 V	Neighted A	verage	
	2,372	ę	9.24% Perv	ious Area	
	23,305	ę	90.76% Imp	pervious Are	ea
	1,422	(6.10% Unco	onnected	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.4	50	0.0380	0.19		Sheet Flow,
0.8	68	0.0380	1.36		Grass: Short n= 0.150 P2= 3.12" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
5.2	118	Total,	Increased t	o minimum	Tc = 6.0 min

Summary for Subcatchment SC3.3:

Runoff = 1.5 cfs @ 12.13 hrs, Volume= 4,947 cf, Depth> 4.05"

A	rea (sf)	CN [Description			
	13,540	61 >	>75% Gras	s cover, Go	ood, HSG B	
	1,127	96 (Gravel surfa	ace, HSG E	3	
	14,666	64 \	Neighted A	verage		
	14,666		100.00% Pe	ervious Are	а	
Тс	Length	Slope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.7	50	0.0600	0.23		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.12"	
1.9	166	0.0422	1.44		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
5.6	216	Total,	Increased t	o minimum	Tc = 6.0 min	

NRCC 24-III D 100-Year Rainfal=0.34Propared by Goldsmith, Prest & Ringwall, Inc.Printed 7/13/2022HydroCAD® 10.10-4a s/n 01036 @ 2020 HydroCAD Software Solutions LLCPage 78Summary for Subcatchment SC4.1:Runoff = 2.5 cfs @ 12.13 hrs, Volume= 10,114 cf, Depth> 8.09"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D 100-Year Rainfall=8.34"Area (sf) CN Description14,99898 Roofs, HSG B14,998100.00% Impervious AreaTc Length Slope Velocity Capacity Description(min) (feet)(ft/ft) (ft/sec)5.00Direct Entry,5.00Total, Increased to minimum Tc = 6.0 minSummary for Subcatchment SC4.2:Runoff = 1.5 cfs @ 12.13 hrs, Volume= 5,187 cf, Depth> 5.94"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D 100-Year Rainfall=8.34"Area (sf) CN Description5,060 61 >75% Grass cover, Good, HSC B1,450 98Unconnected pavement, HSC B3,975 98Paved parking, HSC B10,484 80Weighted Average5,060 48.26% Pervious Area5,424 51.74% Impervious Area1,450 26.72% UnconnectedTc Length Slope Velocity Capacity Description(min) (feet)(ft/ft) (ft/sec)(cfs)4.0 50 <th>DOOT David</th> <th></th> <th>203 Ayer Road, Harvard, MA</th>	DOOT David		203 Ayer Road, Harvard, MA							
Summary for Subcatchment SC4.1: Printed // 13/2022 Runoff = 2.5 cfs @ 12.13 hrs, Volume= 10,114 cf, Depth> 8.09" Runoff = 2.5 cfs @ 12.13 hrs, Volume= 10,114 cf, Depth> 8.09" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34" Area (sf) CN Description 14,998 98 Roofs, HSG B 14,998 100.00% Impervious Area Tc Length Slope Stommary for Subcatchment SC4.2: Summary for Subcatchment SC4.2: Runoff = 1.5 cfs @ Stommary for Subcatchment SC4.2: Summary for Subcatchment SC4.2: Runoff = 1.5 cfs @ Stommary for Subcatchment SC4.2: Summary for Subcatchment SC4.2: Runoff = 1.5 cfs @ Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34" Area (sf) CN Description 5,060 61<>75% Grass cover, Good, HSG B 1,450 <td>PUSI Rev-1</td> <td></td> <td>NRCC 24-nr D 100-Year Rainfall=8.34</td>	PUSI Rev-1		NRCC 24-nr D 100-Year Rainfall=8.34							
Prigle 70 Summary for Subcatchment SC4.1: Runoff = 2.5 cfs @ 12.13 hrs, Volume= 10,114 cf, Depth> 8.09" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34" Area (sf) CN Description 14,998 98 Roofs, HSG B 14,998 100.00% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/scc) (cfs) Direct Entry, 5.0 0 Total, Increased to minimum Tc = 6.0 min Summary for Subcatchment SC4.2: Runoff = 1.5 cfs @ 12.13 hrs, Volume= 5,187 cf, Depth> 5.94" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34" Area (sf) CN Description 5,060 61 >75% Grass cover, Good, HSG B 3,975 98 Paved parking, HSG B 0.48 26% Pervious Area 5,424 51.74% Impervious Area 5,424	Prepared by Go	Idsmith, Prest & Ringwa	All, Inc. Printed 7/13/2022							
Summary for Subcatchment SC4.1:Runoff= $2.5 \text{ cfs} @ 12.13 \text{ hrs}, Volume=$ $10,114 \text{ cf}, Depth> 8.09"$ Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D 100-Year Rainfall=8.34"Area (sf)CNDescription14,99898Roofs, HSG B14,998100.00% Impervious AreaTcLengthSlopeVelocityCapacityDescription(min)(ft/ft)(ft/sec)5.00Total, Increased to minimum Tc = 6.0 minSummary for Subcatchment SC4.2:Runoff=1.5 cfs @ 12.13 hrs, Volume=5.187 cf, Depth> 5.94"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D 100-Year Rainfall=8.34"Area (sf)CNDescription5.060615.060615.060615.060617.4509898Paved parking, HSG B10.4848080Weighted Average5.06048.26% Pervious Area5.42451.74% Impervious Area <td></td> <td>-4a s/1101030 © 2020 Hyd</td> <td>TOCAD Soliware Solutions LLC Page 78</td>		-4a s/1101030 © 2020 Hyd	TOCAD Soliware Solutions LLC Page 78							
$ \begin{array}{rcl} {\sf Runoff} & = & 2.5 \ {\sf cfs} @ 12.13 \ {\sf hrs}, \ {\sf Volume} & 10,114 \ {\sf cf}, \ {\sf Depth} > 8.09" \\ {\sf Runoff} \ {\sf by} \ {\sf SCS} \ {\sf TR-20} \ {\sf method}, \ {\sf UH=SCS}, \ {\sf Weighted-CN}, \ {\sf Time} \ {\sf Span=} 0.00-24.00 \ {\sf hrs}, \ {\sf dt=} 0.05 \ {\sf hrs} \\ {\sf NRCC} \ 24-{\sf hr} \ D \ 100-{\sf Year} \ {\sf Rainfall=8.34"} \\ \hline \\ \hline \\ \hline \\ \hline \ \ \ \ \ \ \ \ \ \ \ \$		Summary for Subcatchment SC4.1:								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"Area (sf)CNDescription14,99898Roofs, HSG B14,998100.00% Impervious AreaTcLengthSlopeVelocityCapacityDescription(min)(fett)(ft/ft)(ft/ft)(ft/sec)(cfs)5.0Direct Entry,5.00Total, Increased to minimum Tc = 6.0 minSummary for Subcatchment SC4.2:Runoff=1.5 cfs @ 12.13 hrs, Volume=5,187 cf, Depth> 5.94"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D100-Year Rainfall=8.34"Area (sf)CNDescription5,06061>75% Grass cover, Good, HSG B1,45098Unconnected pavement, HSG B3,97598Paved parking, HSG B1,45026.72% UnconnectedTcLengthSlopeVelocity Capacity0.4330.05000.21CheatSheet Flow, Grass: Short n=0.1500.4330.5001.57Shallow Concentrated Flow, Short Grass Pasture0.40.0140	Runoff =	2.5 cfs @ 12.13 hrs	, Volume= 10,114 cf, Depth> 8.09"							
Area (sf)CNDescription14,99898Roofs, HSG B14,998100.00% Impervious AreaTcLengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)(ft/ft)(ft/sec)(cfs)5.0Direct Entry,5.00Total, Increased to minimum Tc = 6.0 minSummary for Subcatchment SC4.2:Runoff=1.5 cfs @12.13 hrs, Volume=5,187 cf, Depth> 5.94"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D100-Year Rainfall=8.34"Area (sf)CNDescription5.06061>75% Grass cover, Good, HSG B1,45098Unconnected pavement, HSG B3,97598Paved parking, HSG B10,48480Weighted Average5,06048.26% Pervious Area5,42451.74% Impervious Area1,45026.72% UnconnectedTcLengthSlope4.0500.05000.21Sheet Flow, Grass: Short n= 0.1500.4330.05000.57Shallow Concentrated Flow, Short Grass Pasture0.4330.05001.57Shatllow Concentrated Flow, Short Grass Pasture0.401.01	Runoff by SCS T NRCC 24-hr D 1	R-20 method, UH=SCS, \ 00-Year Rainfall=8.34"	Veighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs							
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Summary for Subcatchment SC4.2:Runoff= $1.5 \text{ cfs} @ 12.13 \text{ hrs}, Volume=$ $5,187 \text{ cf}, Depth> 5.94"$ Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D100-Year Rainfall=8.34"Area (sf)CNDescription $5,060$ 61 75% Grass cover, Good, HSG B $1,450$ 98Unconnected pavement, HSG B $3,975$ 98Paved parking, HSG B $10,484$ 80Weighted Average $5,600$ 48.26% Pervious Area $5,424$ 51.74% Impervious Area $1,450$ 26.72% UnconnectedTcLengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)(ft/sec)(cfs) 4.0 500.05000.21Sheet Flow, Grass: Short n= 0.1500.4330.05001.57Shallow Concentrated Flow, Short Grass Pasture $Ky=7.0$ fps	5.0 0	Total, Increased to mir	imum Tc = 6.0 min							
Summary for Subcatchment SC4.2:Runoff=1.5 cfs @ 12.13 hrs, Volume= $5,187$ cf, Depth> 5.94"Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrsNRCC 24-hr D100-Year Rainfall=8.34"Area (sf) CN Description $5,060$ 61 $5,060$ 61 275% Grass cover, Good, HSG B $1,450$ 98 98 Unconnected pavement, HSG B $3,975$ 98 98 Paved parking, HSG B $10,484$ 80 80 Weighted Average $5,060$ 48.26% Pervious Area $5,424$ 51.74% Impervious Area $1,450$ 26.72% UnconnectedTc Length Slope Velocity Capacity Description(min)(fet)(ft/ft)(ft/sec)(cfs)(cfs) 4.0 50 0.0500 0.21 Sheet Flow, Grass: Short n= 0.150 P2= 3.12" 0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture Ky= 7.0 fps										
$\begin{array}{rcl} \operatorname{Runoff} &=& 1.5 \mathrm{cfs} @ \ 12.13 \mathrm{hrs}, \mathrm{Volume}=& 5,187 \mathrm{cf}, \mathrm{Depth}>5.94"\\ \end{array}$		Summary	/ for Subcatchment SC4.2:							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"Area (sf)CNDescription $5,060$ 61>75% Grass cover, Good, HSG B1,45098Unconnected pavement, HSG B3,97598Paved parking, HSG B10,48480Weighted Average $5,060$ 48.26% Pervious Area $5,424$ 51.74% Impervious Area $5,424$ 51.74% Impervious Area $1,450$ 26.72% UnconnectedTc Length Slope Velocity Capacity Description(min)(feet)(ft/ft)(ft/sec)(cfs)Capacity Description4.0500.05000.21Sheet Flow, Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	Runoff =	1.5 cfs @ 12.13 hrs	, Volume= 5,187 cf, Depth> 5.94"							
Area (sf) CN Description 5,060 61 >75% Grass cover, Good, HSG B 1,450 98 Unconnected pavement, HSG B 3,975 98 Paved parking, HSG B 10,484 80 Weighted Average 5,060 48.26% Pervious Area 5,424 51.74% Impervious Area 1,450 26.72% Unconnected Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs) 4.0 50 0.0500 0.21 Sheet Flow, Grass: Short n= 0.150 P2= 3.12" 0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture	Runoff by SCS T NRCC 24-hr D 1	R-20 method, UH=SCS, \ 00-Year Rainfall=8.34"	Veighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs							
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3,975 98 Paved parking, HSG B 10,484 80 Weighted Average 5,060 48.26% Pervious Area 5,424 51.74% Impervious Area 1,450 26.72% Unconnected Tc Length Slope Velocity Capacity Unconnected Minimity (ft/ft) 4.0 50 0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	1,450	98 Unconnected par	/ement, HSG B							
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5,424 31.74% Impervious Area 1,450 26.72% Unconnected Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 4.0 50 0.0500 0.21 Sheet Flow, Grass: Short n= 0.150 P2= 3.12" 0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	5,060	48.26% Pervious								
Tc Length Slope Velocity Capacity (cfs) Description (min) (feet) (ft/ft) (ft/sec) (cfs) 4.0 50 0.0500 0.21 Sheet Flow, Grass: Short n= 0.150 P2= 3.12" 0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture 0.4 0.0140 0.0140 0.0140	5,424 1 450	26 72% Unconne	us Area							
TcLengthSlopeVelocityCapacity (cfs)Description(min)(feet)(ft/ft)(ft/sec)(cfs)4.0500.05000.21Sheet Flow, Grass: Short n= 0.150P2= 3.12"0.4330.05001.57Shallow Concentrated Flow, Short Grass PastureKv= 7.0 fps	1,400	20.7270 011001110								
(min) (ft/ft) (ft/sec) (cfs) 4.0 50 0.0500 0.21 Sheet Flow, Grass: Short n= 0.150 P2= 3.12" 0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	Tc Length	Slope Velocity Cap	acity Description							
4.0 50 0.0500 0.21 Sneet Flow, Grass: Short n= 0.150 P2= 3.12" 0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	(min) (feet)	(ft/ft) (ft/sec)	(cfs)							
0.4 33 0.0500 1.57 Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	4.0 50	0.0500 0.21	Sneet Flow, Grass: Short_n= 0.150_P2= 3.12"							
Short Grass Pasture Kv= 7.0 fps	0.4 33	0.0500 1.57	Shallow Concentrated Flow.							
		-	Short Grass Pasture Kv= 7.0 fps							
0.6 148 0.0440 4.26 Shallow Concentrated Flow,	0.6 148	0.0440 4.26	Shallow Concentrated Flow,							
$= \frac{Paved KV = 20.3 \text{ Ips}}{5.0}$	5.0 231	Total Increased to min	$\frac{1}{10000000000000000000000000000000000$							

Summary for Subcatchment SC4.3:

Runoff = 0.8 cfs @ 12.13 hrs, Volume= 3,017 cf, Depth> 7.73"

203 Ayer Road, Harvard, MA NRCC 24-hr D 100-Year Rainfall=8.34" Printed 7/13/2022 HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLC Page 79

POST Rev-1 Prepared by Goldsmith, Prest & Ringwall, Inc.

Area (sf) Description CN >75% Grass cover, Good, HSG B 406 61 175 98 Unconnected pavement, HSG B Paved parking, HSG B 4,101 98 Weighted Average 4,682 95 8.68% Pervious Area 406 91.32% Impervious Area 4,276 4.09% Unconnected 175 Тс Length Slope Velocity Capacity Description (feet) (min) (ft/ft) (ft/sec) (cfs) 5.0 Direct Entry, 0 Total, Increased to minimum Tc = 6.0 min 5.0

Summary for Subcatchment SC4.4:

Runoff 0.9 cfs @ 12.13 hrs, Volume= 3,258 cf, Depth> 7.61" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-Year Rainfall=8.34"

89.01% Impervious Area				
-				

Summary for Subcatchment SC4.5:

0.6 cfs @ 12.13 hrs, Volume= Runoff 1,970 cf, Depth> 3.70" =

Area (sf)	CN	Description
6,391	61	>75% Grass cover, Good, HSG B
6,391		100.00% Pervious Area

POST Rev-1							NRCC 24-hr D 100-Year Rainfall=8.34"		
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HydroCA	<u>D® 10.10-</u>	4a_s/n_01	<u>036 © 2</u>	020 HydroC	AD Soft	vare Solu	utions LLC		Page 80
Tc (min)	Length	Slope	Velocit	y Capacit	ty Des	cription			
5.0	(1001)	(1011)	(10,000		o, Dire	ct Entr	/_		
5.0	0	Total. I	ncrease	d to minimu		6.0 min	,		
0.0	·								
	Summary for Subcatchment SC5.1:								
Runoff	=	5.2 c	fs @ 12	2.18 hrs, V	/olume=		20,281 cf, Dep	oth> 3.93"	
Runoff b NRCC 2	y SCS TF 4-hr D 10	R-20 meth 0-Year F	າod, UH Rainfall=	=SCS, Wei 8.34"	ghted-C	N, Time	Span= 0.00-24	.00 hrs, dt= 0.05 h	Irs
А	vrea (sf)	CN D	escriptio	on					
	52.172	61 >	75% Gr	ass cover.	Good. H	SG B			
	5,271	55 V	Voods, C	Good, HSG	В				
	70	98 L	Inconne	cted paven	nent, HS	GΒ			
	1,456	98 P	aved pa	irking, HSC	βB				
	3,033	96 0			эB				
	62,001 60,476	63 V	7 54% E	I Average	00				
	1 525	2	46% Im	pervious A	rea				
	70	4	.56% Ur	nconnected					
Tc	Length	Slope	Velocit	y Capaci	ty Des	cription			
(min)	(feet)	(ft/ft)	(ft/sec	c) (cfs	s)				
3.8	36	0.2000	0.1	6	She	et Flow	, , , , ,		.
0.1	1.1	0 0 0 0 0 0	0.1	1	VVoc	ods: Ligh	it underbrush i	n= 0.400 P2= 3.1	2"
Ζ.Ι	14	0.0200	0.1	I	Grad	et Flow	, t n=0.150 P2	2= 3 12"	
4.1	312	0.0334	1.2	8	Sha	llow Co	ncentrated Flo	- 0.12 W.	
	0.2	0.0001		•	Sho	rt Grass	Pasture Kv=7	7.0 fps	
10.0	362	Total						•	
				Summ	ary for	Pond	CB-1:		
Inflow A	rea =	15.2	15 sf 2	6 17% Imp	ervious	Inflow	Depth > 4 87"	for 100-Year ev	ent
Inflow	=	1.8 c	fs @ 12	2.13 hrs, V	/olume=	mon	6,173 cf		one
Outflow	=	1.8 c	fs @ 1:	2.13 hrs, V	/olume=		6,173 cf, Atte	en= 0%, Lag= 0.0	min
Primary	=	1.8 c	fs @ 12	2.13 hrs, V	/olume=		6,173 cf		
Routing Peak Ele	by Dyn-Si ev= 298.3	tor-Ind m 8' @ 12. <i>1</i>	ethod, T 13 hrs	ïme Span=	0.00-24	1.00 hrs,	dt= 0.05 hrs / 2	2	
Flood El	lev= 301.5	50'							
Device	Routing		Invert	Outlet Dev	vices				
#1	Primary	2	97.50'	12.0" Ro	und Cul	vert			
	2			L= 170.0'	CPP, p	rojectin	g, no headwall,	Ke= 0.900	
				Inlet / Out	let Inver	t= 297.5	0'/293.56' S=	: 0.0232 '/' Cc= 0	.900
				n= 0.013	Corruga	ted PE,	smooth interior	, Flow Area= 0.79) ST

203 Ayer Road, Harvard, MA

Primary OutFlow Max=1.8 cfs @ 12.13 hrs HW=298.35' TW=296.95' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.8 cfs @ 2.48 fps)

Summary for Pond CB-10:

Inflow Area	a =	10,484 sf,	51.74% Impervious,	Inflow Depth > 5.	94" for 100-Year event
Inflow	=	1.5 cfs @	12.13 hrs, Volume=	5,187 cf	
Outflow	=	1.5 cfs @	12.13 hrs, Volume=	5,187 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.5 cfs @	12.13 hrs, Volume=	5,187 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.78' @ 12.13 hrs Flood Elev= 289.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.90'	12.0" Round Culvert
			Inlet / Outlet Invert= $285.90'$ / $285.70'$ S= 0.0400 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior. Flow Area= 0.79 sf
			n= 0.010 Contigued i E, shooti intenor, i low Alca- 0.75 si

Primary OutFlow Max=1.4 cfs @ 12.13 hrs HW=286.74' TW=286.46' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.4 cfs @ 2.02 fps)

Summary for Pond CB-2:

Inflow Area	a =	26,444 sf,	61.92% Impervious,	Inflow Depth > 6.	53" for 100-Year event
Inflow	=	3.7 cfs @	12.16 hrs, Volume=	14,386 cf	
Outflow	=	3.7 cfs @	12.16 hrs, Volume=	14,386 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	3.7 cfs @	12.16 hrs, Volume=	14,386 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 298.79' @ 12.15 hrs Flood Elev= 297.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.70'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.70' / 293.56' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.6 cfs @ 12.16 hrs HW=298.58' TW=297.12' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.6 cfs @ 4.60 fps) 203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D 100-Year Rainfall=8.34"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 82

Summary for Pond CB-3:

Inflow Are	a =	33,487 sf,	38.29% Impervious,	Inflow Depth > 5.4	5" for 100-Year event
Inflow	=	3.5 cfs @	12.21 hrs, Volume=	15,206 cf	
Outflow	=	3.5 cfs @	12.21 hrs, Volume=	15,206 cf, A	Atten= 0%, Lag= 0.0 min
Primary	=	3.5 cfs @	12.21 hrs, Volume=	15,206 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.54' @ 12.18 hrs Flood Elev= 291.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.50'	12.0" Round Culvert L= 8.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 287.50' / 287.40' S= 0.0125 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.5 cfs @ 12.21 hrs HW=290.40' TW=289.02' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.5 cfs @ 4.46 fps)

Summary for Pond CB-5:

Inflow Area	a =	18,563 sf,	51.01% Impervious,	Inflow Depth > 6	6.05" for 10	0-Year event
Inflow	=	2.6 cfs @	12.15 hrs, Volume=	9,363 cf	:	
Outflow	=	2.6 cfs @	12.15 hrs, Volume=	9,363 cf	, Atten= 0%,	Lag= 0.0 min
Primary	=	2.6 cfs @	12.15 hrs, Volume=	9,363 cf	:	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 295.23' @ 12.15 hrs Flood Elev= 298.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	294.00'	12.0" Round Culvert L= 86.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 294.00' / 289.10' S= 0.0570 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.5 cfs @ 12.15 hrs HW=295.22' TW=291.69' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.5 cfs @ 3.22 fps)

Summary for Pond CB-6:

Inflow Area	a =	9,281 sf,	93.59% Impervious,	Inflow Depth > 7.	85" for 100-Year event
Inflow	=	1.6 cfs @	12.13 hrs, Volume=	6,073 cf	
Outflow	=	1.6 cfs @	12.13 hrs, Volume=	6,073 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.6 cfs @	12.13 hrs, Volume=	6,073 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2

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Peak Elev= 296.28' @ 12.13 hrs Flood Elev= 299.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	295.50'	12.0" Round Culvert L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 295.50' / 289.10' S= 0.1103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.5 cfs @ 12.13 hrs HW=296.26' TW=291.42' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.5 cfs @ 2.34 fps)

Summary for Pond CB-7:

Inflow Area	a =	4,682 sf,	91.32% Impervious,	Inflow Depth > 7.7	73" for 100-Year event
Inflow	=	0.8 cfs @	12.13 hrs, Volume=	3,017 cf	
Outflow	=	0.8 cfs @	12.13 hrs, Volume=	3,017 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	0.8 cfs @	12.13 hrs, Volume=	3,017 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.53' @ 12.30 hrs Flood Elev= 292.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.00'	12.0" Round Culvert L= 22.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.00' / 287.70' S= 0.0136 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.0 cfs @ 12.13 hrs HW=289.73' TW=290.02' (Dynamic Tailwater) 1=Culvert (Controls 0.0 cfs)

Summary for Pond CB-8:

Inflow Are	a =	5,136 sf,	89.01% Impervious,	Inflow Depth > 7.6	61" for 100-Year event
Inflow	=	0.9 cfs @	12.13 hrs, Volume=	3,258 cf	
Outflow	=	0.9 cfs @	12.13 hrs, Volume=	3,258 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	0.9 cfs @	12.13 hrs, Volume=	3,258 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.50' @ 12.30 hrs Flood Elev= 292.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.50'	12.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 288.50' / 288.20' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.0 cfs @ 12.13 hrs HW=289.60' TW=289.88' (Dynamic Tailwater)

Summary for Pond CB-9:

Inflow Area	a =	11,540 sf,	37.45% Impervious,	Inflow Depth > 5.3	34" for 100-Year event
Inflow	=	1.5 cfs @	12.13 hrs, Volume=	5,137 cf	
Outflow	=	1.5 cfs @	12.13 hrs, Volume=	5,137 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	1.5 cfs @	12.13 hrs, Volume=	5,137 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.26' @ 12.13 hrs Flood Elev= 290.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.50'	12.0" Round Culvert L= 40.0' CPP, projecting, no headwall, Ke= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.4 cfs @ 12.13 hrs HW=287.24' TW=286.46' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.4 cfs @ 2.31 fps)

Summary for Pond DCB-4:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth > 7.7	73" for 100-Year event
Inflow	=	4.3 cfs @	12.13 hrs, Volume=	16,544 cf	
Outflow	=	4.3 cfs @	12.13 hrs, Volume=	16,544 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	4.3 cfs @	12.13 hrs, Volume=	16,544 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 288.05' @ 12.14 hrs Flood Elev= 288.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.50'	12.0" Round Culvert L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.50' / 284.20' S= 0.0429 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.7 cfs @ 12.13 hrs HW=287.88' TW=286.38' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.7 cfs @ 4.65 fps) 203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D 100-Year Rainfall=8.34"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 85

Summary for Pond DMH-1:

Inflow Area =		41,659 sf,	48.86% Impervious,	Inflow Depth > 5.9	92" for 100-Year event
Inflow	=	5.4 cfs @	12.15 hrs, Volume=	20,560 cf	
Outflow	=	5.4 cfs @	12.15 hrs, Volume=	20,560 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	5.4 cfs @	12.15 hrs, Volume=	20,560 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 297.29' @ 12.15 hrs Flood Elev= 297.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	293.46'	12.0" Round Culvert L= 188.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 293.46' / 287.40' S= 0.0322 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=5.4 cfs @ 12.15 hrs HW=297.24' TW=289.25' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 5.4 cfs @ 6.88 fps)

Summary for Pond DMH-2:

Inflow Are	ea =	75,146 sf,	44.15% Impervious,	Inflow Depth > 5.71"	for 100-Year event
Inflow	=	8.6 cfs @	12.16 hrs, Volume=	35,765 cf	
Outflow	=	8.6 cfs @	12.16 hrs, Volume=	35,765 cf, Atte	en= 0%, Lag= 0.0 min
Primary	=	8.6 cfs @	12.16 hrs, Volume=	35,765 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 289.30' @ 12.16 hrs Flood Elev= 291.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	286.90'	18.0" Round Culvert L= 48.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 286.90' / 286.42' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=8.5 cfs @ 12.16 hrs HW=289.23' TW=285.99' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 8.5 cfs @ 4.78 fps)

Summary for Pond DMH-3:

Inflow Area	a =	25,677 sf,	90.76% Impervious,	Inflow Depth > 7.73"	for 100-Year event
Inflow	=	4.3 cfs @	12.13 hrs, Volume=	16,544 cf	
Outflow	=	4.3 cfs @	12.13 hrs, Volume=	16,544 cf, Atte	en= 0%, Lag= 0.0 min
Primary	=	4.3 cfs @	12.13 hrs, Volume=	16,544 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
Peak Elev= 287.18' @ 12.30 hrs Flood Elev= 288.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	284.10'	24.0" Round Culvert L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 284.10' / 284.00' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.1 cfs @ 12.13 hrs HW=286.38' TW=286.26' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 4.1 cfs @ 1.30 fps)

Summary for Pond DMH-5:

Inflow Area	a =	27,844 sf,	65.20% Impervious,	Inflow Depth >	6.65"	for 100-Year event
Inflow	=	4.1 cfs @	12.14 hrs, Volume=	15,436 0	of	
Outflow	=	4.1 cfs @	12.14 hrs, Volume=	15,436 d	cf, Atte	n= 0%, Lag= 0.0 min
Primary	=	4.1 cfs @	12.14 hrs, Volume=	15,436 0	of	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 291.73' @ 12.15 hrs Flood Elev= 293.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	289.00'	12.0" Round Culvert L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 289.00' / 287.70' S= 0.0148 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
			5

Primary OutFlow Max=3.5 cfs @ 12.14 hrs HW=291.59' TW=290.14' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 3.5 cfs @ 4.48 fps)

Summary for Pond DMH-6:

Inflow Area	a =	32,526 sf,	68.96% Impervious,	Inflow Depth > 6	.81" for 100-Year event
Inflow	=	4.8 cfs @	12.14 hrs, Volume=	18,453 cf	
Outflow	=	4.8 cfs @	12.14 hrs, Volume=	18,453 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	4.8 cfs @	12.14 hrs, Volume=	18,453 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.54' @ 12.24 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.7 cfs @ 12.14 hrs HW=290.12' TW=289.96' (Dynamic Tailwater) -1=Culvert (Inlet Controls 4.7 cfs @ 1.50 fps)

Summary for Pond DMH-7:

Inflow Are	ea =	5,136 sf,	89.01% Impervious,	Inflow Depth > 7.61	for 100-Year event
Inflow	=	0.9 cfs @	12.13 hrs, Volume=	3,258 cf	
Outflow	=	0.9 cfs @	12.13 hrs, Volume=	3,256 cf, At	tten= 0%, Lag= 0.0 min
Primary	=	0.9 cfs @	12.13 hrs, Volume=	3,256 cf	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.50' @ 12.26 hrs Flood Elev= 292.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	287.60'	24.0" Round Culvert
			L= 5.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 287.60' / 287.50' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.8 cfs @ 12.13 hrs HW=289.88' TW=289.87' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.8 cfs @ 0.26 fps)

Summary for Pond DMH-9:

Inflow Area	a =	22,024 sf,	44.25% Impervious,	Inflow Depth > 5	.63" for 100-Year event
Inflow	=	3.0 cfs @	12.13 hrs, Volume=	10,324 cf	
Outflow	=	3.0 cfs @	12.13 hrs, Volume=	10,324 cf,	Atten= 0%, Lag= 0.0 min
Primary	=	3.0 cfs @	12.13 hrs, Volume=	10,324 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.49' @ 12.13 hrs Flood Elev= 290.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	285.45'	15.0" Round Culvert L= 197.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 285.45' / 282.00' S= 0.0175 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
D .			40.40 km 10.01 000.40 10.000 570 (Demonstrates Tellerstein)

Primary OutFlow Max=2.9 cfs @ 12.13 hrs HW=286.46' TW=283.57' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.9 cfs @ 2.70 fps) 203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D 100-Year Rainfall=8.34"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 88

Summary for Pond IB-1:

Inflow Area =	89,812 sf,	36.94% Impervious,	Inflow Depth > 5.44	for 100-Year event
Inflow =	10.0 cfs @	12.16 hrs, Volume=	40,712 cf	
Outflow =	9.8 cfs @	12.17 hrs, Volume=	40,437 cf, At	ten= 2%, Lag= 1.1 min
Discarded =	0.0 cfs @	12.17 hrs, Volume=	1,020 cf	
Primary =	2.2 cfs @	12.17 hrs, Volume=	30,646 cf	
Secondary =	7.6 cfs @	12.17 hrs, Volume=	8,770 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 286.00' @ 12.17 hrs Surf.Area= 1,424 sf Storage= 1,430 cf

Plug-Flow detention time= 10.2 min calculated for 40,437 cf (99% of inflow) Center-of-Mass det. time= 6.1 min (832.9 - 826.9)

Volume	Invert	Avail.	Storage	Storage Description	n		
#1	284.00'	3	3,366 cf	Custom Stage Dat	ta (Irregular) Liste	d below (Recalc)	
Elevatio	on Su	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(166	:()	(Sq-II)	(leet)	(Cubic-leet)		(sq-it)	
284.0	00	208	72.5	0	0	208	
285.0	00	661	129.3	413	413	1,126	
286.0	00	1,425	189.1	1,019	1,432	2,649	
287.0	00	2,492	246.3	1,934	3,366	4,643	
Device	Routing	Inve	ert Outle	et Devices			
#1	Discarded	284.0	0' 1.02	0 in/hr Exfiltration	over Surface area	a	
#2	Secondarv	285.5	0' 8.0'	long x 15.0' breadt	th Broad-Crested	Rectangular Weir	
	J		Head	d (feet) 0.20 0.40 (0.60 0.80 1.00 1	20 1.40 1.60	
			Coef	(English) 268 27	70 2 70 2 64 2 6	3 2 64 2 64 2 63	
#3	Primary	284.5	0' 10.0 '	" Round Culvert	a no headwall K	e= 0.900	
			Inlet n= 0	/ Outlet Invert= 284. .013, Flow Area= 0.	.50' / 282.50' S= .55 sf	0.0667 '/' Cc= 0.900	

Discarded OutFlow Max=0.0 cfs @ 12.17 hrs HW=285.99' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=2.1 cfs @ 12.17 hrs HW=285.99' TW=0.00' (Dynamic Tailwater) **3=Culvert** (Inlet Controls 2.1 cfs @ 3.93 fps)

Secondary OutFlow Max=7.4 cfs @ 12.17 hrs HW=285.99' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 7.4 cfs @ 1.89 fps)

Summary for Pond IB-2:

Inflow Area =	28,415 sf,	34.30% Impervious,	Inflow Depth > 5.	19" for 100-Year event
Inflow =	3.6 cfs @	12.13 hrs, Volume=	12,294 cf	
Outflow =	1.9 cfs @	12.23 hrs, Volume=	11,636 cf,	Atten= 46%, Lag= 6.1 min
Primary =	1.9 cfs @	12.23 hrs, Volume=	11,636 cf	
Secondary =	0.0 cfs @	0.00 hrs, Volume=	0 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 283.78' @ 12.23 hrs Surf.Area= 1,721 sf Storage= 2,273 cf

Plug-Flow detention time= 58.5 min calculated for 11,612 cf (94% of inflow) Center-of-Mass det. time= 28.7 min (859.3 - 830.6)

#1	282.00'		4,792 cf	Custom Stage Dat	a (Irregular) Listed	below (Recald
Elevation (feet)	Surf. (.Area sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
282.00		870	143.0	0	0	870
283.00		1,328	162.0	1,091	1,091	1,356
284.00		1,843	181.0	1,578	2,669	1,902
285.00		2,414	200.0	2,122	4,792	2,509

Device	Rouling	mven	Outlet Devices
#1	Secondary	284.00'	10.0' long x 22.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	282.50'	10.0" Round Culvert L= 37.4' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 282.50' / 278.50' S= 0.1070 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=1.9 cfs @ 12.23 hrs HW=283.77' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Inlet Controls 1.9 cfs @ 3.51 fps)

Secondary OutFlow Max=0.0 cfs @ 0.00 hrs HW=282.00' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.0 cfs)

Summary for Pond IC-1:

Inflow Area	n =	37,292 sf,	93.64% Imp	ervious,	Inflow Depth >	7.84"	for 100-	Year event	t
Inflow	=	6.2 cfs @	12.13 hrs, V	/olume=	24,376	cf			
Outflow	=	2.4 cfs @	12.30 hrs, V	/olume=	16,729	cf, Atte	en= 62%,	Lag= 10.3	min
Discarded	=	0.1 cfs @	5.90 hrs, V	/olume=	8,217	cf			
Primary	=	2.3 cfs @	12.30 hrs, V	/olume=	8,512	cf			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 287.17' @ 12.30 hrs Surf.Area= 4,404 sf Storage= 9,501 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 56.9 min (810.5 - 753.6)

203 Ayer Road, Harvard, MA NRCC 24-hr D 100-Year Rainfall=8.34" Printed 7/13/2022

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POST Rev-1

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Volume	Invert	Avail.Storage	Storage Description
#1A	283.50'	3,942 cf	53.75'W x 81.94'L x 3.50'H Field A
			15,414 cf Overall - 5,559 cf Embedded = 9,856 cf x 40.0% Voids
#2A	284.00'	5,559 cf	ADS_StormTech SC-740 +Cap x 121 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			121 Chambers in 11 Rows
		9,501 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	283.50'	1.020 in/hr Exfiltration over Surface area
#2	Primary	285.70'	12.0" Round Culvert
	·		L= 53.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 285.70' / 279.00' S= 0.1264 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	286.00'	10.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 5.90 hrs HW=283.54' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=2.3 cfs @ 12.30 hrs HW=287.16' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 2.3 cfs of 2.9 cfs potential flow) -3=Orifice/Grate (Orifice Controls 2.3 cfs @ 4.16 fps)

Summary for Pond IC-2:

Inflow Area	a =	52,660 sf,	79.76% Impervious,	Inflow Depth >	7.25" for	100-Year event
Inflow	=	8.2 cfs @	12.13 hrs, Volume=	31,823 c	f	
Outflow	=	4.0 cfs @	12.26 hrs, Volume=	27,146 c	f, Atten= 5	1%, Lag= 7.4 min
Discarded	=	0.1 cfs @	5.55 hrs, Volume=	8,158 c	f	-
Primary	=	3.9 cfs @	12.26 hrs, Volume=	18,988 c	f	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 290.50' @ 12.26 hrs Surf.Area= 4,361 sf Storage= 9,407 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 44.6 min (813.9 - 769.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	287.00'	3,901 cf	39.50'W x 110.42'L x 3.50'H Field A
			15,265 cf Overall - 5,513 cf Embedded = 9,752 cf x 40.0% Voids
#2A	287.50'	5,513 cf	ADS_StormTech SC-740 +Cap x 120 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			120 Chambers in 8 Rows

9,414 cf Total Available Storage

203 Ayer Road, Harvard, MA**POST Rev-1**NRCC 24-hr D 100-Year Rainfall=8.34"Prepared by Goldsmith, Prest & Ringwall, Inc.Printed 7/13/2022HydroCAD® 10.10-4a s/n 01036 © 2020 HydroCAD Software Solutions LLCPage 91

Device	Routing	Invert	Outlet Devices
#1	Discarded	287.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	288.30'	12.0" Round Culvert
	-		L= 120.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 288.30' / 279.00' S= 0.0775 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	288.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.1 cfs @ 5.55 hrs HW=287.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=3.9 cfs @ 12.26 hrs HW=290.49' TW=0.00' (Dynamic Tailwater) -2=Culvert (Inlet Controls 3.9 cfs @ 4.94 fps) -3=Orifice/Grate (Passes 3.9 cfs of 4.3 cfs potential flow)

Summary for Link AP-1:

Inflow A	Area =	=	270,181 sf,	44.92% Impervious,	Inflow Depth >	4.39	" for 100-Year event
Inflow	=	•	22.0 cfs @	12.20 hrs, Volume=	98,834	cf	
Primary	y =		22.0 cfs @	12.20 hrs, Volume=	98,834 (cf, A	tten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Pre-Development Conditions				203 Ayer Roa Pro	ad, Harvard, MA ject No. 211009
			<u>Area (sf)</u>	<u>Area (Ac)</u>	
Total Subcatchment Areas			270,181	6.2	
Total Subcatchment Areas On-Site			270,181	6.2	
Total Area of Hydrolic Soil Groups On-	Site	В	270,181	6.2	
Surface Type Areas					
	Grass	В	181,979	4.2	
	Woods	в	35 529	0.8	

	Woods	В	35,529	0.8	
	Gravel	В	52,672	1.2	
Total Impervious Area			0	0.0	
Infiltration Volume					
Inches of Recharge per Storm Event		A B C D	0.60 0.35 0.25 0.10		

Infiltration Volume = $\sum \{ [(Total Subcatchment Area within HSG) - (Total Impervious Area within HSG) \} \}$ x (inches of Recharge Per Storm)}

Infiltration Volume

7,880	CF
-------	----

Post Development Conditions			203 Ayer Road, Harvard, MA Project No. 211009
		<u>Area (sf)</u>	<u>Area (Ac)</u>
Total Subcatchment Areas		270,181	6.2
Total Subcatchment Areas On-Site		270,181	6.2
Total Area of Hydrolic Soil Groups On-Site	В	270,181	6.2
Surface Type Areas Grass_	В	119,876	2.8
Gravel_	В	9,151	0.2
Woods_	В	19,783	0.5
Unconnected pavement	В	7,036	0.2
Paved parking	В	71,721	1.6
Roofs_	В	42,613	1.0
Total Impervious Area		121,370	2.8
Infiltration Volume			
Inches of Recharge per Storm Event	A B C D	0.60 0.35 0.25 0.10	
Infiltration Volume = $\sum \{ [(Total Subcatchment And Subc$	rea wit	hin HSG) - (Total I: x (incl	mpervious Area within HSG)] hes of Recharge Per Storm)}
Natural Infiltration Volume		4,340	CF
Pre-Development Infiltration Volume		7,880	CF
Required Iniltration Volume		3,540	CF
Provided Infiltration Volume			
Infiltration Chambers (IC-1) Infiltration Chambers (IC-2) Infiltration Basin (IB-1) Infiltration Basin (IB-2)		7,531 4,409 150 488	 CF Volume below 286.00' Orifice CF Volume below 288.50' Orifice CF Volume below 284.50' Orifice CF Volume below 282.50' Orifice
Total Provided Iniltration Volume		12,578	CF

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Elevation Surface Storage Elevation Surface Storage (feet) (cubic-feet) (feet) (sq-ft) (cubic-feet) (sq-ft) 284.00 208 286.65 2,085 2,566 0 284.05 225 11 286.70 2,141 2,672 284.10 242 22 286.75 2,197 2,780 284.15 260 35 286.80 2,255 2,891 284.20 278 48 286.85 2,313 3,006 284.25 297 63 286.90 2,372 3,123 284.30 317 78 286.95 2,432 3,243 284.35 338 95 287.00 2,492 3,366 284.40 359 112 284.45 380 130 284.50 150 403 284.55 426 171 284.60 449 193 473 284.65 216 284.70 498 240 284.75 524 265 284.80 550 292 284.85 577 321 284.90 604 350 381 284.95 632 413 285.00 661 285.05 692 447 724 483 285.10 520 285.15 757 285.20 791 558 599 285.25 825 860 641 285.30 285.35 895 685 285.40 932 730 285.45 969 778 1,007 285.50 827 285.55 1,045 878 1,085 285.60 932 987 285.65 1,125 285.70 1,165 1,044 1,104 285.75 1,207 285.80 1,249 1,165 285.85 1,292 1,228 285.90 1,336 1,294 285.95 1,380 1,362 286.00 1,425 1,432 286.05 1,471 1,505 1,579 286.10 1,518 286.15 1,566 1,656 286.20 1,615 1,736 286.25 1,664 1,818 1,902 1,714 286.30 1,765 1,989 286.35 286.40 1,816 2,079 1,868 2,171 286.45 286.50 1,921 2,266 286.55 1,975 2,363 286.60 2,030 2,463

Stage-Area-Storage for Pond IB-1:

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Stage-Area-Storage for Pond IB-2:

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
282.00	870	0	284.65	2,205	3,983
282.05	891	44	284.70	2,235	4,094
282.10	911	89	284.75	2,264	4,207
282.15	933	135	284.80	2,294	4,321
282.20	954	182	284.85	2,323	4,436
282.25	975	231	284.90	2,353	4,553
282.30	997	280	284.95	2,384	4,672
282.35	1,019	330	285.00	2,414	4,792
282.40	1,042	382			
282.40	1,004	434			
282.50	1,007	543			
282.60	1,110	500			
282.65	1,100	656			
282.70	1,180	715			
282.75	1.204	775			
282.80	1,229	835			
282.85	1,253	897			
282.90	1,278	961			
282.95	1,303	1,025			
283.00	1,328	1,091			
283.05	1,352	1,158			
283.10	1,376	1,226			
283.15	1,400	1,296			
283.20	1,424	1,366			
283.25	1,449	1,438			
203.30	1,474	1,011			
283.40	1,499	1,505			
283.45	1,524	1,001			
283 50	1,040	1,700			
283.55	1,601	1.895			
283.60	1,627	1,976			
283.65	1,653	2,058			
283.70	1,680	2,141			
283.75	1,706	2,226			
283.80	1,733	2,312			
283.85	1,760	2,399			
283.90	1,788	2,488			
283.95	1,815	2,578			
284.00	1,843	2,669			
204.00	1,070	2,702			
204.10	1,097	2,000			
284 20	1,924	3 049			
284.25	1,979	3 147			
284.30	2.006	3.247			
284.35	2.034	3.348			
284.40	2,062	3,450			
284.45	2,090	3,554			
284.50	2,119	3,659			
284.55	2,148	3,766			
284.60	2,176	3,874			
			I		

Stage-Area-Storage for Pond IC-1:

Elevation	Surface	Storage	Elevation	Surface	Storage
292 50				(54-11)	
203.00	4,404	0	200.10	4,404	7,911
203.00	4,404	176	200.20	4,404	0,020
203.00	4,404	264	200.25	4,404	0,139
203.00	4,404	204	200.30	4,404	0,244
203.70	4,404	352	200.00	4,404	0,343
203.75	4,404	440 529	200.40	4,404	0,430
203.00	4,404	520 617	200.45	4,404	8,551
203.00	4,404	705	200.00	4,404	8,020
203.90	4,404	703	286.60	4,404	8 706
200.00	4,404	881	286.65	4,404	8 884
284.05	4 404	1 065	286 70	4 404	8 972
284 10	4 404	1 249	286 75	4 404	9.061
284 15	4 404	1 433	286.80	4 404	9 149
284 20	4 404	1,400	286.85	4 404	9 237
284 25	4 404	1 799	286.90	4 404	9,325
284.30	4 404	1,981	286.95	4 404	9 4 1 3
284.35	4 404	2 163	287.00	4 404	9 501
284 40	4 404	2 344	287.05	4 404	9 501
284.45	4,404	2,524	287.10	4,404	9,501
284.50	4,404	2,703	287.15	4,404	9,501
284.55	4,404	2,882	201110	1,101	0,001
284.60	4.404	3.060			
284.65	4.404	3.237			
284.70	4,404	3.413			
284.75	4,404	3,588			
284.80	4,404	3,763			
284.85	4,404	3,936			
284.90	4,404	4,108			
284.95	4,404	4,280			
285.00	4,404	4,450			
285.05	4,404	4,619			
285.10	4,404	4,787			
285.15	4,404	4,954			
285.20	4,404	5,119			
285.25	4,404	5,283			
285.30	4,404	5,446			
285.35	4,404	5,607			
285.40	4,404	5,766			
285.45	4,404	5,924			
285.50	4,404	6,081			
285.55	4,404	6,236			
285.60	4,404	6,389			
285.65	4,404	6,540			
285.70	4,404	6,689			
285.75	4,404	6,835			
285.80	4,404	6,979			
285.85	4,404	7,121			
285.90	4,404	7,260			
285.95	4,404	7,397			
280.00	4,404	7,531			
200.00	4,404	1,001 7,700			
200.10	4,404	1,100			

Stage-Area-Storage for Pond IC-2:

Elevation	Surface	Storage	Elevation	Surface	Storage
	(SQ-IL)			(SQ-IL)	
287.00	4,361	0	289.65	4,361	7,839
287.05	4,361	87	289.70	4,361	7,955
287.10	4,361	174	289.75	4,361	8,065
287.15	4,361	262	289.80	4,361	8,169
287.20	4,361	349	289.85	4,361	8,267
287.25	4,361	436	289.90	4,361	8,361
287.30	4,361	523	289.95	4,361	8,453
287.35	4,361	611	290.00	4,361	8,541
287.40	4,361	698	290.05	4,361	8,629
287.45	4,361	785	290.10	4,361	8,716
287.50	4,361	872	290.15	4,361	8,803
287.55	4,361	1,055	290.20	4,361	8,890
287.60	4,361	1,237	290.25	4,361	8,978
287.65	4,361	1,419	290.30	4,361	9,065
287.70	4,361	1,601	290.35	4,361	9,152
287.75	4,361	1,782	290.40	4,361	9,239
287.80	4,361	1,963	290.45	4,361	9,326
287.85	4,361	2,143	290.50	4,361	9,414
287.90	4,361	2,322			
287.95	4,361	2,501			
288.00	4,361	2,678			
288.05	4,361	2,856			
288.10	4,361	3,032			
288.15	4,361	3,207			
288.20	4,361	3,382			
288.25	4,361	3,556			
288.30	4,361	3,728			
288.35	4,361	3,900			
288.40	4,361	4,071			
288.45	4,361	4,241			
288.50	4,361	4,409			
288.55	4,361	4,577			
288.00	4,301	4,744			
200.00	4,301	4,909			
288.70	4,301	5,072			
200.70	4,301	5,235			
200.00	4,301	5,390 5,556			
200.00	4,301	5,550 5,714			
200.90	4,301	5,714 5 971			
200.90	4,301	5,071			
209.00	4,301	0,020			
209.00	4,301	0,179			
209.10	4,301	6 4 9 1			
209.10	4,301	6 6 2 8			
209.20	4,301	0,020			
209.20	4,001	0,773			
209.00	4,501	7 056			
209.00	4,501	7,000			
209.40	4,501	7 220			
209.40	4,501	7,000			
289.50	4,001 ⊿ 261	7,402			
289.60	4,301	7,552			
200.00	4,001	7,717			

DRAINAGE GROUNDWATER SEPARATION SUMMARY:

Stormwater Infiltration Basin #1

Groundwater Depth =	45 IN	[See 122-4]
Bottom Basin Elev. =	286.0	
Existing grade @ Bot. Basin =	287.6	
Groundwater Separation =	2.15 FT	
Infiltration Chambers-1		
Groundwater Depth =	50 IN	[See 122-5]
Bottom Bed Elev. =	283.5	
Existing grade @ Bot. Bed =	285.4	
Groundwater Separation =	2.27 FT	
Stormwater Infiltration Basin #2		
Groundwater Depth =	15 IN	[See 122-6]
Bottom Basin Elev. =	282.0	
Existing grade @ Bot. Basin =	280.0	
Groundwater Separation =	3.25 FT	
Infiltration Chambers-2		
Groundwater Depth =	60 IN	[See 122-8]
Bottom Bed Elev. =	287.0	
Existing grade @ Bot. Bed =	290.0	
Groundwater Separation =	2.00 FT	

Infiltration Area Requirements

Drawdown Time

(Per Massachusetts Stormwater regulations, infiltration areas must completely drain within 72 hours)

		Infiltration Chambers (IC-1)	Infiltration Chambers (IC-2)
Infiltration Area Storage Volume	cf	7,531	4,409
Design infiltration Rate	in/hr	1.02	1.02
Infiltration Bottom Area	sf	4,404	4,362

Drawdown Time = Infiltration Area Storage Volume / [Design Infiltration Rate x Infiltration Area Bottom Area]

Drawdown Time (Hrs) 20.1	11.9
--------------------------	------

Mounding Analysis

Per the Massachusetts Stormwater Handbook, mounding analysis is required when ".. The vertical separation from the bottom of an exfiltration system to seasonal high groundwater is less than four (4) feet and the recharge system is proposed to attenuate the peak discharge from a 10-year or higher 24-hour storm." The mounding analysis "... must show that the REQUIRED RECHARGE VOLUME is fully dewatered within 72 hours..."

		Infiltration Chambers (IC-1)	Infiltration Chambers (IC-2)
Hydraulic Conductivity	ft/day	16	16
		Lower Range Standard Value for "Me	edium Sand" material
Specific Yield		0.23	0.23
		Standard Value for "Medium Sand" m	naterial
Initial Saturated Thickness	ft	10	10
		Depth to bedrock	
Design Recharge Rate	ft/day	2.04	2.04
		infiltration rate	
Time	days	3	3
		Minimum 72 hr evaluation period	
Bottom Infiltrating Area	sf	4,404	4,362
Length of Infiltration Area	ft	81.94	110.42
Width of Infiltration Area	ft	53.7	39.5
Time when Infiltration Stops	days	0.84	0.50
		Calculated Drawdown Time (see Abo	ve)
Maximum Water table rise at 72 hours ¹	ft	1.27	0.74
	in	15 1/4	8 7/8

- Resulting mound will not interfere with the full draining of the infiltration area in accordance with Mass Stormwater Standards -

¹- mounding analysis calculated using the Hantush (1967) method. Automated report generated by MOUNDSOLV V2.1 (www.aqtesolv.com) by HydroSOLVE, Inc.

Infiltration Area Requirements

Drawdown Time

(Per Massachusetts Stormwater regulations, infiltration areas must completely drain within 72 hours)

		Infiltration Basin (IB-1)	Infiltration Basin (IB-2)
Infiltration Area Storage Volume	cf	150	488
Design infiltration Rate	in/hr	1.02	1.02
Infiltration Bottom Area	sf	208	834

Drawdown Time = Infiltration Area Storage Volume / [Design Infiltration Rate x Infiltration Area Bottom Area]

Drawdown Time (Hrs)	8.5	6.9
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Mounding Analysis

Per the Massachusetts Stormwater Handbook, mounding analysis is required when ".. The vertical separation from the bottom of an exfiltration system to seasonal high groundwater is less than four (4) feet and the recharge system is proposed to attenuate the peak discharge from a 10-year or higher 24-hour storm." The mounding analysis "... must show that the REQUIRED RECHARGE VOLUME is fully dewatered within 72 hours..."

		Infiltration Basin (IB-1)	Infiltration Basin (IB-2)
Hydraulic Conductivity	ft/day	16	16
	-	Lower Range Standard Value for "Medi	um Sand" material
Specific Yield		0.23	0.23
		Standard Value for "Medium Sand" ma	terial
Initial Saturated Thickness	ft	10	10
		Depth to bedrock	
Design Recharge Rate	ft/day	2.04	2.04
		infiltration rate	
Time	days	3	3
		Minimum 72 hr evaluation period	
Bottom Infiltrating Area	sf	208	834
Length of Infiltration Area	ft	31	12.67
Width of Infiltration Area	ft	6.7	65.8
Time when Infiltration Stops	days	0.35	0.29
		Calculated Drawdown Time (see Above	2)
Maximum Water table rise at 72 hours ¹	ft	0.03	0.08
	in	1/4	1

- Resulting mound will not interfere with the full draining of the infiltration area in accordance with Mass Stormwater Standards -

¹- mounding analysis calculated using the Hantush (1967) method. Automated report generated by MOUNDSOLV V2.1 (www.aqtesolv.com) by HydroSOLVE, Inc.

Stormwater Management Standard 4 WATER QUALITY RETENTION VOLUME

Parameter	Unit	Quantity	Remarks	5
Watershed area	sf	270,181		
Predevelopment impervious area Total impervious area added Total impervious area Total impervious area required for retention	sf sf sf sf	0 121,370 121,370 121,370	_	
Runoff depth over impervious area	IN	0.5	_	
Required Water Quality Volume	CF	5,057		
Provided Water Quality Volume				
Infiltration Chambers (IC-1) Infiltration Chambers (IC-2) Infiltration Basin (IB-1) Infiltration Basin (IB-2)		7,531 4,409 150 488	CF Volu CF Volu CF Volu CF Volu CF Volu	ime below 286.00' Orifice ime below 288.50' Orifice ime below 284.50' Orifice ime below 282.50' Orifice
DESIGN VOLUME PROVIDED	CF	12,578		

Stormwater Management Standard 4 TSS REMOVAL

Process Train No.	Impervious Area (SF)	ВМР Туре	TSS Removal Rate	TSS Remaining at Discharge	TSS Removed at Discharge
SC1.1, SC1.2, SC1.3, SC3.3	33,180	СВ	25%	75%	25%
		FB-1 / IB-1	80%	15%	85%
SC3.1, SC3.2	37,292	СВ	25%	75%	25%
		IC-1	80%	15%	85%
SC2.1, SC2.2, SC4.1,	42,001	СВ	25%	75%	25%
004.0, 004.4		IC-2	80%	15%	85%
SC2.3, SC4.2, SC4.5	4,471	СВ	25%	75%	25%
		FB-2 / IB-2	80%	15%	85%

<u>ABBREVIATIONS:</u> TSS=total suspended solids; SF=square feet; SC=subcatchment; GC=grassed channel; BMP=best management practices; CB=deep CB=deep sump hooded catch basin; FB = Sediment Forebay; IB=infiltration basin; IC=infiltration chambers

Massachusetts Stormwater Standards

Standard 10 - Illicit Discharge Compliance Statement

Site Address:	203 Ayer Road, Harvard, MA
Owner:	Wheeler Realty Trust
Applicant, if different:	Yvonne Chern & Wheeler Realty Trust
Plan Reference:	Commercial Development Special Permit 203 Ayer Rd
DEP File Number:	CE 177-0711

As required by Standard 10 of the Massachusetts Stormwater Standards, I, the undersigned, being the Owner of the subject property do hereby certify that the stormwater system, as shown on the referenced plan, does not permit any illicit discharges to enter the stormwater management system. I also certify that the existing use of the property does not permit any illicit discharges.

Illicit discharges are discharges not associated with the following: stormwater; water from fire fighting; water line flushing or street washing; landscape watering and irrigation; uncontaminated groundwater; potable water; foundation or footing drains; air conditioning condensate; residential vehicle washing; residential non-detergent building cleaning water, de-chlorinated water from swimming pools; flows from riparian habitats or wetlands.

Further, I certify that the stormwater management system shown on the referenced plan will be maintained in accordance with the Operations and Maintenance Manual submitted with the Notice of Intent and approved by the Conservation Commission.

Signed: _____, GPR. AS AGENT OF OWNER

Print:

Owner or Authorized Applicant

LIMHUOT TIV

7/21/22 Date

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